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SECTION - 1

SCOPE, SPECIFIC TECHNICAL REQUIREMENTS & QUANTITIES

1.1.0 SCOPE

1.1.1 The scope of work under this specification is Civil Works for 400kV GIS switchyard at Ennore in Tamil Nadu being executed by BHEL on turnkey basis. The Customer is Tamil Nadu State Generation and Distribution Corporation Ltd.

1.1.2 The Civil Works shall generally include, *but not limited to*, following:

- (i) 400kv tower and equipment foundations including transformer/ reactor foundations.
- (ii) Civil works of GIS building and Control room buildings
- (iii) Ground improvement and piling and pile cap work including testing
- (iv) Cable trenches, drains, roads, road crossings, fencing and gates
- (v) Antiweed treatment, gravel spreading
- (vi) Any other work required for the completion of project.

1.1.3 The works to be performed in the above constructions include preparation of bar bending schedules, based on the drawings released for construction and getting the same approved by the Engineer-in-charge plus the execution of the work including providing of all labour, supervision, materials, scaffolding, power, fuel, construction equipments, tools and plants, supplies, transportation, all incidental items necessary for successful completion of the work including contractor's supervision and in strict accordance with the drawings and specifications and with inspection and testing standards. The nature of work shall generally involve ground improvement, piling, pile head breaking, pile caps, excavation in all type of soil and rock including dewatering, shoring, strutting, and filling under and around structures, backfilling with available excavated earth around completed structures and cable trenches with covers, road crossing of trenches, disposal of surplus soil, concreting including formwork, providing necessary steel embedments and other inserts, concreting, brickwork, doors/windows, false ceiling, roof treatment, flooring, painting and finishing in buildings, rain water harvesting etc., all complete as per detailed specification, drawings and directions of Engineer-in-charge.

1.2.0 SPECIFIC TECHNICAL REQUIREMENT

1.2.1 The specific technical requirements for the execution of civil works shall be as per Customer's Specifications (Section-3) & IS Codes. In case of any conflict between these, Customer's specifications shall prevail.

1.3.0 BILL OF QUANTITIES

- 1.3.1 The Bill of Quantity-cum-Price Schedule shall be as per page 1.3 to page 1. 30.
- 1.3.2 The quantities indicated in the 'Bill of Quantity-cum-Price Schedule' are indicative and can vary to any extent. Contractor shall not be entitled for any claim for any such variation in the quantities.
- 1.3.3 The provision of Bill of Quantity-cum-Price Schedule, specifications and drawings shall be read in conjunction with each other and in case of conflict amongst them, the clarification shall be obtained from the Engineer-in-charge whose decision shall be final and binding.
- 1.3.4 Method of measurement:
Excavation shall be measured in cubic meters. **The lateral dimensions to be considered for working out excavation quantity shall be the PCC dimension below the footing as per approved drawing. *Nothing extra shall be paid for slope cutting, etc.*** Backfilling & disposal quantities shall be worked out based on the above dimensions only. However the contractor shall maintain the required slope and working space as per the safety /statutory requirement and its cost is deemed to be included in the quoted rate.
- For other items, unless otherwise described the method of measurement as described in 'Method of Measurement of Building and Civil Engineering Works'-IS 1200(Part I to XXV) latest edition of BIS shall be followed

SECTION - 2

STANDARD TECHNICAL SPECIFICATION
(N.A.)

SECTION - 3

ENCLOSURES TO THE SPECIFICATION
(CUSTOMER'S SPECIFICATION)



TAMILNADU GENERATION AND DISTRIBUTION CORPORATION

**2 x 660 MW Ennore SEZ Supercritical Thermal Power Project at Ash
Dyke of NCTPS, CHENNAI – 120**

SPECIFICATION


FOR

EPC CUM DEBT FINANCING CONTRACT

BID SPECIFICATION NO: CE/C/P&E/EE/E/OT.No. 03/2013-14

**VOLUME – VI
PART – A & B**

CIVIL, STRUCTURAL & ARCHITECTURAL WORKS

Job No.	D-4027	Name / Sign	Date		 DESEIN PRIVATE LIMITED DESEIN HOUSE, GREATER KAILASH – II. NEW DELHI – 110 048
Prepared By		NITIN	03.05.2013		
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Specification No.	CE/C/P&E/EE/E/OT.No. 03/2013-14	Rev. No.	00		

PART – A

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INTRODUCTION

TANGEDCO has planned to establish 2 x 660 MW Coal Based Super Critical Thermal Power Plant in Vayalur Village Near Ennore Port. Plant will be set up in existing Ash Dyke of NCTPS by reclamation of some portion of the Ash Dyke. TNEB is presently having about 1100 acres of land for Ash Dyke purpose. Due to improved Dry Fly Ash Collection, it is proposed to reclaim 500 acres of the above land for setting up of this proposed Power Plant.

A Project Information & Location

Project Title	:	Ennore SEZ project of 2 x 660 MW Coal Based Super Critical Thermal Power Project at ash dyke of NCTPS
Plant capacity	:	1320 MW (2 units of 660 MW each)
Type of project	:	Green field
Owner	:	Tamil Nadu Generation and Distribution Corporation Limited (TANGEDCO)
Plant site location	:	Ash dyke of North Chennai Thermal Power Station (NCTPS)
Location co-ordinates	:	80° 18' E to 80° 19' E Longitude 13° 17' N to 13° 18' N Latitude
Nearest Village	:	Vayalur
Nearest Town & City	:	Chennai (35 Km)
State Capital	:	Chennai (35 Km)
Nearest Railway Station	:	Athipattu Pudunagar (~5 Km)
Nearest Airport	:	Chennai (~60 Km)
Nearest Seaport	:	Ennore (~5 Km)
Nearest Road access	:	All weather road from Pattamandri on the Thiruvottiur – Ponneri district highway

B Meteorological Condition

Climate	:	Tropical, very dry and hot summer, dry and cold winter and good rain-fall in monsoon accompanied with strong wind
Site Elevation	:	(+) 10.0 Meter above Mean Sea Level
Ambient Temperature		
a) Annual Maximum Mean Temperature	:	32° C



- b) Annual Minimum Mean Temperature : 24° C
c) Design ambient temperature : 35° C

Relative Humidity

- a) Maximum : 100%
b) Minimum : 36%
c) Design : 75%

Annual Rainfall

- Maximum : 2540 mm
Average : 1600 mm
Minimum : 1175 mm

- Prevailing Wind Direction** : November to January – From NW & NE
February to March – From East & SE
April to May – From South & SE
June – From SW
July to August – From NW
September to October – From SE & SW

- Wind Speed** : 11.8 kmph (Avg), 50m/s max)

- Seismic zone** : Zone: III as defined in IS:1893-2002

- Design ambient temperature** : 50° C
For Electrical equipments



1.00.00 CODES AND STANDARDS

Following is a general listing of Codes and Standards to be used in the design of the Plant. Specific applicable codes and standards will be identified in System Design Descriptions / Technical Specifications as appropriate. The latest editions / revision of following codes and standards along with addendums / amendments, if any, shall be followed:

1.01.00 General

- a) Internationally accepted design Codes and Standards where Indian Codes are not available and which are equivalent to Indian Standards.
- b) National Building Code of India.
- c) “Accepted Standards” and “good Practice” listed in the appendix to National Building Code of India.
- d) IS:1200 : Method of measurement of Building and Civil Engineering Works.
- e) IS:1256 : Code of Practice for Building Byelaws.

1.02.00 Earthwork

- a) IS-1498 : Classification and identification of soils for General Engineering purpose
- b) IS-3764 : Safety code for excavation work
- c) IS:7293 : Safety code for working with construction machinery

1.03.00 Concrete

- a) IS-269 : Ordinary and low heat Portland cement
- b) IS-383 : Coarse and fine aggregate from natural sources for concrete
- c) IS-432 : Mild steel and medium tensile steel bars and hard drawn steel wire for concrete reinforcement
- d) IS-455 : Portland slag cement
- e) IS-456 : Code of practice for plain and reinforced concrete
- f) IS-460 : Test Sieves (all parts)
- g) IS-516 : Methods of test for strength of concrete
- h) IS-1199 : Methods of sampling and analysis of concrete



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|----|----------|---|---|
| i) | IS-1566 | : | Hard drawn steel wire fabric for concrete reinforcement |
| j) | IS-1786 | : | High strength deformed steel bars and wires for concrete reinforcement |
| k) | IS-1834 | : | Hot applied sealing compounds for joints in concrete |
| l) | IS-2386 | : | Methods of test for aggregates for concrete (all parts) |
| m) | IS:2502 | : | Code of practice for bending and fixing of bars for concrete reinforcement |
| n) | IS:3370 | : | Code of practice for concrete structures for storage of liquids (all parts) |
| o) | IS-3414 | : | Code of practice for design and installation of joints in buildings |
| p) | IS-4948 | : | Welded steel wire fabrics for general use. |
| q) | IS-6452 | : | High alumina cement for structural use |
| r) | IS-7320 | : | Concrete slump test apparatus |
| s) | IS-7861 | : | Code of practice for extreme weather concreting (all parts) |
| t) | IS-8041 | : | Rapid hardening Portland cement |
| u) | IS-8112 | : | High strength ordinary Portland cement |
| v) | IS-10262 | : | Recommended guidelines for concrete mix design. |
| w) | IS:13290 | : | Ductile detailing of RCC structure subjected to seismic loads |
| x) | SP:34 | : | Handbook on concrete reinforcement and detailing |

1.04.00**Foundations**

- | | | | |
|----|---------|---|---|
| a) | IS-1904 | : | Code of practice for structural safety of building - Shallow foundations |
| b) | IS-2950 | : | Code of practice for design and construction of raft foundations |
| c) | IS-2974 | : | Code of practice for design and construction of Machine foundations (all parts) |



- d) IS-2911 : Code of practice for design and construction of pile foundation.
- e) IS-9716 : Lateral dynamic load test on pile
- f) IS-6313 (Part-1 & Part-2) : Code of Practice for anti-termite measures and treatment.

1.05.00**Loading**

- a) IS-875 : Code of practice for design loads for buildings and structures
- b) IRS : Bridge Rules of Government of India, Ministry of Railways (Railway Board)

1.06.00**Masonry**

- a) IS-712 : Building limes
- b) IS-1077 : Common burnt clay building bricks
- c) IS-1127 : Recommendations for dimensions and workmanship of natural building stones for masonry work.
- d) IS-1528 : Methods of sampling and physical tests for refractory materials
- e) IS-1597 : Code of practice for construction of stone masonry (all parts)
- f) IS-2212 : Code of practice for brickwork
- g) IS-2116 : Sand for masonry mortars
- h) IS-2185 : Concrete masonry units. (all parts – Hollow and Solid concrete blocks)
- i) IS-2250 : Code of practice for preparation and use of masonry mortars
- j) IS:2572 : Code of practice for construction of hollow concrete block masonry
- k) IS-2691 : Burnt clay facing bricks



- l) IS-3414 : Code of practice for design and installation of joints in buildings
- m) IS-3495 : Methods of tests of burnt clay building bricks
- n) IS-4441 : Code of practice for use of silicate type chemical resistant mortars
- o) IS-4860 : Acid resistant bricks

1.07.00**Doors, Windows & Ventilators**

- a) IS-399 : Classification of commercial timbers and their zonal distribution
- b) IS-883 : Code of practice for design of structural timber in buildings
- c) IS-1003 : Timber paneled and glazed shutters (all parts)
- d) IS-1038 : Steel doors, windows and ventilators
- e) IS-1081 : Code of practice for fixing and glazing of metal (steel and aluminium) doors, windows and ventilators
- f) IS-1361 : Steel windows for industrial buildings
- g) IS-2835 : Transparent sheet glass for glazing and framing purposes
- h) IS-1948 : Aluminium doors, windows and ventilators
- i) IS-1949 : Aluminium windows for industrial building
- j) IS-2191 : Wooden flush door shutters (cellular and hollow core type)
- k) IS-2202 : Wooden flush door shutters (solid core type)
- l) IS-3103 : Code of practice for industrial ventilation
- m) IS-3548 : Code of practice for glazing in buildings
- n) IS-3614 : Fire check doors
- o) IS-4021 : Timber door, windows and ventilator frames
- p) IS-4351 : Steel door frames
- q) IS-6248 : Metal rolling shutters and rolling grills



1.08.00**Roof & Flooring**

- a) IS-2204 : Code of practice for construction of reinforced concrete shell roof
- b) IS-3201 : Criteria for the design and construction of precast concrete trusses
- c) IS-2210 : Criteria for design of RC shell structures and folded plates
- d) IS-809 : Rubber flooring materials for general purposes
- e) IS-1195 : Bitumen mastic for flooring
- f) IS-1196 : Code of practice for laying bitumen mastic flooring
- g) IS-1198 : Code of practice for laying, fixing and maintenance of linoleum floors
- h) IS-1237 : Cement concrete flooring tiles
- i) IS-1443 : Code of practice for laying and finishing of cement concrete flooring tiles
- j) IS-2114 : Code of practice for laying in situ terrazzo floor finish
- k) IS-2571 : Code of practice for laying in situ cement concrete flooring
- l) IS-5491 : Code of practice for laying in situ granolithic concrete floor topping
- m) IS-5766 : Code of practice for laying burnt clay brick flooring
- n) IS-1197 : Code of practice for laying of rubber floors
- o) IS:2441 : Code of practice for fixing ceiling coverings.

1.09.00**Waterproofing**

- a) IS-1322 : Bitumen felts for waterproofing and damp proofing
- b) IS-1346 : Code of practice for waterproofing of roofs with bitumen felts



- c) IS-1609 : Code of practice for laying damp proof treatment using bituminous felts.
- d) IS-3036 : Code of practice for laying lime concrete for a water proofed roof finish
- e) IS-3037 : Bitumen mastic for use in waterproofing of roofs.
- f) IS-3067 : Code of practice for general design, details and preparatory work for damp proofing and water proofing of buildings
- g) IS-3384 : Bitumen primer for use in water proofing and damp proofing
- h) IS-4365 : Code of practice for application of bitumen mastic for waterproofing of roofs.

1.10.00**Soil Engineering**

- a) IS-1498 : Classification and identification of soils for general engineering purposes
- b) IS-1892 : Code of practice for sub-structure investigation for foundations
- c) IS-2131 : Method for standard penetration test for soils
- d) IS-2720 : Methods of test for soils (all parts)

1.11.00**Water Supply, Drainage & Sewerage**

- a) IS-404 : Lead pipes
- b) IS-458 : Concrete pipes
- c) IS:651 : Salt glazed stoneware pipes and fittings
- d) IS-771 : Glazed fire-clay sanitary appliances (all parts)
- e) IS-774 : Flushing cisterns for water closets and urinals other than plastic cisterns
- f) IS-783 : Code of practice for laying of concrete piles
- g) IS-1172 : Code of basic requirements for water supply, drainage and sanitation
- h) IS-1626 : Asbestos cement building pipes, gutters and fittings (all parts)



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| i) | IS-1742 | : | Code of practice for building drainage |
| j) | IS-2064 | : | Code of practice for selection, installation and maintenance of sanitary appliances |
| k) | IS-2065 | : | Code of practice for water supply in buildings |
| l) | IS-2470 | : | Code of practice for installation of septic tanks (all parts) |
| m) | IS-3114 | : | Code of practice for laying of cast iron pipes |
| n) | IS-4127 | : | Code of practice for laying of glazed stoneware pipes |
| o) | IS-12251 | : | Code of practice for drainage of building basement |
| p) | IS-1200 | : | Method of measurement : Laying of water and (Part-XVI) sewer lines including appurtenant items |
| q) | IS-1536 | : | Centrifugally cast (spun) iron pressure pipes for water, gas and sewage |
| r) | IS-1537 | : | Vertically cast iron pressure pipe for water, gas and sewage |
| s) | IS-3486 | : | Cast iron spigot and socket drain pipes |
| t) | IS-5329 | : | Code of practice for sanitary pipe work above ground for buildings |
| u) | IS-3076 | : | Low density polyethylene pipes for potable water supplies |
| v) | IS-1538 | : | Cast iron fittings for pressure pipes for potable water supplies |
| w) | IS-1230 | : | Cast iron rainwater pipes and fittings |
| x) | IS-1729 | : | Sand cast iron spigot and socket soil, waste and ventilating pipes, fittings and accessories |
| y) | IS-784 | : | Pre-stressed concrete pipes |
| z) | IS-1726 | : | Cast iron manhole covers and frames |
| aa) | IS-5961 | : | Cast iron grating for drainage purposes |
| bb) | IS-5219
[Part-I] | : | “P” and “S” traps |
| cc) | IS-772 | : | General requirements for enameled cast iron sanitary |



appliances

- dd) IS-775 : Cast iron brackets and supports for wash basins and sinks
- ee) IS-777 : Glazed earthenware wall tiles
- ff) IS-2548 : Plastic water closet seats and covers (all parts)
- gg) IS-2527 : Code of practice for fixing rainwater gutters and down pipes for roof drainage.

1.12.00 Paving & Road Works

- a) IS-73 : Paving bitumen
- b) IS-702 : Industrial bitumen
- c) IS:1201 : Method of testing tar and bituminous materials thru' 1220
- d) IRC-15 : Standard Specification and code of practice for construction of concrete roads.
- e) IRC-58 : Guidelines for the design of plain jointed rigid pavement for highways
- f) IRC-58 : Guidelines on cement fly ash concrete for rigid pavement.

1.13.00 Earthquake Resistant Design

- a) IS-1893 : Criteria for earthquake resistant design of structures
- b) IS-4326 : Code of practice for earthquake resistant design and construction of buildings

1.14.00 Chimney

- a) IS-4998 : Criteria for design of R.C. Chimneys (all parts)

1.15.00 Structural Steel Work

- a) IS-800 : Code of practice for general construction in steel
 - b) IS-802 : Code of practice for use of structural steel in overhead transmission line
- Part-I: Load and permissible stresses



Part-II: Fabrication, galvanizing, inspection and packing.

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| c) | IS-806 | : | Code of practice for use of steel tubes in general building construction |
| d) | IS-808 | : | Rolled steel beams, channels and angle sections |
| e) | IS-813 | : | Scheme for symbols for welding |
| f) | IS-814 | : | Covered electrodes for manual metal arc welding for carbon and carbon manganese steel |
| g) | IS-816 | : | Code of practice for use of metal arc welding for general construction in mild steel |
| h) | IS-817 | : | Code of practice for training and testing of metal arc welders |
| i) | IS-818 | : | Code of practice for safety and health requirements in electric and gas welding and cutting operation |
| j) | IS-819 | : | Code of practice for resistance spot welding for light assemblies in mild steel |
| k) | IS-919 | : | Recommendations for limits and fits for engineering. |
| l) | IS-1024 | : | Code of practice for use of welding in bridges and structures subjected to dynamic loading |
| m) | IS-1161 | : | Steel tubes for structural purposes |
| n) | IS-1182 | : | Recommended practice for radiographic examination of fusion welded butt joints in steel plates |
| o) | IS-1200
[Part-VIII] | : | Method of measurement of steelwork and iron work |
| p) | IS-1239 | : | Mild steel tubes, tubulars and other wrought steel fittings (all parts) |
| q) | IS-1363 | : | Black hexagonal bolts, nuts and locknuts (dia. 6 to 39 mm) and black hexagon screws (dia. 6 to 24 mm) [all parts]. |
| r) | IS-1364 | : | Precision and semi-precision hexagon bolts, screws, nuts and locknuts (dia. range 6 to 39 mm) [all parts] |
| s) | IS-1365 | : | Slotted counter sunk head screws (dia range 1.6 to 20 mm) |
| t) | IS-1367 | : | Technical supply conditions for threaded steel |



			fasteners
u)	IS-1443	:	Code of practice for laying and finishing of cement concrete flooring tiles.
v)	IS-1608	:	Methods of tensile testing of steel products
w)	IS-1730	:	Dimensions for steel plate, sheet and strip for structural and general engineering purpose
x)	IS-1731	:	Dimensions for steel flats for structural and general engineering purposes
y)	IS-1852	:	Rolling and cutting tolerances for hot rolled steel products
z)	IS-1977	:	Structural steel (ordinary quality)
aa)	IS-2016	:	Plain washers
bb)	IS-2062	:	Steel for general structural purposes
cc)	IS-2074	:	Ready mixed paint, air drying, red oxide zinc-chrome, priming
dd)	IS-2633	:	Methods of testing uniformity of coating of zinc coated articles.
ee)	IS-3613	:	Acceptance test for wire-flux combinations for submerged arc welding of structural steel
ff)	IS-3664	:	Code of practice for ultrasonic pulse echo testing by contact and immersions methods
gg)	IS-3757	:	High strength structural bolts
hh)	IS-4000	:	High strength bolts in steel structures
ii)	IS-4759	:	Hot dip zinc coatings on structural steel and other allied products
jj)	IS-5334	:	Code of practice for magnetic particle flaw detection of welds
kk)	IS-7215	:	Tolerances for fabrication of steel structures
ll)	IS-7280	:	Base-wire electrodes for submerged arc welding of structural steel
mm)	IS-7318 [Part-I]	:	Approval test for welders when welding procedure approval is not required.



- nn) IS-8500 : Structural steel – micro-alloyed (medium and high strength qualities)
- oo) IS-9595 : Recommendation for metal arc welding of carbon and carbon manganese steel
- pp) AWS D.1.1 : Structural welding code.

1.16.00 Painting

- a) IS-348 : Specification for French polish
- b) IS-427 : Specification for distemper, dry colour as required
- c) IS-428 : Specification for distemper, oil emulsion, colour as required
- d) IS-1477 (I & II) : Code of practice for painting of ferrous metal in buildings
- e) IS-2338 (I & II) : Code of practice for finishing of wood and wood based materials
- f) IS-2339 : Specification for Aluminium Paints for general purposes in dual containers
- g) IS-2395 : Code of practice for painting concrete, masonry and plaster surface
- h) IS-2932 : Specification for enamel, synthetic, exterior - (a) undercoating , (b) finishing
- i) IS-2933 : Specification for enamel, exterior – (a) undercoating, (b) finishing.
- j) IS-5410 : Specification for cement paint.

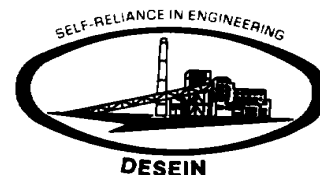
- 1.17.00 a) Indian Road Congress (IRC) Bridge codes
- b) Indian Railways Standard Bridge Rules

1.18.00 Environmental Protection

Charter on Corporate Responsibility for Environmental Protection (CREP) published in Gazette of India dated 27.08.2003.

- 1.19.00 Rules & Regulation of Local Authorities

- 1.20.00 Statutory Regulations of Tariff Regulation Commission (TAC)



2.00.00 SCOPE OF CIVIL & STRUCTURAL WORKS

2.01.00 This specification is to cover design, preparation of general arrangement, construction as well as fabrication drawings, supply of all labour as well as materials and construction of all civil, structural as well as architectural work on EPC basis.

2.02.00 The work to be performed under this specification consist of design, engineering as well as providing all labour, materials, consumables, equipment, temporary works, temporary labour, constructional plant, fuel supply, transportation and all incidental items not shown or specified but reasonably implied or necessary for the completion and proper functioning of the plant, all in strict accordance with the specifications and including revisions and amendments thereto as may be required during the execution of the work.

2.03.00 All materials including cement, reinforcement steel, structural steel etc. shall be arranged by the Contractor.

2.04.00 The scope shall also including setting up by the Contractor a complete testing laboratory in the field to carry out all relevant tests as per BIS or other international standards required for the civil works for the project.

2.05.00 The work shall be carried out according to the design/drawings to be developed by the Contractor and approved by the Purchaser. For all building and structures, foundations, etc. necessary layout and details are to be developed by the Contractor keeping in view the statutory & functional requirements of the plant & facilities and providing enough space & access for operation, use & maintenance. Certain minimum requirements are indicated in this specification for guidance purpose only. However, the Contractor's offer shall cover the complete requirements of the plant & facilities and providing enough space & access for operation, use & maintenance. Also the Contractor's offer shall cover the complete requirements as per the best prevailing practices and to the complete satisfaction of the Purchaser.

2.06.00 Description of various items of work under this specification and nature of work in detail are given hereinafter, but not limited to the items listed below. The complete work under this scope is referred to as CIVIL WORKS. List of various civil works covered under the scope is given below:

1. Turbine building foundation including Turbo-generator foundation and superstructure including service and maintenance bays.
2. Switch Gear and Control Room including Compressor house, D.G. set room and Co2 room.
3. Steam Generator Elevator supporting structure.
4. Foundation for steam generator / ESP / duct supporting structure & other equipments.
5. ESP control room.
6. Mill and bunker building including bunker structures & Mill Foundations.
7. Steel Mill reject silo and foundations to be provided for the same.



8. Foundations for various fans and fan handling system.
9. RCC chimney of 275m height with twin flues.
10. Ash pipe supporting structure both for bottom ash and fly ash (100% dry collection) from plant area upto RCC silo & wet fly ash slurry pipe & supports from silo to ash dyke.
11. Ash / mill reject handling system structures.
12. Fly ash silos.
13. Ash slurry pump house & ash compression house.
14. Buffer hopper tower.
15. Ash handling electrical/ Control room.
16. Intake pump house & channel near existing cooling water forebay of NCTPS-stage II with modification of existing forebay at intake point.
17. Pipe line to carry water from proposed pump house near existing forebay of NCTPS-stage II to proposed power plant area. Pipe line will be supported on RCC piers.
18. Cooling water (CW) pump house, sump including forebay, CW inlet and outlet conduits.
19. Cascade aerator, partial flume, clarifier, reject sump, RO Plant stage –I, RO stage-II, Permeate storage tank , Pump house.
20. RCC CW ducts from CW pump house to Turbine building, Turbine building to cooling tower and RCC channel from cooling tower to CW pump house forebay.
21. Natural draft cooling tower including basin and other apparatus.
22. Service water overhead tank, DM water storage tank, potable water tank, Condensate storage tank & Condensate polishing unit. Capacity to be provided for overhead water tank shall be 500m³ as per mechanical requirement whichever is higher.
23. Filtered Water Pumphouse/ fire water pumphouse & Filtered water reservoir/ fire water storage tank.
24. Clarified water storage tank and pump house.
25. Dual Media filters.
26. DM Plant – Eye washer & whole body shower shall be provided in DM plant area for emergency. Also a building for testing lab for coal, water, chemicals etc., in DM plant should be provided.
27. Electrochlorination Building.



28. Chemical house and bulk chemical area.
29. Neutralization pit, effluent treatment plant & sewage treatment plant.
30. **Rain water harvesting pond**
31. Guard pond and pump house.
32. Foundation of various water pumps.
33. Fuel oil tank (2x3000 KL) Foundation, Fuel oil pump house, fuel oil storage tank dyke area.
34. **External Coal Handling System from JTB-3** upto western side NCTPS Stage I compound wall and pipe conveyors from NCTPS Stage I to plant area with trestles and necessary transfer points.
 From JTB-3 to TH1 -open belt conveyor with all sides completely covered as indicated in layout of ICHP drawing.
 From TH1 to crusher house -pipe conveyor shall be proved by EPC contractor.
 All civil works including foundation for above items is in the scope of EPC contractor.
 - a) Rails complete with fixing accessories for stacker / reclaimer machines and travelling trippers.
 - b) Crusher house and CHP control room.
35. **IN-Plant Coal Handling System:-** Junction towers (underground and above ground) in plant area, trestles and coal Conveyor galleries, cable galleries, stacker reclaimer, emergency reclaim hoppers with RCC water tight pit, underground tunnels, coal stockyard, coal run off pit, dust suppression / extraction and ventilation system, switchgear room, pump houses, cable trays, cable supports, underground cable trenches with cover, monorails, beams for hoists etc.
36. **Switchyard control building and all works for GIS switchyard.**
37. Transformer yard structures including rail track foundation for transformer.
38. Transformer Maintenance Building, Mill & Boiler Maintenance Building & FGD provisions.
39. **Auxiliaries buildings**
 - Service building.
 - Administrative building
 - Air washer building and Plant air compressor building.
 - Hydrogen generation plant (H₂ Plant)
 - Workshop



- Canteen building
- Dozer shed and repair shed
- Fire station
- DG House
- Bank extension counter with ATM and Post office
- Field maintenance building – 3 no.(20 x 10 m²)
- Main gate and Time office cum security complex with two no. of dormitory (100 m² each) with toilet and change rooms for security persons in the first floor. Steel building shall be provided for CW Pump House, **Filtered water/ fire Water Pump House**, Workshop (Excluding Stores and Office area), D.M Plant, Parking sheds. The Structural Steel shall be confirm to IS 2062. The side wall shall be brick cladding for a height of 1.0m above that color coated galvalume sheets of minimum thickness not less than 0.6mm shall be provided. The roof shall be covered with same sheet of adequate thickness to suit the design requirement. Transparent / Translucent sheet may be used to permit natural lighting. Sufficient number of roof extractor shall be provided for effective ventilation. Aluminum glazed windows shall be provided for natural ventilation and lighting.

All other civil items not covered above but required for completion and proper functioning of plant shall form part of scope.

40. Trenches, tunnels, channels, duct banks etc.
41. Structural steel pipe rack and cable racks for pipes and cables for clarified water, D.M. Water, steam pipes, fuel oil pipes and other areas(if required). with walkways.
42. RCC paving from Transformer Yard to chimney including Boiler area.
43. Parking sheds for four wheelers and two wheelers
 - a) Car park – Partly covered parking – total parking area 3000 Sq.M.
 - b) Covered car parking with roof only and sides open shall be provided adjacent to the administrative building for minimum 20 Cars.
 - c) Parking area indicated are net areas only (i.e.) excluding ramp, etc.
 - d) Scooter and Cycle shed shall be provided with roof cover only.(Area 1250 Sq.M).
44. RCC storm water drainage system for the entire plant.
 - a) Site shall be graded upto +8.50m level with available material from existing bund and the balance depth i.e up to +10.00m (+ 10.00m corresponds to RL +



10.00m) shall be filled with approved good quality murrum / gravel (Gravel Blanketing).

Site shall be graded upto +8.50m level by the OWNER. The EPC contractor has to carry out site grading work for the entire plant from +8.50 m level to +9.50 m level with approved quality of murrum/gravel. The finished floor level of Plant shall be +10.00m elevation.

45. Dispensary and first aid centre.
46. Service water system / make up water pipelines.
47. Rain Water Harvesting for all buildings. All roofs tops shall have RWH with filter arrangement. Filtered water shall be pumped to Raw Water Reservoir for its utilization.

Rain Water Harvesting should be implemented as per the scheme / design approved, Central ground water Authority / Board or State ground water authority or MoEF authorized agency shall be consulted for finalization of appropriate rain water harvesting technology. The necessary consultancy charges for the above agency for preparing rain water Harvesting method / scheme for the project to be borne by the Bidder.

48. Site illumination work.
49. All the civil works related to arrangement for lifting of stator.
50. Clearing and removing the buried items if any in the whole proposed plant area and the same is to be deposited to the NCTPS existing stores.
51. The contractor has to fill or excavate the areas to the required formation levels as indicated in the plot plan.
52. 30 metre high lighting towers shall be provided in Coal yard area and Switchyard area.
53. Road Weigh Bridge and Control Room
These shall be RCC framed structure with RCC roof and brick cladding. Adequate windows shall be provided for ventilation & lighting. Roof shall be given liquid membrane water proofing treatment. Flush welded steel doors shall be provided for personnel movement. Toilet facilities shall be provided.
54. All underground structures such as basement, sump etc. and water retaining structures shall be water proofed as per specification.
55. For under deck insulation, Fire Resistant Under deck insulation such as Lloyd foam or equivalent PUF may be considered with suitable specification.
56. Modular furniture of approved make and model with electrical wiring and network cabling shall be provided in all office/operating personnel rooms/conference hall as per owner's requirements.
57. All the building shall have one dining room of owner's requirement.
58. Necessary lift/elevators for the buildings as per standard code shall also be provided by EPC contractor apart from the specifically mentioned lifts/ elevators.
59. Outfall structure- CT blowdown water, RO stage-II reject shall be let out into the seal well of stage-I. Necessary repair work required in seal well of stage-I shall be carried out by EPC contractor.



61. The Work also Includes:

- (a) Site surveying.
- (b) Soil investigation and preparation of soil investigation report and suggestion of type of foundations including design of piles.
- (c) Implementation of ground improvement techniques and post improvement evaluation of sub surface properties up to 15.0 m below the existing Ground level.
- (d) Preparation of Architectural drawings.
- (e) Design, preparation of drawings and construction of all structures.
- (f) Preparation of as built drawings of all structures and facilities to reflect as built status of construction.
- (g) Submission of as built drawings in Autocad in CDs.
- (h) Plumbing & sanitary works.
- (i) Painting.
- (j) Anti-termite and anti-weed treatment in all areas / buildings as per IS: 6313.
- (k) All temporary roads and approach roads necessary for construction purpose shall be of WBM for a minimum width of 4m. All permanent min roads inside the plant, cross road to auxiliary buildings and inspection road around compound wall will be provided by bidder.
- (l) Green belt development-30% of total plant area shall be developed as green belt by the EPC contractor
- (m) Internal compound wall required based on plot plan and layout. External compound wall is in the scope of bidder. Compound wall shall be provided all around the plant boundary. The wall will be 230 thick brick infill/Rcc slab of suitable thickness. Height of wall will be 4.50 m above FGL. Over the 4.5 m high wall 1 m high of 8 string barbed wire of 6 guage with 8 strands and with anti climbing arrangement of 50x50x8 Y shaped angles placed at 2.5 c/c shall be provided.

It is not the intent to specify herein all the works in the scope of this contract. All other buildings, structures and works that are necessary which are not specifically mentioned here but required for construction, operation and maintenance of the power plant or mentioned elsewhere in the specification are deemed to be included in the scope of the Contractor.

In the event of conflict between requirement of any two clauses of this specification, or different codes / standards, the more stringent requirement as per the interpretation of Owner is to be followed by EPC contractor.

All works shall conform to the specification. The works shall conform to high standards of design, engineering and workmanship. Design and construction shall conform in every respect to all local and state regulations governing such works and to stipulations of Indian Standards unless stipulated otherwise in detail specification.



General

It is preferable to adopt Pre fabricated / Pre Engineering structures for CW Pump House, Fire Water Pump House, Workshop (Excluding Stores and Office area), D.M Plant, Parking sheds.

2.07.00**Protection of existing utilities and services**

During construction, the contractor shall provide all protection for existing utilities and services as may be required by his construction operations. Permanent protection of certain items shall be as included under other sections or as instructed by the Owner.

In addition to the requirements as specified, herein the contractor shall comply with the following requirements:-

- a) Use all necessary precautionary and protective measures required to maintain existing utilities, services and appurtenances that must be kept in operation. In particular, the contractor shall take adequate measures to prevent undermining of utilities and services presently in services.
- b) Protect existing or new utilities and services where required by the contractor's operations and/or as directed by the Owner. The contractor shall be responsible for bracing and supporting utilities and services to prevent settlement, displacement or damage.
- c) If any existing structure such as Pipe line, Road drain, Culvert, Towers, Light Posts, Underground Structures, Buildings, Tanks, Rail line etc are damaged during execution of project, same shall be reconstructed by EPC Contractor at his own cost at locations wherever approved by owner.



3.00.00 INSTRUCTION TO BIDDERS**3.01.00 Data to be furnished by Bidder at the time of Bid**

3.01.01 A write-up on survey to be undertaken indicating grid, intervals for taking spot levels, contour intervals, precision of surveying instruments proposed to be used, drawings that will be prepared, details to be covered by survey etc.

3.01.02 Detail general arrangements / architectural drawings of all buildings and structures showing dimensions, levels, plans, sections, elevations, loadings, materials proposed, types of framings, wall / cladding, floors, roofs, types of finishes etc.

3.01.03 Detail design criteria proposed to be adopted for each building, structures, foundations, facilities etc.

3.01.04 A write-up with schematic drawings of cooling water (CW) system indicating size and details of cooling tower, CW conduits, channels and forebay along with method of construction etc.

3.01.05 Organisation chart with no of Engineers to be furnished by EPC Contractor.

3.01.06 A write up on size and construction details of 275m high RCC chimney with steel twin flue and along with appurtenances shall be furnished.

3.01.07 Details of thermal insulation for ceiling and detail of false ceiling assumed with manufactures catalogue and areas where they propose.

3.01.08 All deviations from bid document shall be furnished by the Bidder.

3.01.09 List of equipment to be deployed and rates of labour assumed by the bidder and by other sub-contractors to be associated with him-in is to be furnished.

3.01.10 List of softwares proposed to be used against various areas for analysis, design, drafting and project monitoring along with their source and validation report for the softwares.

3.01.11 The list of documents indicated elsewhere of this section to be submitted by the contractor to the Owner for his approval and manner in which the same need to be submitted. No construction shall commence at site without obtaining approval from the Owner on these documents. Therefore, it is necessary that bar charts for buildings / structures / area wise shall be submitted for design / drawing activity indicating.

- a) A Level-1 pert chart showing the starting and completion date of all civil construction activities.
- b) A Level-2 pert chart showing the time required for preparation of design criteria, for approval of design criteria by Owner, time required for detailed design and drawing preparation and time required for approval of design and drawing by Owner. This part shall taken into account the construction schedule (Level – 1 part).



- c) A Level-3 pert chart for all civil works shall be submitted to Owner for monitoring of progress of works at site.

3.01.12 A detail note on quality plan both for design and construction activity proposed to be adopted for obtaining quality works.

3.01.13 List of all sub-contractors/Consultants/Architects that the bidder proposes to employ, in case the contract is awarded to him, indicating their addresses with telephone number, experience on similar jobs, name, qualification and experience of persons who shall be involved in the job on behalf of the contractor etc. shall be submitted to Owner. Only the sub-contractor approved by Owner shall be engaged by the contractor on the job.

3.01.14 Write-up on proposed storm water drainage system furnishing layout of the drains, types of drains and suggested disposal system.

3.01.15 A write-up on dewatering system proposed at the time of construction where deep construction such as for reclaim hoppers pit/track hopper pit, conveyor tunnels, CW forebay and pump house etc. are to be executed.

3.01.16 Tentative plot plan showing location of all buildings, structures, roads, site office, storage etc. with plan dimensions.

3.02.00 Inspection of Site by Bidder

3.02.01 Bidder shall inspect the site, examine and obtain all information required and satisfy himself regarding matters and things such as access to site, communications, transport, right of way, the type and number of equipment and facilities required for the work, availability of local labour, materials and their rates, local working conditions, weather, sub-soil conditions, natural drainage etc. Ignorance of the site conditions shall not be accepted by the Owner as basis for any claim for compensation or extension of time.

3.02.02 The submission of a bid by the Bidder will be considered as an evidence that such an examination was made and any later claims / disputes in regards to rates quoted shall not be entertained or considered by the Owner.

The contractor shall organize his own arrangement to transport his equipment, men and materials so as to match the construction schedule.

3.03.00 Construction Tools & Materials Supplied by Contractor

3.03.01 Contractor shall provide and maintain at the site necessary number and type of machinery and equipment including survey instruments in good working condition for proper setting out and timely completion of the various works cover under this specification. All arrangements for transporting the equipment to and from the site shall be done by the Contractor at his own expenses. No claim shall be entertained for mobilizing additional equipment and/or personnel to complete the work within the stipulated time.

3.03.02 Contractor shall provide all fuels and lubricants required for the operation and maintenance of construction machinery and equipment as well as his transport vehicles at his own cost.



- 3.03.03** The contractor shall ensure that the work shall proceed uninterrupted even in the event of power failure. As such, adequate number of diesel operated equipment shall be provided by the contractor at his own cost as an alternative arrangement, in case electrically operated equipment are proposed to be brought to site.
- 3.03.04** The contractor shall maintain sufficient stock of material so that there is no interruption of works at any point of time during the contract period & extended period if any.
- 3.03.05** All materials supplied by the contractor shall be original, new and of the best quality and shall conform to the given specifications. Approval in writing shall be obtained from the Owner before any alternative or equivalent material is used other than what is specifically mentioned in the drawings.
- 3.03.06** Contractor shall furnish manufacturer's test certificate for all the manufactured items supplied by him. Representative specimens of the material shall also be submitted to the Owner and shall be tested at a recognized testing laboratory at Contractor's cost in case Owner so desire.
- 3.03.07** The Owner reserves the right to test any construction material supplied by the Contractor in an established testing laboratory at Contractor's cost and to instruct the Contractor to remove all materials, which do not meet the specification requirements.
- 3.03.08** EPC Contractor to arrange third party quality inspection & audit of all construction material at site.
- 3.03.09** Contractor has to set-up a testing laboratory equipped with the following minimum apparatus, materials and competent trained staff required for carrying out field tests:
- Slump cone apparatus to measure slump.
 - Concrete cube testing machine with adequate number of moulds of (15cmx15cmx15cm) to measure compressive strength of concrete.
 - Vicat apparatus to find initial and final setting time of cement.
 - IS sieves with vibrating machine to determine fineness modulus of coarse and fine aggregate.
 - Abrasion & impact testing equipment for testing coarse aggregate and apparatus to determine flakiness index of aggregates.
 - Complete apparatus for the test of air content of concrete by pressure method as per IS:1199.
 - Density bottle to determine sand bulkage.
 - Hydrometer for testing pH value of water
 - Thermometer for checking temperature



- j) Apparatus for measuring proctor density, water content of compacted soil determining CBR values.
- k) All apparatus for determining dry density and water content of sand/soil and aggregates. Any other equipment felt appropriate by the Owner for measurement of paint thickness, testing of structural members, welding etc. The testing machines shall be recalibrated periodically and as directed by Owner to detect errors. The moulds for cubes shall be checked frequently and made to conform to specifications contained in IS-516.

3.04.00 Work Execution and Supervision

3.04.01 Contractor shall have at the site accredited and qualified engineers and foremen / supervisors with adequate number of years of experience in execution of similar works and also operators of machinery and equipment, for satisfactory progress and timely completion of the work.

3.04.02 Contractor's Engineer-in-charge of the work at site shall be capable of interpreting the specification and drawings and make adequate site decisions as and when required. He shall also take instructions from the Owner and be responsible for carrying out the instructions.

3.04.03 Contractor shall be fully responsible for the correctness and accuracy of the tests performed, results obtained / tabulated, interpretation of test results and recommendations made. The work shall be executed in a professional manner, with fully understanding of the importance of work for a project of this magnitude.

3.04.04 In the event of occurrence of any accidents at / near the site of the work or in connection with execution of the work, a report shall be made immediately to the Owner, giving full details of the accidents. He shall also report such accidents to all the competent authorities wherever such reports are required by them.

3.04.05 All temporary electrical installation shall be supervised by a qualified electrical supervisor of the Contractor.

3.04.06 Owner reserves the right to order in writing, from time to time, during the progress of the work, removal and re-execution of any work which in the opinion of the Owner, is not in accordance with the specification.

3.04.07 During inclement weather, rains etc. contractor shall suspend all works for such time as the Owner may direct and shall protect from damage all works already in progress or completed just then. All such temporary protective measures shall be at contractor's cost and any damage to works shall be made good by the contractor at his own expense.

3.04.08 Should the work be suspended by reasons of strikes / riots by Contractor's own employees or any other causes whatsoever save and except the force majeure condition, Contractor shall take all precautions necessary for the protection of works and make good, at his own expenses, any damage arising from any of these causes.

3.04.09 During the course of contractor's works, other works either by the Owner or by the Contractors or by both simultaneously will be in progress within the project area.



Contractor shall make his best effort to work in harmony with others in the best overall interest of the project and towards its speedy completion.

3.04.10 The Contractor shall be responsible for maintaining cleanliness of the site. The site shall be free of unwanted rubbish or filth, which is hazardous & detrimental to health, and affect safety of the work place. Proper watering to be done for dust suppression.

3.04.11 All material supplied shall conform to the specification. Entry of unwanted materials shall be prohibited.



4.00.00**SUBMISSION**

The following documents are to be submitted for the approval of the OWNER, prior to commencement of fabrication & erection / construction. All drawings shall be of standard sizes (metric system) and shall be made on AUTOCAD. Software used for design shall be validated and established ones like STAAD Pro, NISA, and ANSYS. Hard and soft copies of all the drawings / document shall be submitted to the OWNER. The following list is not exhaustive but indicative only.

- 1) The analysis of TG building, Mill building, ESP control room, Chlorination building, Switchyard control building, CW pump house, Raw water pump house, Ash water pump houses and Service building are to be done by developing 3D mathematical model adopting response spectra method of analysis as per IS: 1893-2002 & 2005, in addition to wind load analysis as per IS: 875 Part-3.
- 2) Preparation of design drawings with maximum detailing possible and developing all possible connection / joint details for all the buildings within the scope of civil / structural works.
- 3) General plant layout drawing with coordinates of roads, boundary wall and facilities, piping / cable corridors, pipe and cable trestles, provision of landscaping and green belt development, diversion roads and drains, equipment lay down areas etc.
- 4) Drawing showing underground facilities with coordinates like buried pipes, buried cables, trenches, ducts, sewer, drains, sumps pits, culverts, foundations etc.
- 5) Soil investigation report based on additional geo-technical investigation carried out by CONTRACTOR along with foundation recommendation for various buildings / structures / facilities.
- 6) Site Grading and storm water drainage study furnishing levels of various terraces arrangement and details of drains, culverts etc for storm water drainage system.
- 7) Study note on disposal of sewage and other effluent from the plant to satisfy the statutory requirement.
- 8) Design basis memorandum for all buildings, facilities, services and structures.
- 9) Drawing showing loading data at various levels for all buildings and structures.
- 10) Architectural floor plans, elevations, cross sections and perspective view in colour scheme of all buildings. For main plant building CONTRACTOR shall submit two different schemes along with a report elaborating the underlying philosophy of the proposed architectural concepts.
- 11) Design calculations and drawings for foundations / substructure and superstructure of all buildings including pump houses and other structures.



- 12) Design calculations including dynamic analysis and drawings for all foundations subjected to dynamic loads like foundations for TG, BFP, Mill, Fans (PA, FD, ID), Crushers etc.
- 13) Design calculations and drawings for all facilities and services like roads, culverts, bridges, pavings, road / rail crossings, drainage pump house (if required), drains, sewers, sewage pump house, sewage treatment plant, water supply, water tank, sumps, tunnels, trenches, ducts etc.
The contractor shall resubmit the design document and drawings within a maximum period of 3week from the date of receipt of comments by the contractor. Timely submission of design/drawings to the owner for review/approval is the sole responsibility of the contractor and postal or other delays as reasons for late/non submission shall not be entertained by the owner.
- 14) Drawings of all architectural works including finishing schedule, colour schemes (both internal and external), doors & windows, flooring and false ceiling shall be submitted by EPC Contractor to owner along with or before releasing of construction drawings.
- 15) Design calculations and drawings for plumbing and building drainage.
- 16) Design calculations and drawings for switchyard structures, transformer yard, etc.
- 17) Design calculations and drawings for structures pertaining to condenser Cooling water system.
- 18) Design calculation and drawings / document for the sea water system structures is covered within the scope of the Contractor.
- 19) Design calculations and drawings for civil structures / works associated with fuel oil handling system, ash handling system, coal handling system, Ash disposal system, etc.
- 20) All other designs, details / drawings or any other submissions as indicated elsewhere in this specification and as required by the Owner.
- 21) Details of corrosion protection measures for all structures.
- 22) All design and drawings for the natural draught Cooling towers.
- 23) All design and drawings for RCC Twin flue chimney.
- 24) All design calculations and drawings for foundation of Boiler and ESP area structures.
- 25) Preparation & Review of structural steel fabrication drawings is entirely of the responsibility of contractor & will not be approved by the owner. However, all fabrication drawings shall be submitted by the contractor for owner's reference & records prior to the commencement of fabrication.



- 26) Construction and erection procedure for all major structure with specific reference to main plant building, bunker structures, transfer towers, conveyor galleries, TG foundation and other machine foundations.
- 27) Write up on various statutory requirements and their compliance for various buildings and facilities.
- 28) Quality assurance and Quality Control procedures. Third party quality audit is required.
- 29) Copies of all reports on investigation and studies carried out by the CONTRACTOR as per the scope.
- 30) Soft copies of all design calculation and drawings shall be submitted for records after approval of the OWNER.
- 31) EPC Contractor to ensure that photograph of progress of work at site with required album is being submitted to owner once in every month in soft & hard copy.
- 32) EPC Contractor to ensure that Bar Bending Schedule & Bill of quantities has been furnished in various construction drawings.
- 33) All drawings are approval category except fabrication drawings which are information category.



5.00.00 GEO-TECHNICAL INVESTIGATION

5.01.00 M/s TANGEDCO, owner of this project has carried out preliminary soil investigation work in power block area i.e. from power house to chimney. Few boreholes and static cone penetration tests were performed. The borelog datas are enclosed in soil report attached along with specification.

As per the soil consultant's report for main plant building structures, TG foundation, boiler foundation, ESP foundation, mill building foundation, mill foundation, ID, FD & PA fan foundation, chimney foundation, conveyor gallery trestle foundation, junction tower foundation, crusher house foundation, stacker reclaimer foundation, fly ash silo foundation etc. pile foundation is recommended. Piles will be bored cast-in-situ.

5.02.00 Recommended safe load carrying capacity of Bored Piles will be as follows:

S.No.	Location	Length of Pile below E.G.L (m)	Length of Pile below Cut off (m)	Dia. of Pile (mm)	Safe Load Carrying Capacities (kN)		
					Vertical	Pull Out	*Horizontal
1	Main Plant (Ash Pond Area)	30	27	500	1000	260	50
2				600	1100	340	55
3				700	1500	400	75
4				800	1600	420	80
5				900	1900	500	95
6	Balance Plant Area	30	27	500	1000	260	50
7				600	1100	340	55
8				700	1500	400	75
9				800	1600	420	80
10				900	1900	500	95
11	Coal Conveyor Belt Area	30	27	500	1000	260	50
12				600	1100	340	55
13				700	1500	400	75
14				800	1600	420	80
15				900	1900	500	95

*Horizontal Capacity is considered 5% of Vertical Compression Capacity.

Note :- The Above load carrying capacities are valid only if ground improvement technique is adopted and post improvement properties are estimated to validate the improvement.



Above informations regarding soil investigation is only for the guidance of the bidders. Owner does not take any responsibility for the correctness and interpretation of the result. Any variation of the said information shall not constitute a valid reason in affecting the terms and condition of the bid. The bidder shall fully satisfy himself about the nature of soil expected to be encountered, including the type of foundation, bearing capacity, sub-soil water etc. prior to the submission of his bid.

5.03.00

Detailed geotechnical investigation shall be carried out by the Contractor on award of work. Based on the plot plan developed, the contractor shall identify proposed borehole locations and obtain the approval of Owner prior to commencing the investigation. In the power block bore holes shall be provided and spread judiciously to cover all major building as well as equipment foundations. Generally, a grid of 50 metres c/c both ways is recommended from Chimney to end of Switchyard. In the offsite areas also it shall be ensured that the bore holes are provided near all major structures. If the final geotechnical investigation report is found to be in variance with the assumptions / preliminary investigations made during bid stage, the contractor is not eligible for any extra claim or time extension.

Geotechnical investigation must be completed by EPC Contractor within 3 months from the date of handing over the site to the contractor & same should be wetted by IIT / Anna University before submission to the owner for review / approval.

5.04.00

Following field tests shall be conducted:

- Bore holes and standard penetration tests
- Static plate load tests
- Cyclic plate load test
- Cross hole shear velocity test
- Field permeability tests
- Field density tests
- Vane shear tests
- Static cone and dynamic cone penetration tests
- Electric resistivity tests
- Pressure meter tests
- Percolation test
- California bearing ratio tests

5.05.00

Bore holes shall be located to cover the entire area. All bore holes shall be sunk up to a depth of 40.0 m or 'N' value greater than 100 is obtained for 4 consecutive intervals.



5.06.00 Standard penetration test (SPT) and collection of undisturbed soil samples (UDS) shall be carried out alternatively at 1.0m intervals and at significant change of strata. The interval shall be increased to 1.5m below 5m depth of boring. UDS shall be replaced by SPT in cohesion less strata. Even in highly weathered / disintegrated rock, where core recovery is poor, SPT shall be conducted. The first SPT in any borehole shall be conducted at 1m depth.

5.07.00 Dynamic Cone Penetration Test (DCPT)

The test equipment and procedure shall meet the requirement of IS:4968 (Part II). The driving shall not be done for more than 30 cm at a time after which it shall be stopped for a minute or two. The tests shall be terminated when the blow counts exceed 35 for 100 mm. penetration when the cone is dry and 20 for 100 mm. penetration when the cone is penetrated by circulating slurry.

The results shall be reported in a suitable tabular form giving blow counts for every 30 cm. penetration supplemented by a graphical plot of blow count versus depth.

5.08.00 In rock strata, core recovery, Rock Quality Designation (RQD) and Room Mass Rating (RMR) shall be noted carefully for each run, immediately after cores are taken out of barrel.

5.09.00 During boring, the level at which ground water is struck shall be carefully noted. Ground water samples shall be collected for chemical analysis. Boring shall be carried out without the use of water or drilling mud up to the depth of ground water table.

5.10.00 Following laboratory tests shall be conducted: (preferably on Undisturbed soil samples and if UDS is not possible, on remoulded soil samples)

- i) Grain size analysis
 - a) Hydrometer analysis
 - b) Sieve analysis
- ii) Field density and moisture content.
- iii) Specific gravity.
- iv) Chemical analysis of soil and ground water including sulphates, chlorides, pH value etc.
- v) Chemical analysis of 2:1, water:soil extract of the samples giving SO_3 content.
- vi) Consistency Index: Liquid limit, plastic limit, plasticity index, shrinkage ratio.
- vii) Consolidation test giving all relevant information.
- viii) Swelling pressure and free swell index for expansive soils.
- ix) Unconfined compressive strength on undisturbed soil samples
- x) Direct shear test.



- xi) Triaxial compressive strength tests:
 - a) Unconsolidated undrained test
 - b) Consolidated undrained test
 - c) Consolidated drained test
- xii) Moisture density relations for Standard Proctor and Modified Proctor tests.
- xiii) Crushing strength, specific gravity, unit weight, water absorption test of rock specimens of NX size.
- xiv) Permeability test.

5.11.00

The Geotechnical investigation report shall necessarily include but not be limited to the following information:

- a) Recommended types of foundation.
- b) Allowable safe bearing capacities and settlement values in different strata for shallow foundations indicating relevant design criteria adopted, method of analysis adopted etc.
- c) Type of piles recommended with reasons for the same, length, diameter, allowable capacity (vertical lateral and pullout) of individual and groups of piles, negative skin friction, if any, and magnitude of estimated negative skin friction.
- d) Recommendations for values for modulus of sub grade reaction for foundation design by elastic method.
- e) Type of cement to be used for concrete substructures and in stone/ brick masonry foundations with reference to the chemical nature of subsoil and ground water.
- f) Recommendations regarding excavations (shallow & deep), embankment, safe sides slopes for excavation and embankment, dewatering, site drainage etc.
- g) Recommended soil properties such as density, specific gravity, cohesion, angle of internal friction etc. for design.

5.12.00

The Bidder shall carry out the topographical survey of the area within boundary.

The survey map should identify all topographical features such as but not limited to buildings, structures, burial grounds, wells, natural drains, roads, footpaths, depressions, underground sources, overhead transmissions lines, telephone lines or any other obstruction, trees and prominent vegetation. In addition, all wet land and marshy stretches shall also be identified. CONTRACTOR shall prepare contour map to a scale of 1 in 1000 in AUTOCAD. In addition, CONTRACTOR shall prepare LS and CS of the site at suitable intervals to adequately represent actual topography. For natural drains and water courses, sections at closer intervals shall be provided.

For further detail refer section 19.0 of part B.



5.13.00**SOIL IMPROVEMENT**

Since soil is having very low bearing capacity hence Implementation of ground improvement techniques and post improvement evaluation of sub surface properties upto 15.0 m below ground level is a part of foundation design, which is in the scope of contractor. Method of stabilization / improvement of soil to be used by contractor shall be got vetted by IIT / Anna University before submission to owner / owner's consultant for approval.



6.00.00 GENERAL REQUIREMENT OF BUILDING / STRUCTURES

The CONTRACTOR shall obtain and be conversant with all laws, by-laws and regulations of local and Statutory Bodies as applicable to the project. The architectural concept evolved should also take care of these requirements. The CONTRACTOR shall provide the drawings and documents for such statutory approvals.

EPC Contractor to ensure that all building layout has been finalized in such a way so that adequate space is available all-round the equipment for operation, repair & maintenance.

6.01.00 Roof Access

All roofs shall be provided with access thorough a staircase / cage ladder. Minimum 1000 mm wide access path shall be provided with tiles to approach equipment on roof.

6.02.00 Platform and Walkways

Platforms shall be provided to all major equipment, not directly accessible from - the floors, for maintenance. Platforms and connecting walkway shall have a minimum width of 750 mm. Platforms in front of the entry shall be atleast 900 mm wide. Platforms located close to each other shall be connected with walkways.

All steel platforms above grade level shall be constructed with kick plates at edge of the platform to prevent tools or materials from falling off. It shall consists of 8 mm thick Galvanised steel plate projecting 100 mm above the platform surface. Kick plate shall be painted with the same type of coating as the material to which it is attached.

Continuous walkway with GI Hand rail at least 750 mm (Wherever occurs) wide shall be provided along the crane girder level with handrails, on both side of the building. Approach to EOT crane shall be ensured by Cage ladder and staircase.

The plinth of powerhouse and its surrounding area is to be kept RL.10.0M. Top of finish of main approach road and internal road are to be kept be 250mm above surrounding Grade lvl. at RL.9.5M level.

Finished floor level boiler area / transformer yard area shall be kept 200mm lower than the finished floor level of power house building.

Finished ground floor level of all buildings and pump houses shall be minimum 500mm above the formation level / grade level.

6.03.00 Edge Protection

All-around floor openings an RCC kerb of 100 mm wide 150 mm high shall be provided. All concrete edges, where breakage of concrete corners expected shall be provided with angles of minimum size L 50x50x6 with lugs for edge protection e.g. all-round the cut-outs I openings in floor slab, edges of drains supporting grating covers, edges of. RCC cable I pipe trenches supporting covers, edges of manholes supporting covers and supporting edges of precast covers etc.

6.04.00 Anchor Bolts and Inserts Plates

Anchor bolts shall be designed for working stress, in tension and shear, for embedded length of the anchor bolts and pipe sleeves. Shear and crushing strength of concrete shall also be checked.

Insert plates shall be designed / checked for shear and bending moment. All lugs shall be checked for tension. Bond strength of concrete shall also be checked. Lugs using steel bars shall preferably be fillet welded to the plate to transfer full strength of the lug.

Insert plates required for the equipment supplied by BTG vendor will be supplied by BTG supplier and same shall be fixed in position under this contract and supply and fixing of all the balance inserts shall be under the scope of this contract.

6.05.00 Vertical Headroom

All accessible areas shall be provided with minimum clear headroom as follows, unless otherwise specified.

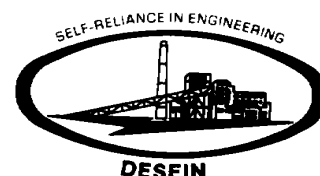
Finished floors to ceiling (buildings)	3000 mm
Doors, Walkways, Platforms, Stairs etc.	2100 mm
False ceiling of office areas	2400 mm
Above false ceiling	1000 mm
Safety cage for ladders	2500 mm
Access for fork lift trucks	2800 mm
Main roads / Railway crossings & crane access	7000 mm
Other plant roads and truck access	5000 mm
Cable & Pipe rack (except at road and rail crossings)	3000 mm

6.06.00 Anti Termite Treatment

Pre-constructional anti termite treatment shall be given to all vulnerable areas susceptible to termite attack and shall include column pits, wall trenches, foundations filling below the floors etc., as per IS: 6313 and other relevant Indian Standards.

6.07.00 STAIRS AND LADDERS

6.07.01 Steel Stairs



All steel staircases shall normally have minimum clear width (back to back of stringer) of 1000mm and maximum inclination with horizontal of 35.75°. However, in case of space restriction, minimum clear width upto 750mm and slope upto 45° may be provided. The vertical height between successive landings shall not exceed 5m. Channels (minimum MC200) shall be provided as stair stringers. Treads shall be minimum 250mm wide of grating, with suitable nosing, and spaced equally so as to restrict the rise to maximum 180mm.

6.07.02 Steel Ladders

Ladders shall be provided to platforms, walkways, instruments and equipments which do not require frequent access. Ladders shall preferably be vertical and its angle with vertical shall not exceed 5°. Ladders shall be of minimum 450mm clear width with 35x35x3 angle diameter MS rungs spaced at 300mm (maximum). Ladders shall be provided with a safety cage of minimum 750mm diameter clear when the top of ladder is more than 4.5m above the landing level. However safety cages shall start at 2.5m above the lower landing level.

6.07.03 RCC Stairs

All stairs shall have maximum riser of 180mm and a minimum tread of 250mm. However, for Administration & Control room building riser shall be limited to 150mm and tread width of 300mm. Minimum width of stairs shall be 1200mm generally. All stairs normally shall have not more than 15 risers in one flight. Aluminium angle nosing with minimum 50x25x3 angle shall be provided for edge protection of RCC stairs.

6.08.00 Handrails

Handrails shall be provided at appropriate places to ensure safety e.g. around all floors / roof openings, projections / balconies, walkways, platforms, steel stairs etc.

All handrails shall be of 32mm nominal bore MS pipes (medium class) as per IS:1161 galvanised using 750 gm/sq. m of zinc. Handrails for platforms, walkways and projections shall be a two-rail 450mm below the top rails. Handrail post spacing shall be limited to 1500mm as far as possible but can be proportioned to the length of the protected horizontal opening. In such a case spacing shall not exceed 1850mm center to center of posts. Handrails shall be shop fabricated for specific locations and field welded or bolted to the erected structural steel. For platforms at elevation more than 30m, three rail system with top rail at 1500mm shall be adopted.

For RCC stairs, handrails with 20mm square MS bar balustrade with suitable MS flat & Teakwood handrail shall be provided, unless specifically mentioned otherwise.

6.09.00 Expansion/Construction Joints

Expansion and construction joints shall be provided wherever required. All expansion and construction joints of water retaining structures in RCC shall be made watertight using PVC ribbed water stops with central bulb. However, kicker type (externally placed)



PVC water stops may be used for the base slabs and in other areas where it is required to facilitate concreting. The minimum thickness of PVC water stops shall be 6mm and minimum width 225mm. At other joints these shall be 150mm wide.

Two part polysulphide sealant conforming to IS:12118 shall be used for sealing of joints in contact with water. For other cases, bitumen sealing compound conforming to IS:1834 shall be used. Dura board HD100 or its equivalent shall be used as joint filler.

6.10.00**Brick / Stone Masonry And Parapet Wall**

All masonry works shall be designed in accordance with IS:1905, IS:2212, IS:4326, IS:2185 and other relevant IS codes as applicable. Structural design of load bearing and non load bearing walls constructed with fly ash bricks shall be in accordance with criteria specified by section-4 of National Building Code of India, Part-VI and codal provisions.

All walls shall be non-load bearing in filled panel walls. External walls of all buildings shall be atleast one brick thick. All internal walls shall be atleast one brick thick except for internal partition walls for office area, canteen, change room, first aid rooms and toilets, which may be half brick thick. RCC bands (transoms and mullions) shall be provided wherever necessary to curtail the unsupported length / width of the wall.

50mm thick DPC (1:1.5:3) with water proofing admixture shall be provided at plinth level before starting masonry works.

Bricks having minimum 75 kg/sq.cm compressive strength shall be used for non-load bearing superstructure brick work. Cement and sand mortar 1:5 for one brick thick wall and 1:4 for half brick thick wall shall be used. For half brick walls, RCC transoms and mullions shall be provided. Transoms shall be provided at lintel / door height. The spacing of mullions shall not exceed 2000 mm centre to centre. The size of transoms / mullions shall be minimum 115mm square with four numbers, 8 mm dia bars and 6mm stirrups 150 mm centers.

Type, thickness and height of external wall, facing the transformer yard to take care of fire accidents in transformer yard shall be according to the requirements of Tariff Advisory Committee.

Even where metal cladding is specified, for initial 3m height from the ground level, minimum one brick thick masonry wall shall be provided.

All upstands and parapet walls on roof shall be of RCC construction, minimum height of parapet walls shall be 750mm and thickness 125 mm.

6.11.00**Waterproofing Of Underground Structures**

All underground structures like water retaining structures, track hoppers, transfer house, conveyor tunnel & other deep underground structures etc., shall have plasticizer cum water proofing cement additives conforming to IS:9103. In addition, limits on permeability as given in IS:2545 shall also be met with.



All water retaining structures shall be hydro tested as per IS 3370 for leakage & in case leakage is noticed pressure grouting or any other approved method shall be used to rectify the same.

The concrete surface of these structures in contact with soil shall be provided with water proofing treatment as stated below.

- i) For base slab, a layer of PCC 1:4:8 shall be laid. Over PCC layer, 25mm thk CM 1:3 mixed with approved water proofing compound at the rate specified by the manufacturer shall be applied. Over this two coats of acrylic polymer modified cement based flexible water proofing membrane of approved make laid as per manufacture's and instruction. Over water proofing membrane, 25mm thk CM 1:3 mixed with approved water proofing compound at the rate specified by the manufacturer shall be provided over which protective layer of 15mm thk kota / cuddapa or equivalent stone with joints sealed with C1:3 shall be laid. Over this stone layer, 25mm thk CM 1:3 mixed with approved water proofing compound at the rate specified by the manufacturer shall be applied before RCC base slab shall be casted.
- ii) For side walls, two coats of acrylic polymer modified cement based flexible water proofing membrane of approved make laid as per manufacture's specification and instruction shall be applied to the sides of RCC walls. 25mm thk CM 1:3 mixed with approved water proofing compound at the rate specified by the manufacturer shall be applied over it. Protective layer of 15mm thk kota / ciddapa or equivalent stone with joints sealed with CM 1:3 shall be laid. Finally 25mm thk CM 1:3 mixed with approved water proofing compound at the rate specified by the manufacturer shall be applied.

For all other foundation & pedestal work below ground coming in contact with soil, bituminous painting of grade 85/25 conforming IS 702@ 1.7 kg/ sqm.(min)

6.12.00

Miscellaneous Requirements

Doors and windows on external walls of buildings shall be provided with RCC sunshade over the openings With 300 mm, projection on either side of the opening. Projection on sunshade from the wall shall be minimum 450 mm over window openings and 750 mm over door openings.

Doors and windows on the external walls of buildings with metal cladding shall be fixed by creating recesses in the cladding system.

No cable trenches shall be provided in TG hall, Boiler/ESP area, fuel oil pump house, Ash pump houses etc.

Duct banks consisting of MS/PVC conduits for cables shall be provided with proper sealing arrangement consisting of fire retardant sealing compound.

All floor openings for cables below electrical panel shall be sealed with fire sealing compound after cables are laid.



All openings in external walls provided for pipes, cables, duct etc. shall be effectively sealed to prevent water seepage, after the routing of the services are completed.

Natural lighting & Ventilation.

The area of windows shall be a minimum 15 % of the floor area to ensure adequate natural lighting. Fans shall be provided in general office areas as per standard norms. Sewerage system shall be provided with adequate ventilation for the pipe work as well as manhole.

6.13.00

Statutory Requirements

All the applicable statutory rules pertaining to Indian Factories act, Factory rules of State Government, Fire safety rules of Tariff Advisory committee, Water act of Pollution Control boards, Explosives act, Environmental requirements etc., and stipulations of other relevant statutory authorities shall be taken into consideration at the time of design and construction.

Provisions of safety, health and welfare according to Factories act shall be complied with at design stage. These shall include provision of continuous walkway (minimum 750 mm wide) along crane-girder at crane girder level on both sides of the building, comfortable approach to **EOT** crane. cabin, railings, fire escape locker room for workmen, pantry, toilets, rest rooms etc.

Adequate no. of fire escapes shall be provided in a building. Fire proof doors, no. of staircase, fire separation walls, lath plastering on structural steel member (in fire prone areas) shall be made according to the recommendation of TAC.

Obtaining statutory approval clearance/ license from relevant agency will be within the scope of EPC Contractor.

For fire safety requirements of buildings IS: 1641 and IS: 1642 shall be followed in addition to TAC requirements. All masonry firewalls shall be minimum 345 thick and RCC firewall shall be minimum 200 mm thick.



7.00.00 MATERIAL**7.01.00 Structural Steel**

Steel will conform to Grade-A of IS:2062 (latest) for rolled steel members or plates upto 20 mm thickness. For plates above 20 mm thickness and welded construction steel conforming to Grade-B (killed and normalized) of IS: 2062(latest) shall be used except for crane girders where Grade-C (IS: 2062) steel shall be used. Steel shall be procured from SAIL or any other approved main producers.

Chequered plate shall conform to IS: 3502 (latest) and minimum thickness of chequered plate for floorings, covers etc shall be 8 mm O/P.

Complete hopper including vertical and conical portion of bunker in mill building shall be provided with lining of atleast 6mm thick SS plate grade SS 316L .

The electrodes classification as per AWS shall be as follows:-

- a) For welding of stainless steel to stainless steel : E308L
- b) For welding of stainless steel to mild steel : E309

7.02.00 Cement

Ordinary Portland Cement (OPC) shall be used for all structures except for foundations, under ground structures & structures coming in contact with sea water where in sulphate resistant cement with C3A content limited to 5% to 8% shall be used. Grade of cement shall be 43 conforming to IS: 8113.

7.03.00 Reinforcement

The reinforcement used shall be cold worked steel high strength deformed bars of grade Fe 415 / Fe 500 conforming to IS:1786 – latest.

Fusion bonded epoxy coated reinforcement steel with coating conforming to IS:13620 shall be used for the complete project.

Intermixing of different grades of rebars or rebars of different material composition in same structure shall not be allowed.

Welding of reinforcement shall not be carried out without the permission of the Owner.

Projecting reinforcement or dowel bars for future connection of the structural works shall be protected by cement paint, if they are to be left exposed for a long time.



8.00.00 LOADING**8.01.00 Dead Loads**

Dead loads shall include the weight of structure complete with finishes, fixtures & partitions and shall be taken as per IS: 875 (Part-I)

8.02.00 Imposed Loads

Imposed loads in different areas shall include live loads, erection, operation and maintenance loads. Equipment loads (which constitute all loads of equipment to be supported on the building frame) are not included in the imposed loads furnished below and shall be considered in addition to imposed loads.

For consideration of imposed loads on structures, IS:875 (Part-2) "Code of practice for design loads (other than earthquake) for buildings & structure" shall be followed. The following minimum imposed loads as indicated for some of the important areas shall however be considered for the design. If actual expected load is more than the specified minimum load, then actual load is to be considered

Sl.No.	Location	Imposed Loads (T/Sq.m)
A)	Turbine Building	
i)	Ground floor (general)	2.50
ii)	Ground floor (Heavy equipment storage area)	5.00
iii)	Mezzanine Floor	1.50
iv)	Operating Floor	
	a) Rotor Removal area Rotor removal area beams shall also be checked for ½ the rotor load at the center of beam	5.00
	b) Equipment Laydown area	3.00
v)	Gratings, chequered floors, walkways, platforms, stairs etc.	0.75
vi)	Roof (Where no equipment are located and non-accessible)	0.15
	Roof (where equipment are located and accessible)	0.50
B)	Deaerator and Heater Bay	
i)	HP / LP heater floor	2.0



	ii)	Deaerator floor	1.5
	iii)	Cable gallery	0.50 (In addition to this actual cable load shall be considered)
	iv)	MCC & Control Building floors	1.00
	v)	AHU room battery room, air washer room	1.5
C)	Mill & Bunker Bay		
	i)	Ground Floor	2.5
	ii)	Feeder Floor	2.00
	iii)	Tripper Floor	2.00 Beams however shall be checked for feeder / tripper concentrated load
	iv)	Roof	0.15 (where no equipment are located) 0.50 (where equipment are located)
	v)	Dust Load	0.1
D)	Pump Houses operating floor		1.50
E)	Underground Structures such as Channels, Sumps, Underground Pump House, Tanks, Trenches, Reservoirs, Cooling Water Ducts etc. In addition to earth pressure and ground water pressure, the surcharge load of 2T/sqm. shall also be considered for design of all underground structures.		
F)	Road Culverts / Bridges and its allied structures including RCC Pipe Crossings & Road Crossing of Trenches. Design for class 'AA' loading (wheeled & tracked both) and checked for class 'A' loading as per IRC Standard.		
G)	Covers for Channels / trenches		Trench cover at entry to building and road



		crossings shall be designed for live load of 100 kN at centre. Trench cover at other location shall be designed with a surcharge of 100 kN/m ² or a concentrated load of 100 kN at center which is worst.
H)	Railway Supporting Structures, Rail Culverts	As per railway 'Bridge Rules'
I)	Boiler / ESP Support Structures	0.50
J)	General (Unless Specified Otherwise)	
i)	Stairs, Landings and Balconies	0.50
ii)	Toilets	0.20
iii)	Chequered plates, grating floors, etc.	0.50
iv)	RCC floors (General)	0.50
v)	a) Flat roofs (where no equipment are located)	0.15
	b) Flat Roofs (where equipment are located)	0.50
vi)	Inclined Roofs	As per IS:875 (Part-II)
vii)	Walkways (General).	0.50 T/sq.m or a concentrated load of 0.3 T/m which ever is greater
viii)	Walkways of conveyor galleries.	0.50
ix)	Floor of control room of switchyard control building	1.00
x)	Cable and pipe trestles	0.20 for walkway and in addition, friction loads as applicable.



NOTE :

1. Additional load for cable, piping / ducting, shall be considered as applicable.
2. Hung load shall be based on minimum loading equivalents of 1 kN/ m² for piping and 0.5 kN/m² for electrical ventilation and air conditioning.
3. Loading resulting from concentrations of facilities in specific areas shall be substituted where listed base loadings are excluded
4. Cantilever loads of not less than 2000 kg/metre at a distance of 1200 mm from the external face or the columns, on both sides of the ESP, for cable trays and walkways.
5. Cantilever loads of not less than 500 kg/metre at a distance of 1200 mm from the external face or the columns, on both sides of the boiler, for cable trays and walkways.
6. All ESP hoppers filled up with ash upto the top of the hoppers or the bottom of the electrodes (whichever is more) using a bulk density of not less than 1600 kg/cum for the ash, along with additional ash build-up from the end of the third field upto the inlet duct bottom level at a natural repose angle (not less than 30 degree to horizontal in any case)
7. Boiler supporting structures shall be so configured that the temperature of steel does not exceed 60 deg.C unless specified otherwise. Brackets shall be provided on both sides of the outermost row of columns of both the boiler and ESP for supporting cable trays and walkways, at a height not exceeding 10.0m. The exact levels shall however, be decided during detail engineering. Each ESP hopper shall be supported at four corners by providing four columns from the ground.
8. In design of boiler /ESP support structures dynamic piping loads need not be considered acting simultaneously with wind or seismic loads. Increase in permissible stresses shall be allowed in load combinations where dynamic piping loads are considered and shall be as permitted under seismic load conditions.

8.03.00 Equipment, Piping and Associated Loads

Equipment loads shall be considered over and above the imposed loads. Equipment loads shall be considered as given by equipment supplier.

8.04.00 Crane Loads

For crane loads, an impact factor of 25% and lateral crane surge of 10% (of lifted weight + trolley weight) shall be considered in the analysis of frame according to the provisions of IS:875. The longitudinal crane surge shall be 5% of the static wheel load. Longitudinal surge and lateral surge shall not be considered to act simultaneously.

8.05.00 Seismic Load

The project site is in Zone – III as per IS: 1893/Part I-2003 & IS:1893 – Part II – 2005.

Zone factor $Z = 0.16$

Importance factor $= 1.75$

Response reduction factor = 5.0 (for MRF) AND 3.0 (for OMRF)

8.06.00 Wind Load

Basic wind speed at project site is 50 m/sec. as per IS:875 -1987 Part-3

Probability factor, (k1 risk coefficient), terrain, height and structure size factor, k2 and topography factor, k3 shall be as per IS:875.

8.07.00 Damping in Structures

The damping factor to be adopted shall not be more than as indicated below:

Type of Structure	Wind Load	Seismic Load
a) Welded steel structure	1%	2%
b) Bolted steel structure	2%	2%
c) RCC structure	1.6%	5%

8.08.00 Temperature Load

For temperature loading, the total temperature variation shall be considered as 2/3 of the average maximum annual variation in temperature. The average maximum annual variation in temperature for this purpose shall be taken as the difference between the mean of the daily minimum ambient temperature during the coldest month of the year and mean of daily maximum ambient temperature during the hottest month of the year. The structure shall be designed to withstand stresses due to 50% of the total temperature variation.

Suitable expansion joints shall be provided in the longitudinal direction wherever necessary with provision of twin columns. The maximum distance of the expansion joint shall be as per the provisions of IS:800 and IS:456 for steel and concrete structures respectively.

Systems and system component design criteria which require ambient temperature extremes shall use the range from 22C to 40C for dry-bulb temperatures.

8.09.00 The following densities shall be considered for design of coal bunkers:

- (a) For volume calculations : 800 kg/cum
- (b) For structural design : 1200 kg/cum

The coal bunker will be designed as per criteria outlined in IS:9178 (Part I&II)



8.10.00 Loading for Switch Yard

The loading for the design of switchyard structures shall be as per IS:802 Part 1/Sec 1:1995 (latest edition). Following loads shall be considered:

- (a) Dead load due to equipment and dead load due to structure
- (b) Wind load on towers, conductors, ground wires and insulator strings calculated as per clause 8 and 9 of IS:802
- (c) Temperature effects consisting of effect of temperature variation and sag tension as per clause 10 of IS:802
- (d) Climatic loads as per clause 11.2 of IS:802
- (e) Anti concading loads as per clause 11.3.1 of IS:802
- (f) Torsional and longitudinal loads caused by breakage of conductor as per clause 11.3.2 and 16 of IS:802
- (g) Construction and maintenance loads
- (h) Seismic loads as per IS:1893
- (i) Short circuit forces including “snap effect” in the case of bundled conductors.

8.10.01 Switchyard structure shall be designed for the worst combination of above loads. The factor of safety for design of members shall be considered as 2 for normal and broken wire conditions and 1.5 for combined short circuit and broken wire conditions. Short circuit forces and wind forces shall not be considered simultaneously.

8.10.02 Foundation shall be designed for a factor of safety 2.2 for normal and broken wire condition and 1.65 for combined short circuit and broken wire conditions. Design of foundation shall be carried out as per IS:4091.



9.00.00 STEEL STRUCTURE**9.01.00 Framing**

All steel framed structures shall be either “rigid frame” or “simple space frame” or a combination of two.

Lateral forces shall be resisted by stiff jointed moment connections in rigid frame design. The column bases shall generally be fixed to concrete foundation pedestal by providing moment resistant base detail.

The power house building design shall be a combination of rigid frame in transverse direction and simple frame in longitudinal direction.

If RCC floor / roof is assumed to act as diaphragm transmitting lateral loads to braced bays, it shall be provided with shear connectors. However, whenever large / more number of cut-outs are provided in the floor slab, horizontal floor bracings shall be provided below slab to transfer horizontal force to columns without considering diaphragm action from slab.

Floors for vibrating machines of all kind together with supporting framework shall be adequately braced in both horizontal and vertical planes. Floors or structure supporting mechanical equipment shall be designed to minimise vibration, avoid resonance and maintain alignment and level.

9.02.00 Design Concepts

Individual members of the frame shall be designed for the worst combination of forces such as bending moment, axial force, shear force, torsion, etc. Criticality of erection / maintenance loads shall also be checked separately in combination with other simultaneously occurring loads for possible design loadings.

The different load combinations shall be taken as per IS:875 (Part-5) and other relevant IS Codes.

- a. Wind and seismic forces shall not be considered to act simultaneously.
- b. For the design of main plant structures during seismic condition, the deaerator feed water tank shall be considered full upto operating level. However, for other load combinations, deaerator feed water tank in flooded condition shall also be considered.
- c. In the analysis of main plant building & bunker building, the stresses arising due to temperature shall be considered.
- d. ‘Lifted load’ of crane shall not be considered during seismic condition.
- e. In case two cranes are provided and tandem operation is not envisaged, the load shall be taken as one crane fully loaded and second crane without lifted load but standing idle adjacent to first crane.



- f. In case two cranes are provided and tandem operation is envisaged for some bays, then the load shall be taken as both the cranes fully loaded and standing side for these bays. For other bays, load shall be taken as one crane fully loaded and second crane without lifted load but standing idle adjacent to first crane.
- g. Permissible stresses for different load combinations shall be taken as per relevant IS codes.
- h. For the design of pipe / cable supporting structure, the soil weight shall be considered as backfilled upto grade level for the condition of pipe running full / cables in position.
- i. Frictional force between the pipes and supporting structure in longitudinal direction need not be considered along with seismic or wind forces.

The design of steel structures shall be done by working stress method. Design shall be as per provision of IS:800 (latest) and other relevant IS standards. For design of coal bins and loading hopper IS:9178 (Part I to III) shall be followed.

Roof decking sheets shall be designed as per IS:801 to carry the self load, dead load due to RCC slab and finishes and imposed load. The deflection of metal deck shall be limited as per BS:5950. In case composite action is considered in the design, suitable shear studs shall be provided as per BS: 5950.

Permissible stresses for different members shall be allowed to exceed upto 33.33% only under normal loads along with wind and seismic conditions. The members which are designed primarily to resist wind load such as bracing members, no increase in permissible stress will be permitted. However, permissible stresses in bolts and welds shall be allowed to exceed up to 25 % only under wind and seismic conditions.

For design which requires the use of the minimum column load (such as, uplift on anchor bolts, column axial tension, etc.) the following criteria shall be used in determining minimum load: Use 90% of the column dead load, No live load is used, Uplift forces from vertical bracing are included where applicable and Wind uplift on the roof is included where applicable.

Base plates shall be placed on foundation pedestal with grouting. For large base plates necessary grout holes shall be provided. All anchor bolts for fastening steel columns on foundation shall be embedded in foundation during concreting itself. No anchor pockets in foundation shall be allowed. Design of base plates shall be based on design pressure on foundation which shall not exceed the following:

Pedestal in concrete grade M20	5.0 N / sq.mm
Pedestal in concrete grade M25	6.25 N / sq.mm
Pedestal in concrete grade M30	7.5 N / sq.mm

The total horizontal shear force at the base of column is transferred to the column pedestals through friction between the base plate and the grout. A coefficient of



friction of 0.30 shall be used in conjunction with the minimum column load as defined above. If the horizontal shear force exceeds the frictional resistance force or if the column is subjected to a net uplift load, the total force shall then be transmitted through shear bars / shear keys welded to the base plate. Anchor bolts are not assumed to resist any horizontal shear force. Necessary recesses shall be kept in the foundation concrete for shear lugs.

Welding shall be used for fabrication and erection. Site connections shall generally be with welding. However, high Strength Structural (HSS) bolts shall be used for all important connections to be decided during detail engineering stage. In few cases, for shear connections or removable beam connections, bolted joints with MS bolts may be adopted. For HSS bolt connection, IS:4000, IS:3757, IS:6623 and IS:6649 shall be followed. IS:814, IS:816, IS:1024, IS:4353 and IS:9595 shall be followed for welding of structures.

Trestles supporting coal conveyor galleries shall be so proportioned that the transverse deflection of trestles due to wind / seismic load shall not exceed trestle height / 1000 as stipulated in IS:11592.

In the case of galleries, temperature expansion joint shall be introduced at intervals less than 90 m to divide the galleries into temperature block. In each block at least one number four legged rigid support guaranteeing stability of structure in the longitudinal direction shall be provided. This shall also take care of all longitudinal forces in the given block. Effect of wind load acting on 2-legged trestle shall also be considered while designing the 4-legged trestle.

Base plates for trestles shall be designed as gusseted bases with shear lugs to transfer horizontal forces. Anchor bolts shall be designed only for uplift forces.

Anchor fasteners shall not be used for supporting equipment imparting dynamic forces.

Pedestals supporting gravity take-up shall be designed to resist 100% impact.

For calculation of coal load on moving conveyor, a multiplication factor of 1.6 shall be used to take care of inertia force.

- a) Conveyor gallery structure & trestles shall be designed considering both conveyors operating simultaneously.
 1. Dynamic analysis of conveyor galleries and conveyor supporting system shall be carried out for spans greater than 25m.
 2. All structures close to railway line shall have clearances conforming to Railway norms.

Transverse coal pressure on Bunker/ Silo / Hopper walls shall be calculated using Walker's theory and IS:9178. The Coal Bunker / Silo / Hopper shall be designed for the following conditions.



- i) The Bunker / Silo / Hopper is full up to its full capacity with top surface nearly horizontal.
- ii) The Bunker / Silo / Hopper is partially empty with the top surface of coal at an angle of repose of 37 degrees.

Design pressure on coal bunker / hopper walls shall take into account all possible flow regimes (core flow, mass flow, etc.), and different aeration regimes (radial, diametrical, radial and core, impulsive etc.)

9.03.00 Permissible Deflections

The permissible deflections of various steel members under normal loading conditions shall be as specified below. For calculation of deflections in structures and individual members dynamic effects shall not be considered, unless specified otherwise. Also, no increase in deflection limits shall be allowed when wind or seismic load are acting concurrent with normal loading conditions.

9.04.00 Vertical Deflection

- 9.04.01**
- a) For beams supporting dynamic equipment : Span / 500
 - b) For beams supporting floors / masonry : Span / 325
 - c) For beams supporting pipes : Span / 400
 - d) For roofing and cladding components : Span / 325
 - e) For gratings and chequered plates : Span / 200 subject to a maximum of 6 mm
 - f) Coal/ Ash conveyor gallery bridges : Span / 450

9.04.02 For crane gantries or any member subjected to working loads, the maximum deflection under dead load and live load excluding impact shall not exceed the following values:

- a) For manually operated cranes & monorails : Span / 500
- b) For electric overhead cranes
 - i) Up to 50 t capacity : Span / 750
 - ii) Over 50 t capacity : Span / 1000

9.05.00 Horizontal deflections

The permissible horizontal deflections shall be as per following unless specified otherwise:

- a) Single storey building (without crane load) : Height / 325
- b) Multistoried building (without crane load) : Height / 500



c)	Pipe rack columns	:	Height / 200
d)	Crane gantry girder due to surge	:	Height/200
e)	Building main columns at crane rail level due to action of crane surge load only	:	Height / 2500 limited to maximum of 10 mm
f)	Open gantry columns at crane rail level due to action of crane surge load only	:	Height/4000 limited to maximum of 10 mm
g)	Open structures	:	Span / 2000 Limited to Maximum of 15mm
h)	Coal handling trestles	:	Height / 1000

9.06.00 Provisions of IS: 800 and relevant IS Code shall be followed for limiting deflections of structural elements not listed above.

9.07.00 MINIMUM THICKNESS OF STRUCTURAL STEEL ELEMENTS

The minimum thickness of various components of a structure and hot rolled sections shall be as follows. The minimum thickness of rolled shapes shall mean flange thickness regardless of web thickness. Structural steel members exposed to significantly corrosive environment shall be increased suitably in thickness or suitably protected otherwise as per good practice and sound engineering judgement in each instance.

a)	Trusses, purlins, girts and bracing	6mm
b)	Columns and beams	8mm
c)	Gussets	8mm
d)	Stiffeners	8mm
e)	Base plates	10mm & above
e)	Chequered plates	8 mm o/p & above
f)	Grating flats	5 mm

Minimum thickness of structural members other than gratings directly exposed to weather and inaccessible for painting and maintenance shall be 8 mm.

9.08.00 Minimum Sizes

The flange width of purlins supporting light weight concrete slab shall not be less than 65 mm and for those supporting roof sheeting and wall cladding it shall not be less than 50 mm. Width of steel rolled section connected to other member shall be at least 50 mm. The depth of beams for platform of all structures shall not be less than 125 mm.



9.09.00**Slenderness and Depth Ratio**

The slenderness ratio of main members in tension, compression or bending shall be in accordance with IS:800.

The following limiting ratios of depth to span shall be considered as a general guide.

- | | |
|--|--------|
| a) Truss | 1 / 10 |
| b) Rolled beams and girders for ordinary floors and rafters | 1 / 24 |
| c) Supporting floor beams for vibrating machinery /equipment | 1 / 15 |
| d) Roof purlins and girts | 1 / 45 |
| e) Gable columns | 1 / 30 |

9.10.00**Joints / Connections in Steel Structures:****9.10.01**

Steel structures shall be detailed and connection and joints provided as per the provisions of IS:800, IS:9595, IS:1367, IS:9178 and IS:816 and as per following requirements:

- a. Connection of vertical bracings with connection members and diagonals of truss members shall be designed for full tensile capacity of the bracings unless actual loads are indicated on the drawings.
- b. Size of fillet weld for flange to web connection for built up section shall be as follows:
 - i) For box section weld size shall be designed for 60% of full shear capacity or actual shear whichever is more. Where fillet weld is not possible, full penetration but weld shall be provided.
 - ii) For built-up I section, weld size shall be designed for 80% of full shear capacity or actual shear, (if indicated in drawings) whichever is more. However, weld size shall not be less 0.5 times the web thickness. Weld shall be double fillet.
 - iii) All welds shall be continuous unless otherwise specifically approved. The minimum size of the fillet weld shall be 6 mm.
- c. Shear connections shall be designed for 70% of section strength for rolled sections and 80% of section strength of built-up section or rolled section with cover plates. However, if actual shear load is more than above, the connection shall be designed for actual load.
- d. Moment connection between beam and column shall be designed for 100% of moment capacity of the beam section. This can be achieved either by direct butt welding of the top flange of beam with column flange or by providing top moment plate with suitable notch for additional weld length.
- e. All bolts and nuts shall have property class compatible to each other. For bolts carrying dynamic or fluctuating loads and those in direct tension



shall be provided with an additional double coil helical spring washer conforming to IS:6755. The threaded portion of the bolt shall project through the nut at least by one thread.

- f) Where a steel beam or member is to be connected on RCC structure, it shall be connected using an insert plate and preferably through shear connection.
- g) All butt welds shall be full penetration butt welds.
- h) The connection between top flange and web of crane girder shall be full penetration butt weld. Bottom flange, connection with web can be fillet weld or butt weld as directed by Purchaser. Bearing edges of crane girders shall be machine finished.
- i) Connection of base plate and associated stiffeners with the columns shall be designed considering the total load transferred through welds. However, minimum weld size (double fillet) shall not be less than 0.6 times the thickness of stiffeners.
- j) Splicing: All work shall be full strength. Field splicing shall be done with web and flange cover plates for full strength. In exceptional cases, the field splicing shall be designed for 50% of load carried by the cover plates and remaining 50% load through full penetration butt weld. Shop splicing for all sections other than rolled shall be carried out by full penetration butt welds with no cover plates. Splicing for all rolled sections shall be carried out using web and flange cover plates.

9.10.02 All bolted connections shall have bolts of minimum 16 mm dia. The connections of stairs and hand railing shall be made with 20 mm diameter threaded fasteners conforming to IS:1363. Erection bolts shall be black bolts of minimum 12 mm dia.

9.10.03 Efficiency of site welds to be considered shall be as follows:

- a) Butt weld above 25 m from ground --- 50%
- b) Others --- 80%

9.11.00 Specification for Painting of Steel Structures / Material

Refer Volume-VI, Part-B



10.00.00 REINFORCED CONCRETE STRUCTURES AND FOUNDATIONS**10.01.00 General**

All structures, building foundation, machines / equipment foundation, water retaining structure, trenches, pits, etc., shall be designed as per relevant IS codes in general. Construction in general shall follow provisions of IS :456 and IS:3370 for normal and water retaining structures respectively

10.02.00 Design Methodology

- a) The design and construction of RCC structures shall be carried out as per IS: 456-2000. Working stress method shall be adopted for the design wherever specifically mentioned in this specification.
- b) For all concrete structures, ductile detailing has to be done as per IS: 13920 and IS: 4326.
- c) For reinforcement detailing, IS: 5525 and SP34 shall be followed.
- d) Two layers of reinforcement (on both inner and outer faces) shall be provided for RCC wall sections having thickness 150mm or more.

10.03.00 Foundation and Underground structures

- a) Foundation system to be adopted for structures shall be based on loading arrangement , load intensity and soil strata.
- b) All RCC liquid retaining / conveying structure / slurry pump house / underground structures, such as conveyor tunnel, reclaimer hooper pit, etc., shall be designed in accordance with IS:3370 (Part 1 to 4) . For under ground structures not in contact with liquid on inside face may be designed as cracked sections with crack width limited to 0.1mm for both faces.
- c) All liquid retaining / carrying structures shall be tested for water tightness as per the provisions of IS:3370 and IS:6494 and in case of leakage, the same shall be rectified by chemical injection grouting through nozzles.
- d) Earth pressure for all underground structures shall be calculated using coefficient of earth pressure at rest. Co-efficient of passive earth pressure shall be used only in design of shear keys for stability against sliding.
- e) In all liquid retaining structures , pvc water bar shall be provided at each construction / expansion joint. No pressure relieving devices shall be permitted in underground structures except forebay.
- f) For design of all underground structures, including CW ducts, pump house and forebay etc. ground water table shall be assumed at the finished ground level unless specified otherwise. In addition to ground water pressure, minimum



surcharge load of 2 T/M² shall also be considered for design of all underground structures.

- g) All building sub-structures including pump houses shall be checked for sliding and overturning stability during both construction and operating conditions for various combinations of loads. Factor of safety for these cases shall be as per IS:456 and other relevant IS codes. However, following minimum factor of safety shall be followed,
 - a) Factor of safety against overturning due to wind, seismic or other lateral load shall be 1.5 minimum.
 - b) Factor of safety against sliding shall be 1.5 minimum.
 - c) Factor of safety against uplift due to hydrostatic forces shall be 1.2 and due to any other loads shall be 1.5.
- h) In cases where dead load provides the restoring forces, only 0.9 times characteristic dead load shall be considered. Imposed loads shall not be considered as restoring force.

10.04.00 Grouting

- i) Non-shrink flowable grout shall be used for under pinning work below base plate of columns. Non-shrink cum plasticiser admixture shall be added in the grout. For grouting of base of machine foundation high strength flowable ready mixed non-shrink grout shall be used.
- ii) Type and grade of grouting for structural columns and equipment bases shall be indicated. Crushing strength of the grout shall generally be one grade higher than the base concrete. Minimum grade of grout shall be M30.
- iii) Nominal thickness of grouting shall be at least 50 mm for building columns and pedestals of major equipment. For secondary posts, stair and ladder base, etc. grouting shall not be less than 25 mm thick.

10.05.00 Edge Distance For Bolts

Minimum distance from the center line of foundation / anchor bolt to edge of pedestal shall be the maximum of the following:

- i) Clear distance from the edge of base plate / base frames to the outer edge of the pedestal shall be minimum 50 mm.
- ii) Clear distance from the face of pocket to the outer edge of pedestal shall be 75 mm.
- iii) Clear distance from the face of pocket to the outer edge of pedestal shall be 75 mm.



10.06.00 Loading conditions for underground structures

Following loading conditions shall be considered in addition to the loading from super structure for the design of substructure of pump house, channels, sumps, tanks, trenches and other underground structures containing liquid.

- a. Water pressure from inside and no outside pressure, like earth pressure, ground water & surcharge pressure (applicable only to structures which are liable to be filled up with water or any other liquid.)
- b. Earth pressure, surcharge pressure and ground water pressure from outside and no water pressure from inside.
- c. Design shall also be checked against buoyancy due to the ground water during construction as well as after construction stages. Minimum factor of safety against buoyancy shall be ensured considering empty condition inside and ignoring the superimposed loadings. Provision of pressure relief valves / flap valves, etc. may be considered only in forebay of CW Pump House to counter the buoyancy. When pressure relief valves are used, 60% of the hydrostatic pressure shall be considered for design of the base slab.
- d. Base slab of the pump houses shall also be designed for the condition of different combination of pump sump being empty during maintenance stages with maximum ground water level.
- e. Intermediate dividing pier of pump sumps and partition wall (if applicable) in channel shall be designed considering water on one side only and other side being empty for maintenance.
- f. All pump houses and other substructures (wherever applicable) shall be checked for stability against sliding and overturning during construction as well as operating conditions for various combinations of loads.

10.07.00 Machine Foundations

The design of equipment foundation shall be as per IS: 456 and IS: 2974. The provisions of DIN 4024 shall also be followed for machine foundations.

10.07.01 All machine / equipment foundations and structures subject to vibrations shall suitably proportioned so that amplitude and frequency of the foundation / structures are within permissible limits.

10.07.02 All block foundations resting on soil shall be designed using the elastic half space theory or Barkens theory. Block foundation resting on piles shall be designed using Novak's theory or Barken's theory. The mass of the RCC block shall not be less than three times mass of the machine and the CG of the combined mass of foundation and equipment should pass through the CG of the base area with tolerance not more than 5%. Dynamic analysis shall be carried out to calculate natural frequencies in all modes including coupled modes and to calculate vibration amplitudes. Frequency and amplitude criteria as laid down by the relevant codes or



machine manufacturers shall be satisfied. Minimum reinforcement shall be governed by IS: 2974 and IS: 456.

10.07.03

For the foundations supporting minor equipment weighing less than one ton or if the mass of the rotating parts is less than one hundredth of the mass of the foundation, no dynamic analysis is necessary. However, if such minor equipment is to be supported on building structures, floors, etc. suitable vibration isolation shall be provided by means of springs, neoprene pads, etc. and such vibration isolation system (VIS) shall be designed suitably.

10.07.04

- a) For the foundation of Turbo-generator, Boiler feed pumps, Fan (ID, FD and PA), Mills etc., detailed static and dynamic analysis shall be done. The static analysis shall include all operating conditions, load cases and abnormal loads like short circuit, loss of blade, unbalance and seismic forces. Unbalance loads for normal operating condition as given by machine manufacturer and / or VDI 2060 whichever is more conservative shall be used for calculating dynamic response. The dynamic analysis. Transient analysis shall be carried out for the short circuit condition with an appropriate force function. Frequency separation criteria and amplitude criteria as laid down in IS: 2974 and / or DIN 4024 and / or VDI 2056 and / or as required by the machine manufacturer, whichever is more stringent shall be satisfied. RCC design shall be done by working stress method for all machine foundations. A fatigue factor of 2.0 shall be considered for dynamic forces. Minimum reinforcement shall be governed by IS:2974 as well as IS:456. However minimum reinforcement in bottom face of the foundation raft resting on soil or pile shall not be less than 0.2% of effective cross sectional area of the raft.

The special requirements for concreting including grade, type of aggregate, use of admixture, temperature control, ultrasonic testing, etc. shall be as mentioned elsewhere in this specification.

- b) All block foundations supporting rotating equipment resting on soil or piles shall be designed using the elastic half space theory. The mass of the RCC block shall not be less than three times the mass of the machine. Dynamic analysis shall be carried out to calculate natural frequencies in all the modes including coupled modes and to calculate vibration amplitudes. Frequency and amplitude criteria as laid down in the relevant codes and / or by machine manufacturer whichever is more stringent shall be satisfied. Minimum reinforcement shall be governed by IS: 2974 and IS: 456. Minimum Reinforcement in base raft in either direction shall be as follows:-

- | | | |
|-----|----------------|--------------------------------------|
| i) | At bottom face | 0.2% of gross cross-sectional area. |
| ii) | At top face | 0.12% of gross cross-sectional area. |

- c) For the foundations supporting minor rotating equipment weighing less than one tonne or if the mass of the rotating parts is less than one hundredth of the mass of the foundation, no dynamic analysis is necessary. However, if such minor equipment is to be supported on building structure, floors, etc. suitable vibration



isolation shall be provided by means of springs, neoprene pads, etc. and such vibration isolation system shall be designed suitably.

- d) TG, BFP, Mill, PA, FD & ID fan foundations shall be supported on vibration isolation system. The vibration isolation system shall consist of steel helical spring units and viscous dampers supporting the RCC deck which would support the machine. The spring units shall conform to DIN 2089 and DIN 2096.
- e) GERB or equivalent manufacturer's vibration isolation system shall be supplied for supporting machines like TG, BFP, Mill, PA, FD & ID fan.
- f) Isolation efficiency of at least 90% shall be provided for the Turbo generator, ID fan foundations.
- g) All approved drawings of equipment foundations shall be vetted by execution agency of civil works in order to match between the equipment sizing and foundation.

10.08.00 Increase in Stresses

10.08.01 Where stresses due to wind (or seismic) and temperature are combined with those due to other loads, the allowable stresses in concrete and reinforcement steel shall be increased by 33.33% in case of working stress design.

10.08.02 Bearing capacity of the soil / pile capacity shall be allowed to increase by 25% under seismic / wind load condition except for chimney where increase in bearing capacity/pile capacity is not considered.

10.09.00 Minimum thickness of structural elements:

The following minimum thickness shall be followed:

Flat roof slab	:	125 mm
Suspended floor / slab / walkways / canopy slabs etc.	:	150 mm
Ground floor slab (non-suspended)	:	200 mm
Water retaining slabs / walls	:	200 mm
Cable / pipe trenches / underground pits / Launder walls and base slab	:	125 mm
All footings (including raft foundations)	:	200 mm
Parapets	:	125 mm
Sunshades at edge	:	75 mm
Pre-cast louvers / fins	:	50 mm



Pre-cast trench cover slabs / floor slabs / louvers	:	75 mm
Paving	:	200 mm
Basement walls and base slab	:	200 mm
Silo / bin walls	:	150 mm
Underground reservoir:		
Below ground	:	200 mm
Above ground	:	150 mm

10.10.00 From fire resistance point of view minimum thickness of reinforced concrete members shall be as per fig. 1 or table 16a of IS:456.

10.11.00 MINIMUM HEIGHT FOR PEDESTALS / ENCASEMENT OF STEEL COLUMNS

10.11.01 Pedestals to steel columns for building structures

In case the top of pedestal is kept at a lower level so that the column base plate together with gussets and stiffeners remain below finished floor level (FFL) the column bases as well as the column sections shall be encased in concrete above FFL as per following:

- a. Open area : 500 mm above paved level
- b. Covered area
 - Internal Column : 1000 mm above FFL
 - Peripheral Column : 500 mm above FFL

Stair and ladder pedestal shall be kept 500 mm above the finished floor level.

Boiler structure supporting steel columns will be encased upto scarper feeder top level.

10.11.02 Pedestals to steel columns for equipment structure:

- a. Equipment in open area : As required (300 mm min.)
- b. Equipment in covered area : As required (150 mm min.)
- c. Structures and equipment : As per vendor's data subject to supplied by vendor minimum as specified above.

Dense and durable concrete with controlled water cement ratio preferably 0.45 shall be used for all underground concrete structures such as foundations, basements, pump



houses, water retaining structures, cable and pipe trenches etc. for achieving water tightness and durability.

All foundation, embedments, inserts, block-outs etc. required for equipment shall be provided by the Bidder.

Fly ash bricks shall be used for masonry work. Bidder shall ascertain himself at site regarding the availability of fly ash bricks of minimum 75kg/cm² compressive strength before submitting his offer.

Aluminium nosing shall be provided for edge protection in RCC stairs. Wherever possible around floor openings an RCC kerb of 100 mm wide 150 mm high shall be provided. All concrete edges, where breakage of concrete corners expected shall be provided with angles of minimum size L 50x50x6 with lugs for edge protection e.g. all round the cut-outs & openings in floor slab, edges of drains supporting grating covers, edges of RCC cable & pipe trenches supporting covers, edges of manholes supporting covers and supporting edges of precast covers etc.

All cables & pipes in outlying area shall run above ground over steel trestles or other supporting structures for easy inspection & maintenance except in transformer yard area and some other localized area where the same can run in RCC trenches. However, for facilities for which buried pipe & cables are permitted by Purchaser, the same can be provided. In case of trestles, minimum 7.0 m head clearance shall be provided for road crossings. In other areas, the clear height shall be 3.0m minimum.

All cable and pipe trenches shall be of RCC of minimum M25 grade. Trenches located outside buildings shall project at least 200mm above the finished formation level so that no storm water shall enter into trench. The bottom of the trench shall be sloped suitably for draining out the collected water into sump pit. The pre-cast covers shall be of minimum M25 grade and shall not weigh more than 65kg. Lifting hooks shall be provided in the pre-cast covers. Pre-cast covers shall have edge protection angles at top and bottom on all the four sides along with lugs.

All construction drawing furnished by Bidder shall consist of total quantity of concrete (grade-wise), reinforcement steel (diameter wise) and structural steel (Section – wise).

Duct banks consisting of PVC / GI conduits for cables shall be sealed using approved fire retardant sealing compound.

All water retaining structures designed as un-cracked section shall also be tested for water tightness at full water level in accordance with IS:3370 (Part – I) and IS:6494.

10.12.00 Formwork Requirements

Refer Volume-VI, Part-B.

10.13.00 The number of construction joints in the columns of steam turbine foundation shall be restricted.

Construction joints at the following three locations shall be provided:

- a) At the meeting points of the columns and the raft.



- b) At the meeting points of the column and the top deck.

Additional reinforcement and shear keys shall be provided at the construction joints.

The base raft for steam turbine and GT foundation and table top for steam turbine shall be cast in single pour.

Mild steel bars required for earthing / grounding mat shall also be supplied and installed.

All cable vaults shall be located above ground level i.e. cable vaults shall not be provided as basements in the buildings. Minimum clear height below beam bottom shall be 2200mm in all cable vaults.

Connection work at terminal points is included in the scope of the Bidder.

All transformer / shunt reactor foundations shall be designed as per relevant IS Codes. This will include collection of oil, drainage of oil along with rainwater collected in such foundation.

Ground floor slab of all the building and RCC paving shall be of minimum 200mm thick of M-20 grade laid over minimum 75mm thick PCC and 230mm soling (minimum) course unless specifically mentioned otherwise. The reinforcement shall consist of minimum 8mm diameter bars at 200 mm c/c of grade Fe 415 at top and bottom in both directions.

10.14.00 Concrete Mix

The following minimum grades of concrete as per IS:456 shall generally be used for the type of structures noted against each grade.

- | | | | |
|----|-----------------|---|---|
| a. | Grade M15 | : | Fill concrete |
| b. | Mix (1:4:8) | : | Foundation below brick wall, blinding layer below foundations, trenches and underground structures, minimum thickness of the layer shall be 75 mm. |
| c. | Grade M20 | : | Base plate encasement, pavement around building including plinth protection work, encasement of structural steel work, grade slab & grade beams etc. |
| d. | Grade M25 & M30 | : | Chimney raft and all RCC members, e.g. foundation and superstructure, pedestals, roof slabs, cable and pipe trenches, water retaining structures, cooling water channel, CWPH forebay, raft & sump etc. |



- e. Grade M35 : TG foundation, TG top deck, chimney shell etc. and for structures coming in contact with sea water.

For superstructure of RCC chimney and natural draft cooling towers richer mix may be used as per design requirement. However, requirement of Table-4 & 5 of IS:456 shall be satisfied as per the exposure condition. As per IS:456, exposure condition will be “severe” for all structures under the civil scope of work except for sea water intake well and pump house, cooling tower, CWPB forebay & sump etc. coming in contact with sea water, where in exposure condition shall be “very severe”.

Washing and screening of coarse and fine aggregates to remove fines, dirt or other deleterious materials shall be carried out by approved means, if desired by the Engineer-in-charge.

The water cement ratio by weight shall be 0.45 maximum including free moisture in the aggregate and slump should be suitably decided to provide good quality concrete work.

10.15.00 Special Painting For Sea Water Structures

All concrete surface coming in contact with sea water will be painted with 1.5 mm thick polyurea coating meeting ASTM D-16, Type V (two component, chemical cure). This will be applicable for pump house, Natural draught cooling tower and CW pump house forebay and sump, Clarified Water reservoir, Guard pond, Fire water sump, RO stage I Permeate Tank etc.

For NDCT inside surface special painting shall be provided upto 2.0m above drift eliminator level.

10.16.00 Allowable Settlement

The total permissible settlement and differential settlement of the foundations will be governed by IS:1904, IS:13063 and from functional requirements whichever is more stringent.

Maximum allowable total settlement should be restricted to 25 mm for foundations of all plant structures like cooling towers, CW pump house, DM plant, crusher house, transfer house etc.,

Maximum allowable total settlement should be restricted to 40 mm for all other foundations.

All foundations shall be so designed that the settlements are within permissible limits as per relevant Indian Standard or from consideration of safe equipment / machine operation whichever is critical. In case of open foundations without piles, Ground improvement methods opted by contractor, like sand drains / stone columns etc. to reduce settlement and increase the rate of settlement, detail methodology of such installation supported by calculation based on relevant field data shall be furnished for approval of Owner / Consultants before execution. In the event, the contractor adopts any patented method of ground improvement or retain any specialized agency for such purpose, the same can be done subject to prior approval of owner.



Boiler and ESP support structures shall be checked for differential settlement of foundations which shall be restricted to 1 in 1000 of span or 8 mm whichever is less.



11.00.00 DESCRIPTION OF BUILDINGS, STRUCTURES AND OTHER FACILITIES.

11.01.00 TEMPORARY STRUCTURES FOR EXECUTION

11.01.01 Construction water:

The OWNER shall provide water at one point within the plant boundary or a limited purpose such as for construction. The Contractor may avail this facility. Necessary charges for drawing the water shall be borne by the CONTRACTOR.

11.01.02 Construction Power

Construction power will be made available at a single point, metered and supplied on chargeable base by the owner. Further extension of power line to the various locations required shall be executed by Contractor at his cost. Contractor has to make provision of D.G sets as standby power source. This is especially in the case of structure involving deep dewatering as well as where uninterrupted concreting has to be resorted to. The bidder along with his bid shall indicate his average and peak power demand.

11.01.03 Temporary site buildings

The Contractor shall provide for at his cost the following building facilities for proper execution and quality control of the job, while meeting the provision stipulated by Factory Rules regarding staff welfare facilities. All these buildings shall have brick cladding, Steel / AC sheet roofing over steel roof truss with cement concrete flooring and false ceiling with A/C as required.

a) Site office for Owner

The site office shall make a provision of about 700 sqm of office area with A/C and false ceiling for the use of Owner / Owner's representative in addition to the Contractor's requirement. Additionally an A/C Conference room to accommodate about 70 people shall also be provided in the site office complex for the Owner's use. In addition to these, basic facilities like toilet for gents and ladies, potable water tanks, soak pit and septic tank for sewage disposal shall also be provided. Covered parking area for parking 10 cars shall also be provided for Owner's use. EPC Contractor to ensure that finished site office is handed over to owner within 3 months from the date of taken over the site by Contractor

b) Stores

A covered store shall be provided with brick cladding and G.I./colour coated sheeting to store at least one month requirement of cement. Cement in bags shall be stored on a raised floor well away from outer walls and insulated from the floor to avoid moisture. Not more than 15 bags shall be stacked in any tier. Each consignment of cement shall be stored separately and consumed in its order of receipt.

Covered storage area may also be provided to store other construction material which will be affected on exposure to wind, sun and rain.



Reinforcement shall be stacked on top of timber sleepers to avoid contact with ground / water.

Storage yard paved or unpaved shall be provided with in the stores complex for storage of other material.

Proper fencing and security arrangement shall be provided for the stores complex.

Contractor may consider using the existing building in the site with necessary repairs and restoration for use of this facility, provided the same does not interfere with construction of Permanent works.

11.01.04 Fabrication yard

Depending on the extent of fabrication envisaged at site, the CONTRACTOR shall establish a full fledged structural fabrication yard with adequate handling facility during and after the fabrication. A fully equipped testing laboratory providing radiography, ultrasonic, dye penetration, magnetic particle test facilities shall be ensured adjacent to the fabrication yard to enforce strict quality control. Portion of the yard shall have covered shed with H.O.T / E.O.T cranes so that fabrication work can proceed even during inclement weather.

11.01.05 Quality control laboratory

A fully equipped quality control laboratory shall be established at site with qualified personnel to conduct acceptance test on all construction material, weldments, concrete cubes etc. This laboratory shall be housed in a covered building with A/C facility as required by the testing facility. All testing equipment shall be periodically calibrated to the satisfaction of the OWNER. All testing shall be carried out in presence of OWNER. Finally the laboratory shall be handed over to OWNER in good condition after completion of project.

11.01.06 Fuel storage area

CONTRACTOR shall obtain necessary permission from competent authorities and establish and operate a POL outlet with proper storage, dispensing and adequate fire fighting facility.

11.01.07 Staff Welfare facility

CONTRACTOR shall provide adequate facility for his staff inside the plant boundary such as Toilets for both gents and ladies, Canteens, drinking water facility, rest places, creches etc.

Necessary approach roads to the construction facility complex and internal roads within the complex as well as proper drainage of the area shall be the CONTRACTOR's responsibility.

CONTRACTOR shall also provide for proper disposal of sewage and other wastewater to



meet with the requirement of Pollution Board.

CONTRACTOR shall identify sufficient area outside the plant boundary to locate his staff and labour colony. Construction and maintenance of the staff and labour colony to satisfy all statutory requirement is the sole responsibility of CONTRACTOR.

11.02.00 TURBINE GENERATOR BUILDING

Turbine building (TG bay and heater bay) framing shall be of structural steel with moment connected framing in the transverse direction and bracing in the longitudinal direction.

Service and maintenance bays shall not have any intermediate floors, however a 1500 mm wide observation gallery with handrails shall be given along the wall at the operating floor level to observe the TG erection operation. Intermediate floors shall preferably be provided at 8.50 m and operating floor at 17.0m. Floor at 8.50m and operating level shall be RCC floors over steel framing, where as the floor at any other level if required may have chequered plate supported on structural steel frame work.

Crane capacity and crane rail level shall be fixed based on the equipment to be lifted and the method of lifting generator stator. At crane girder top flange level a crane walkway shall be provided in line with factory rules. Access shall be provided to crane walkway through staircase from operating floor in addition to cage ladder at two ends. Bottom level of roof framing shall be decided by the crane clearance requirement duly taking into account clearance required for mounting light fixtures. Roofing shall consist of in-situ RCC slab of minimum 150 mm thickness laid to a slope of 1 in 100, constructed over permanently colour coated galvanized MS troughed metal sheet of approved profile supported on steel purlins and trusses. TG bay roof shall be provided with exhaust system.

Permanent colour coated sandwiched insulated metal cladding system from 3m (approx) above ground floor up to roof shall be provided on gable end and A row. Brickwork shall generally be provided from ground floor to 3m (approx) height. On other rows brickwork shall be provided upto roof level. Wall in front of transformers shall be of adequate thickness to satisfy "fire rating" as per TAC regulations. On the heater bay side the brick wall shall be provided upto 1m above deaerator floor level. Cladding beyond the above level shall of permanent colour coated sandwiched insulated metal cladding system.

Windows shall be side-hung steel glazed using 6mm thick wired glass generally. However in areas where cladding is of sheeting fixed glazing in anodized aluminium framework and in accessible areas sliding windows in anodized aluminium framework using 6mm thick wired glass shall be provided. All the doors on external walls shall be of double plate flush steel doors. For equipment entry into the service bay specially designed steel sliding cum / folding / rolling shutters shall be provided with appropriate operating mechanism. Rolling shutter shall also be provided in front of condenser to facilitate tube removal.

Staircase protected on all sides with fireproof enclosure shall be provided to satisfy LPA regulations. All the doors leading to the inside of the power house from staircase shall be automatically closing fire proof door satisfying LPA regulations. Access staircase shall be provided to TG building roof.



A roof shall be provided with roof water proofing treatment using high solid content liquid applied elastomeric water proofing membrane with separate wearing course as per ASTM-C-836 & 898. Thickness of the membrane shall be 1.5mm (min.). This treatment shall include application of polymerized mastic over the roof to achieve smooth surface and primer coat. Wearing course on the top of membrane shall consist of 25 mm thick PCC (1:2:4) cast in panels of maximum 1.2x1.2m size and reinforced with 0.56mm dia galvanized chicken wire mesh and sealing of joints using sealing compound/ elastomeric water proofing membrane. However, chequered concrete tile flooring 22mm (min.) thick of approved colour and shade conforming to IS:13801 shall be provided for path way of 1m width for access of personnel and handling of equipment and for the entire area of the roof where equipment like A/C ventilation plant, cooling towers, etc. are provided in place of PCC wearing course. Equipment shall be installed on raised pedestal of minimum 30 cm height from the finished roof to facilitate maintenance of roof treatment in future. HDPE rain water pipe of min. 150 dia OD conforming to IS:4984 shall be provided to drain off the rainwater from roof. They shall be suitably concealed from the façade.

Adequate no of roof extractors shall be provided As/mech layout details.

Thickness of structural RCC floor shall be minimum 150 mm above the steel floor beams for the intermediate floors with an additional 50 mm floor finish, which shall be provided later.

All the intermediate floors shall be provided with proper drainage. Catch pit with CI grills shall be provided near the internal column in a regular pattern and the floor shall be sloped towards the catch pit by varying the thickness of the bedding concrete for the floor finish. Catch pits shall be at the same co-ordinate for all floor levels. 80 NB galvanized MS pipes conforming to IS:1239 shall be provided to carry the drainage down. The pipes shall be clamped on to the intermediate column. Sumps shall be provided in ground floor to receive the water. Proper drainage arrangement shall be provided in the ground floor by means of RCC gravity channels with MS grills on top. The drainage shall be led to sumps from where the water shall be pumped to plant drainage, concrete kerb shall be provided to isolate the area and water shall be led through sump and channels to the floor drainage system. Oil water separator should be provided as required to isolate oil from the drainage water where the water is mixed with oil.

In operating floor the design live load shall be painted on the floors prominently showing the extent of the area where such loading is permitted.

Wherever openings are provided in the floor for handling of equipment using EOT cranes such openings shall be covered with gratings provided over removable steel beams.

Expansion joint shall be provided in the building to satisfy the requirements of IS:800 and IS:3414.

Suitable provision for future expansion shall be made by BIDDER while designing / executing the foundation system for last grid of main plant building.

In any case auxiliary columns shall not be supported on TG foundation. Preferably auxiliary columns shall be fabricated box section.



Separate Maintenance room for Turbine and Generator are to be provided.

11.03.00**CONTROL ROOM / ELECTRICAL BAY STRUCTURE**

The building framing shall be structural steel with moment connection in the transverse direction and bracing in longitudinal direction located by the side of turbine building. It shall be ensured that in front of control room no cross bracing is provided.

A minimum headroom of 3000 mm below beam bottom shall be given in the cable vaults. In control room false ceiling level shall be kept 4000 mm above furnished floor level

All floors shall be of deck RCC slab provided over structural steel beams with colour coated permanently laid deck sheet shuttering. Minimum uniform thickness of structural concrete above deck corrugation shall be 150 mm with an additional 50 mm provided for finish. Roof shall also be of in-situ concrete with colour coated permanently laid deck sheet shuttering. The concrete shall be given a slope of minimum 1 in 100 to effect proper drainage. No openings will be permitted on the roof of control room except possible opening for A/C duct to exercise proper control on air conditioning.

Cladding shall be of brick walls. Windows from switchgear room and control room / electronic cubicle room will not normally be allowed on the Boiler side to avoid coal dust nuisance. Walls shall normally be supported on wall/floor beams.

All the wall beams shall be provided with 50mm gunniting using Lugs and steel mesh.

Portion of the structural steel column projecting into the room shall be encased with brick masonry to shield it from fire hazards.

Cable vault in ground and second floor shall be provided with minimum 2 doors per unit. The doors shall be flush steel doors. No windows shall be provided on the turbine bay side

Switchgear room shall be provided with minimum two steel doors to satisfy statutory requirement. In addition a two leaf sliding doors of slush welded steel construction shall be provided to move the switchgears into the room making use of equipment lifting hatch provided in turbine building.

Main entrance to control room shall be provided with air locked lobby with automatic closing sliding glass doors. Lobby shall be formed of anodized aluminium framing with toughened sheet glass 6 mm thick. Partition between control room and adjoining rooms shall be of glazed aluminium partition with 300 mm high brick wall at bottom for toe protection. All the doors shall be single leaf glazed aluminium doors for all cabins. For movements of panels suitably sized double leaf aluminium glazed doors shall be provided. Control room wall upto the false ceiling level shall be of aluminium glazed partition on either side of air lock. In other area cladding shall be of brick wall. UPS room shall be provided with brick cladding all-round to reduce sound nuisance.

Battery room shall be provided with metal door. For Air washer room steel doors shall be used which shall be airtight. No window shall be provided in air washer room.



Necessary separation walls for the UPS charger, panels and the batteries in the battery room shall be provided.

Staircase properly enclosed to satisfy LPA regulations shall be provided at either ends of each unit. All doors leading to staircase shall be automatic closing fireproof door. Staircase shall not be provided around the passenger lift. Staircase shall extend upto roof. Toilets shall be provided at the switchgear room level and control room level. Toilet floors shall be sunk by 50mm and shaft shall be provided to route the soil pipe. Sunken floor shall be provided with damp proofing treatment.

Control room and electronic cubicle room is to be provided with false ceiling. False ceiling shall be designed aesthetically properly arranging, supply air diffuser, return air grill, fire protection sprinklers and light fittings. Aluminium ceiling system shall be provided. Under deck insulation shall be provided on the ceiling, on the walls and beams above false ceiling. Under deck insulation shall be provided for air washer room also.

Floor above control room where APRDS and other pipes are located as well as floor below the deaerator area shall be provided with proper drains to prevent any accumulation of water. This is very important to prevent seepage of water into the control room.

All openings in floor for switchgear and other panels shall be sealed with fireproof material after cables are connected. Adequately designed RCC storm water drains shall be provided to lead the roof drainage on to the plant drainage system. These drains shall have pre-cast RCC slotted cover. Plinth protection shall be given on the side of the drain up to the brick wall.

Provision of central control room shall be satisfied the requirement of vol. V also.

11.04.00

MILL AND BUNKER BAY BUILDING

The framing shall be of structural steel. This shall be designed as a moment connected framing in the transverse direction and braced in the longitudinal direction. These structures primarily support coalbunkers, coal feeders and tripper arrangement to feed the coal into coal bunker.

The bunker shall be circular in plan and shall have the capacity as per the requirement of steam generator.

A mill maintenance platform shall be provided at appropriate elevation to attend to routine maintenance. This platform shall be of chequered plate over steel framing with handrail all around. Above this floor a framing shall be provided to support under slung crane used for handling mill maintenance work. The crane runway girders shall extend to the full length of mill bay to enable approach to the service bay also.

Framework for supporting the circular bunker are provided above the feeder floor based on geometry of the hopper. Preferably bunker shall be supported on equally spaced (min.) 8 supports.

Bunker shall be provided with a top steel cover with slit openings for entry of coal. On sides of the bunker necessary openings shall have to be provided for bunker ventilation / dust extraction.



Tripper floor is immediately above the bunker. This floor is of RCC supported on structural steel framework.

Tripper roof shall be of RCC supported on structural steel framework. A fall of about 300 mm is given for the roof framing to effect proper drainage. Tripper roof supports bunker ventilation / dust supporting structure. An RCC parapet shall be provided for the tripper roof. Access stair/ladder shall be provided from tripper floor to roof in each unit.

In addition to the above, framing arrangement for structural steel platform at various levels around the bunker shall be provided with proper access ladder, for poking, striking and attending to air canons

Tripper floor and roof shall be given access through boiler staircase / elevators.

Bunker supporting structure is normally left uncladded up to the bottom of tripper floor. For tripper floor upto 3000 mm above finished tripper floor level, brick wall shall be provided. Above this level permanent colour coated galvanized trough sheet cladding shall be provided. Fixed steel windows with 6mm thick wired glass shall be provided in the tripper house as per requirements for conveyor gallery

Roof slab will be sloped to one side to permit easy drainage. Liquid membrane water proofing treatment is to be provided. As the roof is accessible and maintenance crew is likely to work on this floor, the entire roof shall be provided with min. 50 thick screed over the water proofing treatment.

Rainwater down take pipes shall be UPVC pipe of class-3 conforming to IS:4985.

One number goods cum passenger elevator, and two nos. of staircase shall provide access to all floor/platforms extending upto the roof level. These structures primarily support coal bunkers, coal feeders and tripper arrangement to feed the coal into coal bunker. Requisite nos. of corridor as per operational requirements shall be provided to access steam generator building from Mill building. The Mill building and steam generator building shall have expansion gap and corridor shall be connected with sliding supports.

Stainless steel liners shall be provided in bunkers for the conical / hyperbolic as well as vertical portion of Bunker. The grade SS liner shall be SS316L.

Colour coated metal deck sheet will be used below RCC floors, it shall be treated as shuttering material only. Uniform thickness of slab, excluding corrugations shall be min. 150mm. In this case, top of steel of beam shall be decided carefully.

11.05.00

STEAM GENERATOR AREA PAVING

Entire steam generator area upto the end of chimney shall be provided with RCC paving over rubble stone soling (for the details of RCC paving refer clause 10.13 of Volume-VI, Part-A).

Paving shall be carefully planned such that maintenance access roads of minimum 4m wide to all major equipment are available. This area shall be designed for heavy loads similar to design of roads. Such approach road shall be provided to the SG elevator also



to enable handling of equipment at higher level during maintenance. Such roadways shall be clearly identified with kerbs / painting. All drain / trench covers in these areas shall be designed to withstand truck loading.

The drainage of the entire SG area shall be properly planned. Network of RCC drains with RCC cover with slotted holes or MS grills shall be provided covering the entire area which lead the storm as well as process leakage water to the peripheral drains, which in turn will lead to plant drainage system. Where open drains are not permitted under ground pipes may be provided in which case at all junctions and ends proper manholes shall be provided to permit periodic cleaning of the pipes. All structural steel columns in the area shall be provided with encasing to a height of minimum 1000 mm above paving level and in addition the bases also need encasing if base plates are below paving level. For periodical cleaning of manholes suitable screens or baskets shall be provided for easy cleaning from time to time.

11.06.00 TRANSFORMER YARD

Generator transformer, station transformer, unit auxiliary transformer and service transformer are located in front of the turbine building. Transformers shall be installed on RCC foundations with rails on the top and oil soak pits filled with hard stone aggregate. Burnt oil pits are provided to collect leaked oil from the soak pit through salt glazed, pipes laid to slope. RCC blast wall / RCC frame with brick wall of adequate thickness and height to satisfy TAC regulations shall be provided in between transformers as fire barrier wall. Common oil sump & emergency oil sump with RCC roof shall also be provided with man hole arrangements.

RCC foundations shall be provided with rail to transport transformers out of transformer yard during maintenance. Entire area shall be surrounded with 3m high chain link fencing with gates. Where rails cross the fencing, fencing shall be made of removable type to facilitate transport of transformer. Floors shall be paved with plain cement concrete and shall be sloped towards peripheral drains, which shall lead to a sump from which the drainage is led through an oil water separator. The clear water shall be led to plant drainage. All oil sumps shall be covered with RCC cover slab with manhole arrangement.

11.07.00 ESP Control Room

ESP panel control room buildings shall be RCC framed building with brick walls plastered on both faces, complete with drains, space conditioning, and building lighting. The building shall have a separate air-conditioned area; and a separate non air-conditioned area except as required for the contained equipment. The contractor shall ascertain the building size according to the design requirements. ESP Transformer yard, firewalls & fencing shall also be provided as per requirement.

One maintenance room of size 20 x 10m² shall also be provided.

11.08.00 SWITCHYARD STRUCTURES & CONTROL BUILDING

Extent of GIS switchyard and number of towers and gantries and the size of switchyard control room will be as per the layout of the switchyard which has been outlined in Vol. IV of the specification.



Switchyard control building will be of concrete frame structure having brick wall cladding. A crane of suitable capacity will be housed in switchyard building.

The building shall be complete with toilets, drains, plumbing, sanitary drainage and treatment piping, space conditioning for a non-air-conditioned area, building lighting, station combinations and fire protection etc. An adequately sized dedicated air conditioning plant shall be provided for the control room. For switchyard Civil Work refer Section-28 of Part-B, Volume-VI. Epoxy flooring shall be provided in all switchyard control room buildings and GIS buildings.

Maintenance room, Mini conference hall, Pantry room, battery room and EPBAX room shall be provided in these area.

Necessary covered pathways shall be made for GIS and other adjacent buildings.

11.09.00

INTAKE WATER PUMP HOUSE (AT EXISTING COOLING WATER FOREBAY OF NCTPS STAGE-II)

Near existing cooling water forebay of NCTPS-stage II, a pump house is proposed for makeup water with necessary modification of existing forebay at intake point. Raw water would be conveyed through steel conduits / pipe to the plant area.

The pump house structures shall be designed in accordance with IS-3370 (Part -1 to 4) . Cladding shall be of brick work. Crane girder shall be of steel. Cage ladder shall be provided for access to crane platform. Side of the pump house up to motor floor level on the pump discharge side shall be of RCC. Maintenance bay of the pump house shall have RCC grade slab with granolithic finish and non-metallic floor hardener. Motor floor shall also have similar finish.

Pump sump shall be designed as water retaining structure. Each sump shall be provided with stainless steel (SS 316L) groove for inserting coarse screen, fine screen and stop logs. Grooves for stop log shall be provided before and after the screens. Number of pump sumps shall be minimum of 3. Stop logs, coarse screen and fine screen shall be of material as mentioned in mechanical part of the specification. Top of the sump floor shall be kept same as of the top of forebay. This floor shall have openings for lowering the lifting of stop logs and screens. These openings shall be covered with galvanized mild steel gratings when the screens are placed in position. Provision for handling of the screens and stoplogs shall be made. Sufficient paved space shall be provided to keep the stop log when not in use and also for maintenance of screens. Galvanized MS handrails shall be provided on 3 sides of the pump floor (sump roof). Each individual pump sump shall also be provided with a drain sump to empty the sump for attending maintenance of pump.

Vertical wet pit type of pumps are envisaged in the pump house.

Dimension of the pump sump shall be decided as per mechanical requirements.

EOT crane of adequate capacity shall be provided for maintenance of the pumps and other auxiliary equipments.

An annex to sea water pump house shall be provided in RCC framed structure to accommodate switchgear and control room with cable vault below, chlorinator room and toilet block. Side cladding shall be of brickwork. All floors and roofs shall be RCC. Roof shall have proper slope and Elastomeric Membrane water proofing treatment shall



be provided and rain water down fall pipe shall be provided. Control room shall have false ceiling / false flooring and shall be air-conditioned.

Necessary office rooms 200m² and store room with adequate space shall be provided on the first floor of proposed pump house building annex as approved by Owner.

All doors shall be of aluminium for control room. For switchgear room, the main entrance door shall be of adequate size to facilitate movement of panels. All windows shall be aluminium glazed side hung.

Additional corrosion protection measures for steel, and concrete required if any due to the presence of sea water shall be submitted by BIDDER along with the bid.

11.10.00 CIRCULATING WATER SYSTEM

The circulating water system envisaged for the plant is sea water. From intake water pump house near the existing forebay of NCTPS STAGE-II, sea water will be pumped to CW forebay of proposed plant area. From CW pump house, water will be drawn by volute casing type CW pumps for condenser cooling.

From Intake water pump house to proposed CW forebay sea water will be conveyed through GRP pipes. The pipes shall be designed for worst possible combination of loads.

11.11.00 CW PUMP HOUSE, FOREBAY & COOLING WATER CHANNEL

The pump house and forebay structures shall be of RCC. Cladding shall be of brick work. Crane girder shall be of steel. Cage ladder shall be provided for access to crane platform. Side of the pump house up to motor floor level on the pump discharge side shall be of RCC. Maintenance bay of the pump house shall have RCC grade slab with granolithic finish with non-metallic floor hardener. Motor floor shall also have similar finish.

Pump sump, CW channel and forebay structure shall be designed in accordance with IS-3370 (Part 1 to 4). Each sump shall be provided with stainless steel (SS 316L) groove for inserting coarse screen, fine screen and stop logs. Grooves for stop log shall be provided before and after the screens. The stop log shall be epoxy painted and coarse / fine screens shall have stainless steel mesh over galvanized MS frame. Number and size of pump sump and stop log shall be as per the requirement of mechanical design. Top of the sump floor shall be kept same as of the top of forebay. This floor shall have openings for lowering the lifting of stop logs and screens. These openings shall be covered with galvanized mild steel gratings when the screens are placed in position. Provision for handling of the screens and stoplogs shall be made. Sufficient paved space shall be provided to keep the stop log when not in use and also for maintenance of screens. Galvanized MS handrails shall be provided on 3 sides of the pump floor (sump roof). Each individual pump sump shall also be provided with a drain sump to empty the sump for attending to maintenance of pump.



Since concrete volute pumps are envisaged separate pump floor, motor floor & valve floor shall be built in the pump house with proper access arrangements. Suitable staircase, ladders & cross-over platforms shall be provided.

An annex to the CW pump house shall be provided in RCC framed structure to accommodate switchgear room and control room and engine room with cable vault below, chlorinator room, Chlorine cylinder storage room and toilet block. Side cladding shall be of brickwork. All floor and roof shall be of RCC. Roof shall be given proper slope and heavy duty water proofing. Rain water down comers shall be of UPVC pipe of class-3 conforming to IS:4985. Roofs shall be made accessible by MS galvanized cage ladder. Staircase shall be provided from pump floor to operating floor.

Provision shall be made for installing three Nos. C.T blow down pumps at CW pump house to drain the C.T blow down water to sea.

All doors shall be of flush welded steel construction. For switch gear room, the main entrance door shall be of adequate size to transport the panels. All windows shall be steel glazed side hung. Wall in front of transformer yard shall be designed to satisfy fire rating as per TAC regulations with fencing.

An auxiliary transformer yard may be required in front of the switchgear room. Civil work similar to the transformer yard in front of ESP control room shall be included in the scope of work.

Outside the pump house support blocks for butterfly valves along with proper handling facility shall be provided.

Since soil is very soft in nature, piles shall be provided as a foundation system for CW pump house structures. PVC, PRV can be used to relieve uplift water pressure on the base slab of CW forebay. In this case, base slab of the CW forebay shall be designed for 60% of water uplift pressure.

Suitable thrust block to hold CW pipe line at various locations as per mechanical requirement is also in the scope of BIDDER.

RCC cooling water channel shall be rectangular shape and designed as a water retaining structure. Depth of channel and width of channel shall match with the width and depth of the cooling water channel from cooling tower. These branch channels shall join together and form common channel leading to the forebay of the pump house. Dimension of the channel to be decided on the basis of the quantity of water to be carried by the channel. Floor of the channel shall be given a mild slope to ensure necessary velocity of flow. The top of the channel shall be kept at least 500 mm above formation level with suitable galvanized MS pipe hand railing on either side. Expansion joint shall be provided at a spacing of about 30 m. Pedestrian cross over shall be provided across the channel at approximately 300 m centres. The channel shall smoothly diverge and form the forebay in front of the pump house.

Depth of the fore bay shall generally match with C.W. pump sump level, which shall be decided based on submergence requirement of the C.W. pumps at the lowest water level in the sump. Forebay shall be provided with cage ladder at four corners constructed with galvanized steel. Top of the fore bay shall be kept 500 mm above grade level and handrail of galvanized M.S. pipes shall be provided for the entire length on either sides.



Water level indicators (float type) shall be provided in the forebay close to the pump sump on either side to observe the depth of water.

11.12.00 CW Inlet and Outlet conduits

RCC Cooling water closed conduits of adequate size shall be provided from C.W pumps to condenser and hot water conduits from condenser to cooling tower. The inside of conduits shall be given special painting as per cl10.15 of this spec. conduits shall be checked for uplift considering water at FFL/FGL considering inside empty. Further it has to be designed for traffic load at road crossing.. The conduits shall be tested before commissioning of the system . RCC Duct shall be designed in accordance with IS-3370 (Part 1 to 4). Stress in r/f shall be limited to 130N/mm^2 as / IS 3370. Expansion joints shall be provided at spacing of about 30 meter. Following design requirements shall also be met:

- a) Provide a minimum clear distance of 1.5 m between ground level and the top of RCC.

Suitable protection devices such as air release valves shall be provided to ensure that design vacuum pressures are not exceeded.

- b) Internal Pressure Condition:

The design of CW ducts shall be done for the following conditions:

- i) Maximum design water pressure
- ii) Surge or water hammer pressure
- iii) Pressure, under testing condition.

- c) External Pressure Condition:

- i) It shall be designed for external pressure due to soil overburden, surcharge loading, ground water, applicable vacuum pressure etc. taken together.
- ii) Additional traffic load at road crossings.

- d) General Requirements:

It shall be designed as a RCC closed conduit water retaining structure subject to

- i) Maximum external pressure when inside empty
- ii) Maximum internal pressure in absence of outside soil
- iii) Uplift when inside empty and ground water level upto finished ground level.

- e) Maximum velocity of water in the duct shall not exceed 2.0 m/sec.

- e) The conduit shall be designed for surcharge of 2 t/m^2

- f) Manholes shall be provided at a maximum spacing of 150 m in



straight reach of conduit. One manhole shall be provided at all horizontal bends and one at the lowest stretch between two vertical bends. Manhole size shall be provided of 1.0m diameter.

- g) Thrust block/anchor blocks shall be provided at bends and other locations, if required, as per design.

11.13.00 FILTERED WATER/ FIRE WATER PUMP HOUSE AND TANK FOUNDATIONS

Dedicated Fire water storage shall be provided in the filtered water storage tank (RO Stage I Permeate) and the fire water pumps shall be installed in the filtered water pumphouse having two compartments with covered RCC roof. It will be over ground. Vent pipes shall be provided at the roof. The Filtered Water/ fire Water pump house shall be single storied flat roofed RCC building with brick as side cladding. Crane shall be provided for handling pumps in line with Vol III.

The building shall be complete with doors, windows and rolling shutter. Capacity shall be as per Vol. III and Composite Water Scheme.

The pump house shall include operator room, dinning room, maintenance room & toilet also.

11.14.00 NON PLANT BUILDING

11.14.01 Service building

The service building shall be a reinforced concrete framed building. The building shall be directly connected to the main plant building at operating floor. The floor area shall not be less than 1500 sqm in each floor. Number of floors shall be described by the Contractor depending upon the space requirements of various facilities. A minimum number of floors shall be 4 (four). The building shall generally be air conditioned with false ceiling. Elevators as indicated in Vol III shall be provided. Hence, clear head room below beams shall be designed to run AC ducts above false ceiling of these air conditioning areas. Level of false ceiling may be kept at 2700 mm in the ground floor and 2400 mm in upper floors in these air conditioned areas. For elevators, necessary civil works to be executed including foundation, roofing etc. based on Mechanical Specification. A grid of concrete beams and columns shall support the reinforced concrete roof (minimum 1% slope) and the floor slabs. The walls shall be of masonry construction, plastered internally and rendered externally. The service building shall have the following provisions:

- a. Seating accommodation for senior O&M executive along with supporting staff including modular furniture of latest style. For the accommodation of all Executives permanent brick wall not to be provided, instead wooden partition wall of cubicle type is to be provided.
- b. Safety and Welfare Cell, First aid room
- c. Reception, drawing hall, library, printer rooms and records room/ archives.



- d. Conference hall with all amenities like latest style modular furniture, loud speaker systems, Projector equipments, service room etc.
- e. Mini conference hall with furniture.
- f. Relay testing lab, electrical lab, C&I repair lab and transmitter lab.
- g. Pantry in each floor.
- h. Toilets – Ladies & Gents in each floor as per specification.
- i. Switch yard control room.
- j. Server room
- k. Battery room
- l. Simulator training room
- m. Connecting platform between TG building and Service building
- n. Electrical and LAN points at each seat in the cubicle, cabins & conference rooms with wiring and fittings.
- o. Minimum clear width of staircase in service building shall be 1.5m.
- p. Cabins for senior officers and conference rooms shall have attached Toilet. The cabins shall be provided wherever required as directed by the owner.
- q. Entrance to the building shall have glazed aluminium door swing type with double leaf. Glazed aluminium partition shall be provided on either side of entrance door. Internal partition in the building shall be provided with a combination of pre-laminated particle board and frosted glass in anodized aluminium framework. All doors to the cabins shall be teak veneered flush plywood door. In other utility areas, aluminium glazed or steel flush doors shall be used. Toilet shall be provided with PVC doors. All windows shall be of glazed aluminium sliding types. For windows located in the ground floor, aluminium grills shall be provided for security.
- r. The service building fascia shall have pleasing aesthetic. Landscaping shall be done around the area by developing garden with flowering plants and fountains etc.
- s. Polished teakwood handrail with stainless steel post shall be provided. At least one staircase shall lead to the roof.

11.14.02 Administrative Building

The Administrative building shall be a reinforced concrete Framed building with two floors having a minimum area of 1000 sqm in each floor. The floor area shall be as per the specification. The building shall generally be air conditioned with false ceiling. Elevators as indicated in Vol III shall be provided. A grid of concrete beams and columns shall support the reinforced concrete roof (minimum 3% slope) and the floor slabs. The walls shall be of masonry construction, plastered internally and rendered externally. The Administrative building shall have the following provisions:

- a. Safety and Welfare Cell, First aid room
- b. Reception, drawing hall, library, printer rooms and records room/ archives.



- c. Conference hall with all amenities like latest style furnitures, loud speaker systems, Projector equipments, service room etc.
- d. Mini conference hall with furnitures.
- e. Pantry
- f. Toilets – Ladies & Gents in each floor as per specification.
- g. Telephone Exchange of 500 lines
- h. Electrical and LAN points at each seat in the cubicle, cabins & conference rooms with wiring and fittings.
- i. Minimum clear width of staircase in administrative building shall be 1.5m.
- j. Ceiling fans

11.14.03 MAINTENANCE OFFICE CUM STORES

Maintenance office cum stores with provision for tool room of adequate shall be provided in the following area. This can either be fully independent building or can be part of Pump house / MCC / other buildings nearby.

- Minimum size shall be 5m x 5m
- Mill area – may be accommodated in GF of bunker bay or power house building
- Boiler area – may be accommodated in GF of ESP control building
- ESP area – may be accommodated in GF of ESP control building
- Ash handling area
- Coal handling area
- Cooling water pump house / Fore bay area – may be accommodated in GF of CW pump house or MCC building
- Sea water intake pump house area
- Sea water outfall pump house area
- Common for transformer area and Switch area – may be accommodated in GF of Switch yard control building
- Common for desalination plant / DM plant / Water treatment plant – may be accommodated in GF of DM plant
- Fuel oil tank area – may be accommodated in GF of fuel oil pump house.

11.14.04 Storage Yard

A storage yard of 70,000msq. shall be provided with a provision 6000m.sq. covered area. The storage yard shall have compound wall of 3m height with anti climbing devise. Covered area shall be of RC construction with RCC roof and brick wall for side cladding. Office space and toilets shall also be provided. Entry & exit of trucks shall be thru rolling shutters of adequate size. In addition a 2 single leaf steel flesh door shall be provided for entry of the staff. Adequate windows shall be provided. The covered sheds shall be connected with bituminous roads in side storage yards. The storage yard shall be provided with 150mm thick gravel paving along with suitable drainage arrangement connected to plant drainage system.

It is preferable to adopt Pre fabricated / Pre Engineering structures for covered area of storage yard. The Structural Steel shall be confirm to IS 2062. The side wall shall be



brick cladding for a height of 1.0m above that color coated galvalume sheets of minimum thickness not less than 0.6mm shall be provided. The roof shall be covered with same sheet of adequate thickness to suit the design requirement. Transparent / Translucent sheet may be used to permit natural lighting. Sufficient number of roof extractor shall be provided for effective ventilation. Aluminum glazed windows shall be provided for natural ventilation and lighting.

EOT crane of suitable capacity shall be provided in storage yard at appropriate locations.

11.04.05 **Parking Sheds**

Parking sheds shall be provided at service building, security complex, at CW pump house area etc., at all locations for operating staff. The parking sheds shall be of RCC.

The bidder has to furnish the three dimensional architectural plans along with the colour codes for the above buildings for approval of the Purchaser for adopting one among the alternatives proposed.

11.14.06 **Workshop**

The building shall house electrical , mechanical workshops and shall comprise the following facilities as a minimum requirement.

Mechanical workshop – consisting of carpentry, Machine shop (for lathes, drilling machines, grinding machines etc.) ,Fabrication / welding and store.

- b) Electrical room
- c) Offices, rest room and toilet
- d) Heavy vehicle shed
- e) Dozer shed

The workshop building area shall be based on the equipment layout during detail engineering. Minimum area of workshop shall not be less than 1000m². The building must have provision of EOT crane for handling. The building shall be of RCC/ steel framed structures with brick masonry side cladding. For other detail and finishes refer to Architectural specification.

11.14.07 **Fire Station**

Fire station shall be single storey RCC framed building with brick walls in office area. Provision for parking of 2 fire tenders including 1 bay for maintenance with pit, office, duty room, stores, recreating room, record room and toilet blocks shall also be made. In addition adequate space shall be provided for hose drying and drill ground. The arrangement shall satisfy the requirement of local fire brigade. Minimum area for this facility shall be 150sq.m.

11.14.08 **Canteen**

Canteen building shall be provided at 2(two) locations as indicated below for all personnel working in the power plant area.



1. Near the Admin Building with dining arrangement for 50 people at one time. This shall have a dormitory facility in the upper floor for 50 personnel (four separate halls for accommodating 5, 10, 10 and 25 personnel). Separate dining area for ladies with capacity of 20 personnel shall be provided.
One hall shall be provided with furniture for conducting meetings, retirement functions, etc.
2. Near the Service Building - The Canteen Building will be able to cater for minimum 300 persons with staggered meal times in four group with an interval of 30 minutes. Seating arrangement of at least 100 persons shall be made. In the upper floor a conference hall shall be provided with A.C. and other facilities as provided for the conference hall elsewhere in this specification. Separate dining area for ladies with capacity of 30 personnel shall be provided.

Both these Canteen buildings shall have the following facilities provided as a minimum requirement

- a) Kitchen
- b) Pantry
- c) Stores
- d) Space for Plate , pot wash and hand wash.
- e) Area for freezer / refrigerators
- f) Staff rooms and toilets
- g) General gents and ladies toilets
- h) Administrator / caretaker room
- i) Dining Hall and service counter
- j) Parking area.

Tables and chairs in the dining hall shall be fixed to the floor and shall have heavy duty stainless steel topping.

Sinks and wash basins shall be of heavy duty stainless steel.

An Air-conditioned VIP Dining room shall be provided for 15 people. General Dining room shall be with Desert Coolers

The building shall be of RCC framed structure and flat roof with brick masonry as side cladding. Architectural details shall be as per specification.

11.15.00 MISCELLANEOUS PLANT BUILDING

11.15.01 Air Washer Room & Compressor House

This shall be a single/double storied steel framed building adjoining to the Turbo generator bay. Roof shall be of RCC supported on colour coated trough shaped permanent decking supported on steel beams. Side cladding shall be by brick supported on concrete encased steel beams. Internal wall surfaces shall receive cement mortar plastering mixed with water proofing compound. Water tank and RCC supporting structures for louvers, filter and foundation for equipment such as fans and pumps are also included in the scope. Proper drainage of the floor has to be provided for. Doors shall be of flush welded steel and they shall be made airtight. Rolling shutter of adequate size should be provided for taking the equipment inside and truck with compressor. The



roof framing shall support an underslung crane of adequate capacity. Compressor foundation shall be isolated from the grade slab. Air receiver and Air drier can be supported from RCC grade slab. All trenches shall be sloped towards drain sumps. Trenches shall be provided with chequered plate cover. Steel glazed windows shall be provided for ventilation and natural lighting. A minimum of 2 flush type steel door shall be provided for movement of personnel. Dimension of the building shall be decided by the CONTRACTOR to suit the dimension of the equipment he is supplying duly taking into account maintenance requirements.

11.15.02 Diesel Generator House

This shall be a single storied RCC framed structure with electric hoist. The dimension of the building shall be decided by the CONTRACTOR to suit equipment supplied. D.G set foundation shall be isolated from the floor.

All trenches shall be of RCC and shall be designed as water retaining structure. There shall be no entry of cables / pipes to these trenches below grade level from outside. All these trenches shall be covered with chequered plate over steel framing. Adequate number of sumps shall be provided to drain these trenches.

11.15.03 Pipe racks and cable racks

All cable and pipe routing in out lying area shall be clubbed and shall run over ground on structural steel 'pipe / cable' racks at a height not less than 4000 mm clear above grade level. Where the pipes cross roads / railway clear headroom shall be 8500 mm. The racks can be multi-tiered. Cable shall normally be laid above the pipes.

Pipe racks/cable racks shall be of structural steel frame and longitudinally braced and shall be designed for worst combination of loadings. Expansion provision shall be provided wherever there is a change in direction or where length of the rack exceeds 100 metres.

Pipe racks for all connections from the existing station, existing pipe racks is to be used with necessary modification (or) new rack is to be erected.

All pipe & cable racks shall be provided with minimum 750mm wide walkway.

Necessary access ladders and platforms to be provided for maintenance. Where so required chequered plate platforms shall be provided for maintenance purpose. Pipe rack columns shall be supported on RCC foundation with bottom of base plate 300mm above adjoining ground level.

11.1600 FUEL OIL HANDLING SYSTEM

Unloading pump house to unload oil from road tankers shall be of single storey structural steel frame structure with GI sheet roofing and Brick walls. A paved platform shall be provided between the pump house and the road tanker parking area with plain cement concrete over rubble soling, sloped towards a peripheral drain. The drain will be led to a sump with oil water separator. No cable trenches will be permitted in the pump house.



Road level of fuel oil handling area shall be decided carefully so that oil from tanker can flow to unloading house under gravity flow only.

Fuel oil pressurisation pump house will house fuel oil pumps, heaters, operator room, toilet and MCC's. This is a single storeyed RCC framed structure with RCC roofing. The pumps will be located in an R.C.C basement. Protective hand rail shall be provided all round the basement. MCC shall be located at plinth level. Proper access shall be given to the basement. Basement shall be given proper drainage facility. The oil / water mixture collected in the sump will be led to an oil water separator. The basement shall be designed as a water retaining structure. Electric hoist crane or monorail facilities will have to be provided in all the pump houses as specified in the Mechanical section of the specification.

Fuel oil tanks shall be of structural steel designed as per IS:803. Foundation of the fuel oil storage tank shall be provided with bitumen sand mixture laid over confined river sand bedding formed with RCC ring beam. The type of foundation to be adopted will depend on the soil condition. Where soil is very poor, tank may have to be founded on concrete pad resting on piles or the bearing capacity will be improved using ground improvement technique.

The storage tanks shall be surrounded by RCC wall to prevent the spread of fire or leakage of oil from a leaking tank to other areas. The 750 mm wide walkway shall be provided on the dyke wall all-round. The height of the wall, c/c spacing of tanks as well as area provided within the concrete wall are to be decided based on the statutory requirements for the type of oil stored and the quantity of oil stored. The entire area outside the tank foundation and within the tank farm shall be paved with concrete. The paving shall slope towards RCC peripheral drains which shall lead to an oil water separator. RCC crossovers shall be provided at appropriate locations for entry of personal to the tank farm. Fuel oil storage area shall be protected by chain link fencing as in the case of Transformer yard. Extent of fencing shall be to satisfy statutory requirement. Foundations for trestles and pedestals for supporting the pipes, anchor blocks etc shall provided in RCC at appropriate locations.



12.00.00 REINFORCED CONCRETE CHIMNEY**12.01.00 Scope**

This specification covers the general requirements for design and construction of reinforced concrete twin flue chimney of circular cross section including RCC shell, steel flue, internal platforms, staircase, appurtenances, fixtures, fittings, conduit and other embedment, lift (inside windshield), natural ventilation etc. complete.

12.02.00 Codes and Standards

- | | | | |
|----|--|---|---|
| a) | IS:456-2000 | : | Code of practice and reinforced concrete |
| b) | IS:4998 | : | Criteria for design of reinforced concrete chimneys – Part 1 Design Criteria -1975 |
| c) | IS:4998 | : | Criteria for design of reinforced concrete Part 1 Assessment of loads - 1992 |
| d) | IS:875
(All Parts) | : | Code of Practice for Design loads for buildings & Structures |
| e) | IS:1893-2002
(Part-1)
Fifth revision | : | Criteria for earthquake resistant design of structures |
| f) | IS:1893-2002
(Part-4) | : | Criteria for earthquake resistant design of structures: Part 4- Industrial structures including Stack like structures |
| g) | IS:432 (Part 1) | : | Mild steel and tensile medium steel bars |
| h) | IS:1786 | : | Cold twisted steel bars for concrete reinforcement |
| i) | IS:800 | : | Code of Practice for general building construction steel. |
| j) | IS:158 | : | Ready mixed paint, brushing, bituminous, black. lead free, acid, alkali, water and heat resisting for general purposes. |
| k) | IS:1239 | : | Mild steel tubes, tubulars and other wrought steel fittings, Part-1 – Mild steel tubes |
| l) | IS:1904 | : | Code of practice for design and construction of foundations in soils : General requirements |
| m) | IS:2062 | : | Hot Rolled low, medium & high tensile structural steel. |
| n) | IS:3043 | : | Code of practice for earthing |



- o) IS:3346 : Method for the determination of thermal conductivity of thermal insulation materials (two slab guarded hot plate method)
- p) IS:3677 : Un-bonded rock and slag wool for thermal insulation.
- q) IS:8183 : Bonded mineral wool.
- r) ASCE-1975 : Design and construction of steel chimney liners. Task committee on steel chimney liners, Fossil Power Committee, Power Division, ASCE.
- s) IS: 6533 : Design construction of steel chimney- code of Practice
(Part1 &2)
- t) Vickery, B.J “Wind Induced Loads on Reinforced Concrete Chimneys” Paper presented at National Seminar on Tall Reinforced Concrete Chimneys, 25-27 April, 1985, New Delhi.
- u) Vickery, B.J. and Basu, “The response of reinforced concrete chimneys to vortex shedding”, R I Journal of Engineering Structures, 1984, Volume-6.
- v) Manohar, S.N. “Tall Chimneys – Design and Construction”, Tata McGraw Hill Publishing Company limited, New Delhi.
- w) Pinfold, G.M, “Reinforced Concrete Chimneys and Towers”, View Point Publication, cement and concrete association, U.K.
- x) The requirements of department of Civil Aviation, Government of India.
- y) Reference may also be made to ACI:307-1979 “ Specification for design and construction of RC Chimney”, if some items are not covered in India Codes.

12.03.00 Design Parameters and Requirements

12.03.01 General

- a) General parameters of the chimney:
 - 1) Total height of the chimney : 275 M
above grade level
 - 2) Number of flues : 2
 - 3) No. of boilers : 2
 - 4) Exit velocity of gas at top :
 - 5) Density of gases :



- | | | | |
|-----|--|---|--|
| 6) | Volume of gases | : | |
| 7) | Temperature of flue gases | : | |
| 8) | Inlet duct center line elevation | : | As per Boiler manufacturer's drg. |
| 9) | Inlet duct opening dimensions | : | As per Boiler manufacturer's drg. |
| 10) | Internal diameter of flue | : | As per design. |
| 11) | Maximum ambient temperature | : | |
| 12) | Minimum ambient temperature | : | |
| 13) | External diameter of windshield :
at top of raft (min.) | : | |
| 14) | Minimum top shell thickness | : | 400 mm |
| 15) | Minimum bottom shell thickness: | : | As per design requirement,
but not less than 950mm. |
| 16) | Chimney foundation type | : | |
| 17) | Flue gas pressure at chimney
entry level | : | |

12.04.00 Grades of concrete and steel

Concrete grade to be used for foundation and shell shall be M30 (minimum). Cement content in concrete shall not be less than 400kg/m³.

Cement to be used for pile cap and below grade level shall be sulphate resistant cement with C₃A content 5 to 8%. For above grade level 43 grade ordinary port land cement shall be used. Only one grade of cement shall be used throughout the height of shell.

HYSD bars of grade Fe 415/ Fe 500 bars conforming to IS 1786 shall be used as reinforcement. All the reinforcement bars to be used for below grade and above grade structure shall be fusion bonded epoxy coated only.

Structural steel sections to be used for flue shall be corrosion resistant steel type "COR-TEN-B" having an yield stress 250MPa conforming to IS 2062 (E250) of minimum thickness 10mm unless noted.

12.05.00 Design Criteria

12.05.01 Dead Load



All permanent loads due to the weight of chimney shell, internal platforms and lining supported on them, ladders, flue ducts, staircases, other accessories etc.

12.05.02 Imposed Load

- i) Imposed load on service platform around Chimney shall, shall be taken as 500 Kg/m². Design live load during construction / erection shall be considered as 1000 Kg/m².
- ii) Imposed loads from duct joining the Chimney shall be considered.

12.05.03 Wind Load

Wind load calculation will be done as per IS:4998 (Part-1) and IS:875 (Part-3) Dynamic analysis will be carried out and stability ensured under such condition.

Wind parameters to be considered for calculating design wind speed and pressure shall be as follows.

Basic wind speed = 50 m/s (upto 10m above normal ground level) as per IS:875 (Part-3) : 1987.

K1	=	1.08
K2	=	(Table 2 & Table 33, Category = 1, Class-B)
K3	=	1.0

A provision of 10% increase in the wind load forces (due to dynamic interference effect) as calculated based on relevant codes, shall be considered in the initial designs. However, for final designs, the increase can be as per the recommendations of the agency carrying out wind tunnel studies.

12.05.04 Seismic / Earthquake Load

Earthquake forces acting on the Chimney and analysis for the same shall be carried out as per IS: 1893-2005 using the Response Spectrum Method and 5% damping shall be considered for the analysis. The analysis shall be furnished for a combination of five modes.

12.05.05 Thermal Effect

Due to the effect of temperature gradient ΔT , vertical and circumferential stresses are developed. These stresses induced in the concrete shall not exceed values given in IS:4998.

Flue gas temperature at inlet / exit 127⁰C (approx.)

The temperature gradient ΔT across the shell thickness of windshield shall be calculated as per IS: 4998 (Part-I) – 1975 but subject to a minimum of 20⁰C. Temperature stresses will be calculated according to the procedures given in ACI-307 and IS: 4998.

12.05.06 Local Loads

Effect of local loads such as moment produced by corbels, platforms, ovaling, oscillation and thermal gradient in addition to other if any shall also be considered.



12.05.07 Load Combination

Various load combination for calculation of stresses shall be as under.

- a) Dead load.
- b) Dead load + Wind Load
- c) Dead load + Earthquake forces.
- d) Dead load + Temperature effect.
- e) Dead load + Wind load + Temperature effect.
- f) Dead load + Earthquake force + Temperature effect.
- g) Circumferential stresses due to temperature effect.
- h) Circumferential tensile stresses due to wind inducing ring moment.
- i) Circumferential compressive stress due to wind induced ring moment combined with temperature.

In Load combinations (a) to (f) above, dead load considered shall be with or without the weight of steel lining for flues & platforms, whichever condition is more critical shall be adopted for design. Across wind loads shall be combined with co-exiting along wind loads. The combined design moment at any section shall be taken as SRSS of the moments due to across wind loads and co-exiting along wind loads.

12.06.00 Permissible Stresses for Chimney Shell

The Stress in Steel reinforcement and concrete shall not exceed the limits as prescribed in clause 7.0 of IS:4998 (Part-1) – 1975 for various combination of loads. Except for case of (dead load + wind load), the maximum permissible stress in concrete in this case shall not exceed $0.275 F_{ck}$.

Where F_{ck} – Characteristic compressive strength of concrete

12.07.00 Analysis**12.07.01 Wind Analysis**

The “gust factor” will be calculated according to the method given in IS:4998 (Part-I) – 1992. Dynamic modulus of Elasticity of concrete as recommended in IS:4998 (Part-1) – 1992 will be used for calculating the natural frequencies of the chimney.

C_d will be taken as 0.8 for the concrete shell in general. It will be increased to 0.96 as per IS:4998 over the portion covered with strakes, if required.

Along wind response of chimney shall be calculated both by gust factor method in A-5.1 and simplified method in A-4.1 of IS:4998 Part-1, 1992. For design, higher of the along wind loads shall be used.

The across wind response of the Chimney will be evaluated as per IS:4998 (Part-1)/ACI307-98. Whether strakes will be provided or not will be decided only after wind tunnel test results.

12.07.02 Ring Moments Due to Wind

The circumferential ring moment due to wind will be calculated in accordance with clause 5.4 of IS:4998 (Part-I) – 1992. The wind induced stresses in concrete and steel shall be calculated in accordance with Cl. No. D-2.2.7, D-2.2.8 and D-2.2.9 of IS:4998 (Part-I) – 1975.

12.07.03 Seismic Analysis

The Seismic Analysis will be carried out using the Response Spectrum Method according to IS: 1893-2002 taking the first 5 modes of vibration in the account.

12.08.00 Component Design Criteria

12.08.01 Shell

The design conditions for the concrete shell shall be as follows:

- 1) The concrete shall be designed for natural phenomena and loads and loads combinations as specified in CL-18.5.7. Working stress method shall be used for design of shell. The modular ratio shall be calculated as per Annexure – B of IS-456 – 2000.
- 2) The permissible stress in concrete and steel reinforcement shall not exceed the specified in CL-7.0 of IS:4998-1975 for various load combinations. Permissible stresses in concrete shall be taken as .275F_{ck} for the load case (dead load + wind load) against the value specified in CL-7.1.1(a), IS:4998 (Part-1)-1975. The stress in concrete at the junction of shell and foundation will be limited to 0.275F_{ck} where F_{ck} is the characteristic compressive strength of the weaker concrete between shell and foundation.
- 3) The thickness of RCC shell shall be provided as required by stress calculations. However, the thickness of the shell has also to be determined on the basis of wind tunnel model studies and any increase, in the thickness of shell or quantities required to be made as per report shall be followed without any extra cost.



- 4) Stresses in the shell shall be checked at 10metre intervals along the height of the shell or at every corbel location or whichever distance is less these stresses shall be within permissible limits.
- 5) The maximum deflection at the top of the chimney for both static and dynamic cases shall not be more than $H/500$ where H is the total height of the windshield above top of the pile cap.
- 6) The dynamic modulus or elasticity of concrete for various concrete grades shall be taken as lower values in the range of values specified in IS:4998 (Part-I) – 1992.
- 7) The static modulus of elasticity of concrete shall be taken as under.
 $5000\sqrt{f_{ck}}$ for instantaneous loadings. Where f_{ck} is the characteristic compressive strength of concrete as per clause 6.2.3 of IS: 456-2000.
- 8) Minimum thickness of the shell at top shall be 400mm.
- 9) Reinforcement in the shell shall be provided as per IS:4998 (Part-I) – 1975. Minimum 0.125% reinforcement shall be provided on either face and on either side of foundation. However, the maximum spacing of reinforcement shall not be more than 250mm both ways in the shell and 300mm for foundation pile cap.

While providing vertical reinforcement steel in the shell, the total number of vertical bars shall be continued till such height when alternative bars can be discontinued. However, reduction of bar diameter along the height is permissible. At any section of the shell vertical bars shall be uniformly spaced. Non uniform spacing of vertical bars is not acceptable.

One third of the vertical bars can only be lapped at one section.

However, for a height from the top equal to half the shell outer diameter or 3 metres whichever is more, the quantity of circumferential reinforcement shall be twice that arrived at as stated above due to design considerations.

Circumferential reinforcement shall be placed around the exterior of and securely wired or welded to the vertical bars at an interval of not more than 600mm.

- 10) Openings in the shell shall be provided for ductworks, access doors, ash channel and ventilation system etc. The maximum width of opening shall be limited to an angle of not more than 30° subtended at the centre of the concrete shell.

The total plan area of the openings at a particular section shall not be more than 15% of the plan area of concrete shell at that location. The opening size for the purpose of stress calculations shall be taken as 1.1 times the actual width of the opening. The extra reinforcement around opening shall satisfy the requirements given in the following documents and the highest shall be provided.

- a) IS:4998 (Part-I)
- b) ACI 307.



- c) Reinforced concrete chimney and tower by M.G. Pinfold. The value of K1 as given in the book by M.G. Pinfold on page no. 186 shall be taken as 0.11.
- d) Minimum half number of extra horizontal bars in shell around the openings to continue for complete circle all round for both faces and both sides.
- 11) Embedments to support the stair case shall be provided in the shell
- 12) Expansion anchors shall be used to attach conduit, lightning and protection equipments, lighting fixtures and other lightweight appurtenances.

12.08.02 Foundation

The chimney foundation will be resting on piles. A circular pile cap preferably annular shall be provided. The pile cap (min. M-30) shall be designed as rigid member with uniform thickness and no tapering of thickness shall be allowed. Minimum thickness of pile cap shall neither be less 0.09 times the diameter of pile cap or 0.4 times the overhang of the foundation beyond shell whichever is greater. Foundation diameter to depth ratio shall not exceed 12. Minimum reinforcement shall not be less than 0.12% in either face and in each direction. Wind and earthquake shall be treated as normal load and no enhancement of stress is permitted on this account in soil, concrete and steel. No tension will be allowed under the pile cap during earthquake and wind. Working stress method shall be adopted for design of foundation. Foundation will be designed for SRSS of moments of along wind response and across wind response.

12.08.03 Steel Liner (Flue)

1. General

Liners shall essentially be constructed from structural steel and shall be of the hung type (i.e. of tension type). The liners shall be provided with externally wrapped thermal insulation. The portion of the liners projecting above the chimney roof shall be constructed of stainless steel. Stainless steel liner shall commence immediately above the flue supporting platform but below the roof supporting platform.

The liner shall be of corrosion resistant steel type "COR-TEN B" of minimum 10 mm thick. Top 10m length or length equal to 2 times flue diameter whichever is larger shall be provided using material confirming to AISI:316L or BS:1449. The liner shall be supported atleast at five different levels, and restrained laterally at several levels, with a small length at the bottom near breach elevation supported from the bottom with a suitable expansion compensator in between.

The structural steel transition inlet ducting shall be bottom supported. The transition ducting shall be suitably profiled from a rectangular shape at the chimney inlet to a circular shape up inside the chimney where it shall be connected to the suspended circular steel liners through suitable (non-metallic) fluoroelastomeric fabric expansion compensator.

2. Design of Steel Liners



Steel liners shall, in general, be designed meeting the requirements of the document, "Design and construction of steel chimney liners", prepared by Task committee on steel chimney liners, Fossil power committee, Power division published by ASCE-1975.

The flue diameter shall be so sized to ensure that the flue gas exit velocity is of the order of 20 to 25 meters/second at the normal continuous operating load. It should be ensured that the flue gas exit velocity at the lowest continuous unit load is high enough (of the order of 15 meters/second) to enable adequate dispersion of the flue gases. For this purpose, 100% turbine MCR condition with design coal firing shall be considered as normal continuous operating condition, and 60% turbine MCR condition with design coal / worst coal firing (whichever yields lesser flue gas quantity) shall be considered as the lowest continuous load condition.

The supporting / restraining arrangement of the liners should be such that expansion of the liners longitudinally or circumferentially is not restrained.

Clear space between flues in multi-flue stake shall be 1000mm after insulation is installed.

3. Insulation

(a) On exterior surface of flue.

The flue shall be insulated externally. The insulation shall be semi-rigid, resin bonded type, in the form of slabs and shall conform to IS:8183. Blanket type insulation shall not be used. The density of insulation shall not be less than 64 kg/cum for resin bonded rock wool. Maximum coefficient of thermal conductivity shall be 0.062 Kcal/m/hr/ $^{\circ}$ C at a mean temperature of 150 $^{\circ}$ C.

The insulation thickness shall be determined based on the maximum ambient temperature, surface air velocity worked out based on the draught of ventilation air in the annular space between the flue liner and chimney shell, insulation surface emissivity of 0.3 and the insulation cold face maximum temperature not exceeding 60 $^{\circ}$ C. The draught of air in the annular space shall be the natural draught by the heating of air by the flue liner and the air being vented out through the openings in the chimney shell. The increase in the annulus air temperature due to the rising heated air shall be taken into account while calculating the insulation thickness. However, a minimum of 50 mm thick (2 layers of 25 mm) insulation shall be provided on the external face of flue.

The insulation shall be tightly secured to the exterior surface of the liner by impaling them on studs welded to the surface at 450mm c/c both horizontally and vertically. The studs shall be galvanized plated and be of a minimum thickness of 10 gauge and 75 mm wide. The studs shall extend a minimum of 25 mm beyond the thickness of insulation. Circular or square metal plate speed washers of standard thickness shall be placed on the extended portion of the studs to hold the impaled insulation material well in place. Further, 20 gauge galvanized wire mesh with a 25 mm hexagonal pattern conforming to IS:3150 shall be wrapped around. Where the wire mesh is jointed, a minimum 150 mm overlap shall be provided. The mesh shall be bound and tied in place with a 16 gauge GI wire at 300 mm centers. Any form of lacing the mesh fibres together will not be permitted.



Insulation for the exposed portion of flue at the top shall consist of 6 layers of insulation material each of a minimum thickness of 25 mm and all joints shall be staggered. The material shall have a minimum density of 200 kg/cum. This shall be protected from the elements by means of a stainless steel cladding, flashing and hood of grade 316 L stainless steel.

Testing of insulation material to satisfy the specific requirements and properties as outlined in this specification IS: 8183 and in the relevant drawings by the contractor shall be carried out.

Insulation materials shall be added to walls where necessary to reduce cooling loads. Also, insulation shall be used for sound absorption on walls which enclose equipment that has been determined as generating excessive noise. This sound insulation shall be provided in interior concrete block, sound block, or insulated metal wall panel liners as appropriate. The liner panels shall be as described above, except they shall include perforations and insulation enclosed in plastic bags. The overall noise Reduction Coefficient shall be 0.80 or better.

(b) Load Bearing and Side Restraints of Flues

Load bearing insulation assembly to have (i) a properly machined mild steel plate with recess at its top for seating PTFE (Poly Tetra Fluoro Ethylene) sheets conforming to BS:5400 (ii) saddle plate (MS) in the middle having stainless steel plate fixed at its bottom surface and lead / elastomeric sheet at top, and (iii) top plate formed of two numbers insulation blocks each made of minimum 50 mm thick rigid, non-combustible asbestos fibre reinforced lime-silica board (SINDANYO BLOCKS NATURAL GRADE CS-51) or equivalent bonded to mild steel plates at top and bottom. For side restraints assembly of insulation blocks of SINDANYO Natural Grade CS-51 or equivalent and stainless steel plate shall be used. All stainless steel in these assemblies shall conform to AISI-316L and mild steel to IS:2062. SINDANYO BLOCKS or equivalent shall be suitable for operation at 320°C and shall primarily satisfy the following physical properties:

- i) Minimum compressive stress prior to onset of compression yield of not less than 12 N/sq.mm.
- ii) Minimum shear strength of 30 N/sq.mm when tested in accordance with BS:3497-1979.
- iii) Thermal conductivity shall not exceed 0.67 W/m Deg.C at a mean temperature of 200°C and its coeff. of linear expansion not to exceed 1.2×10^{-5} per Deg.C.
- iv) Adhesive used for bonding purposes shall be of material with equivalent high temperature properties as approved Foundation Engineer. It may be of "Fortafix Fiborclad Adhesive" as manufactured by Fortafix Ltd., England or equivalent.

The insulation thickness shall be determined based on the maximum ambient temperature, surface air velocity worked out based on the draught of ventilation air in the



annular space between the flue liner and chimney shell, insulation surface emissivity of 0.3 and the insulation cold face maximum temperature not exceeding 55°C. The draught of air in the annular space shall be the natural draught created by the heating of air by the flue liner and the air being vented out through the openings in the chimney shell. The increase in the annulus air temperature due to the rising heated air shall be taken into account while calculating the insulation thickness. The insulation thickness shall not be less than 100 mm, in any case, and shall be provided in two layers with the second layer of insulation covering the joints of the first layer. The insulation shall be wrapped on the outer most surface with galvanized wire mesh.

4. Liner Hood:

The liner hood provided at top of flue shall be fabricated from 6mm thick (minimum) stainless steel sheets of grade 316L. The hood shall completely cover the annular area packed with insulation material between the stainless steel flue and cladding. All sections of the hood shall be anchored in places with stainless steel bolts / nuts. Slot holes shall be provided to make allowances for differential expansions / movements.

12.08.04 Internal Platforms

Internal platforms shall be provided (minimum 5 numbers excluding roof platform) unless indicated otherwise in Data Sheet-A.

The platform shall be supported on a grid work of structural steel beams supported from the wind shield. Topmost grid work shall support RCC roof slab, where as other grid work shall support M.S. gratings as required from functional requirement. Corrosion allowance of 2 mm (minimum) shall be kept in the design of girders. Live load for the design of platform shall be 5 kN/sqm.

The members of the beam grid shall be assembled by high precision bolted connections only. The member dimensions shall be made to suit the as built dimension within the wind shield. The beams shall be painted with 2 coats of zinc silicate primer and 2 coats of heat & acid resistant epoxy paint of approved brand and shade after surface preparation as per IS:1477.

The support of the beams on the R.C.C. shell shall be so designed that only vertical load is transferred to the wind shield without any temperature effect and local torsional / local bending moment.

After fabricating, the members of the individual platforms shall be pre-assembled at ground level and checked for trueness with respect to dimensions and orientation.

The gratings shall be fabricated from M.S. flats comprising of 40x5 thick bracing bars spaced at 40 c/c and 25x4 thick space bar. The grating shall be hot dip galvanized.

Handrails shall be provided with kick plates 65x8 mm along the platform edges. Openings for the elevator shall also be surrounded by a handrail as above, with a hinged gate section on the elevator door side only. The same arrangements shall be provided at all stair case landing/ladder access points, stopper plates shall be provided to prevent gates from swinging outwards. 32mm galvanized steel pipe posts at not more than 1500mm spacing shall be provided for handrail. Height of hand railing shall not be less



than 1250mm. There shall be minimum of three handrails at 450mm, 850mm, & 1250mm above platform level. 32mm dia G.I. drainage spouts shall be provided in the platform for drainage of water.

Maximum deflection of main plate girders supporting flues and gratings shall be restricted to L/600, however maximum deflection for secondary beams supported on main girder shall be L/325. The beams shall be provided with heat and acid resistant epoxy paint and as such no corrosion allowance will be considered.

12.08.05 External Platform

External platform if required, shall be provided as per directorate of air routes and aerodromes (DARA) circular for locating aviation warning lights. The platforms shall be equally spaced at a spacing of 40m (maximum)

The minimum clear width of platform shall be 1200mm and a minimum live load of 500 kg per m² shall be considered for design in addition to dead loads and incidental loads.

Hand railing shall be provided all around external platforms including hood platforms using 32 NP GI pipes. The spacing of railing of posts shall not be more than 1500mm center to centre. Height shall be 1250mm. There shall be 3 hand rails at about 450mm, 850mm & 1250mm respectively above platform level.

32mm GI drainage spouts shall be provided in platforms for drainage of rain water.

12.08.06 Design of Transition Ducting

The number, size and location of flue opening in the shell shall be as per the requirement of boiler supplier. The Contractor shall make arrangement for the proper support of ducting on the shell and provision of restraint / support arrangement as required.

The CONTRACTOR shall be responsible for furnishing, fabricating, shop painting and delivery of ducting which shall run from the chimney liner to the flange of the boiler vendor's duct including all necessary auxiliary ducting as well as soot collecting hoppers located outside the chimney shell. Soot hoppers shall be lined with SS grade AISI 304. Suitable flange connection shall be provided at the exit of the hopper to provide SS / CI pipe connection to drain ash / condensed acid mixed water.

The duct work profile and the guide vanes shall be so configured and sized to achieve the desired flue gas flow characteristic and to minimize flue gas pressure losses.

The plate thickness of the ducts shall be arrived at from minimum (i.e. codal) requirements, structural considerations and corrosion allowances. Material of construction shall be COR-TEN steel conforming to IS:2062. Minimum thickness shall be 10 mm.

The duct work shall be insulated with 75 mm thick insulation (3 layers of 25 thick sheets) as per requirements of clause 18.11 and protected with aluminium foil of minimum 1mm thickness. Access doors shall be provided for the ducting where required for inspection and cleaning.



The duct work and its supporting structures shall be designed for the most onerous of the possible combinations of gravity loading (accounting for ash accumulation), seismic loading, wind loading, flue gas pressure loading and thermal loading.

12.08.07 Expansion Compensator

The suspended portion of the liner shall be connected to the bottom supported portion of the liner by an expansion joint. The joint shall be able to compensate for the large thermal movements of the steel flue, gas tight, acid resistant, heat resistant and provide an adequate insulating medium to avoid excessive overheating in the access void.

The materials used for the fabrication of the expansion joints shall be suitable for the flue gas conditions specified herein, and shall not deteriorate during transit, site handling, storage and installation.

The expansion joint shall be manufactured by a specialist for similar type of joints. The BIDDER shall furnish outline proposals for the expansion joint during the bid stage along with the manufacturer's qualifications and experience for the EMPLOYER's review. On award of contract, full details of materials to be supplied along with fabrication drawings shall be supplied by the CONTRACTOR for the EMPLOYER'S approval. The CONTRACTOR shall procure the units only after obtaining the EMPLOYER'S approval.

The expansion joints shall be made of flexible fibre. They should be air tight and impermeable and should withstand the maximum serviceable temperatures of the flue gas.

The CONTRACTOR shall furnish in complete an installation, dismantling and maintenance user handbook. A draft of the handbook shall be submitted to the EMPLOYER for his approval well before the installation commences.

The CONTRACTOR shall also supply spare expansion joints complete with instructions for storage, fitting, instructions, spare accessories, tools for installation, etc., to replace faulty / used expansion joints at a later date. The spares should withstand a storage period of not less than 10 years.

The fabric for expansion compensator shall comprise several layers of materials given below in order from the gas side:

- a) Two layers of insulation, each consisting of heavy weight glass cloth impregnated with graphite suspension, having an approximate weight of 1.00 kg/sqm.
- b) A continuous filament glass cloth coated both sides with a fluoro-elastomer having an approximate weight of 1.5 kg/sq.m.
- c) Two layers of PTFE (Poly Tetra Fluoro-Ethylene) film of minimum thickness 0.15 mm each.
- d) A layer of glass felt of minimum thickness 10 mm and an approximate weight of 1.5 kg/sq.m.

12.08.08 Liner Test Ports



Gas sampling ports fabricated from stainless steel with flanged ends shall be provided for liner at platform levels where indicated including proper insulation, blank plates, nuts, bolts, etc. The location, orientation and level of ports shall be as per Central Pollution Control Board regulations. Two opacity measurement ports fabricated from stainless steel pipe shall also be provided in each liner at the same platform level of the gas sampling ports.

Bidder shall provide for the purposes of instrumentation other than the sampling ports mentioned above required number of 38 NB conduits including pull wires, junction boxes and pull boxes, all securely tacked / anchored to internal of shell and S 316 L plates below each pipe insert for fixing of measuring instruments.

12.08.09 Staircase

A structural steel staircase shall be provided inside the wind shield, connecting the grade slab to the topmost platform below the roof slab. The staircase shall be supported by a structural steel framework supported from platform to platform independently of the wind shield. Connection of the structural support to the platform beam shall be so chosen as to permit unrestrained deflection of the platform supporting beam. Clear width of the stair treads inside stringers shall be 750 mm. Tread shall be of 25 mm thick M.S. gratings hot dip galvanized with anti-skid nosing. Tread shall be minimum 250 mm wide. Maximum riser shall be 175 mm. Handrail shall be of M.S. pipe type medium of 32 mm NB as per IS:1161 with toe protection of 65x8 mm flats, the complete handrail being hot dip galvanized. 32mm galvanized steel pipes posts at not more than 2000 mm spacing shall be provided for handrail. Height of hand railing shall not be less than 1250 mm. There shall be minimum of three handrails at 450 mm, 850 mm & 1250 mm above platform level. All structural steel work other than stair treads shall be provided with 2 coats of zinc chromate red oxide primer followed by 3 coats of synthetic enamel painting of approved brand and shade. All field connection shall be by bolting only. The arrangement of staircase shall ensure easy accessibility to elevators. Staircase and its connection shall be designed for a live load of 5 kN/sqm.

12.08.10 Ladder

Steel cage ladder hot dip galvanized shall be provided from the last platform to the roof slab. Stringers shall be of flat 75x10 with a clear distance of 400 mm in between; rungs shall be of 20 mm diameter mild steel rods spaced at 300 mm centres. Ladder stringers shall be provided with suitable stays connected to the platform supporting framework. The ladder and its connection shall be designed for a load of 175 kg at any location.

12.08.11 Lift

Lift shall be provided for the chimney inside the wind shield. The lift shall be of open cage type and suitable for corrosive and dusty atmosphere that generally exists in thermal power stations/industrial complexes. It shall generally confirm to the provisions of IS:1860 and IS:4289 unless otherwise specified.

The lift shall have a minimum carrying capacity of 400 kg at a speed of 0.70 metre per second. The lift shall be of rack and pinion driven service type of approved make.



All the control devices inside the car shall be housed in a dust free and water proof enclosure. Only copper conductor FRLS insulated armoured cable of suitable size shall be included in BIDDER's scope.

Landing at all locations of platforms shall be provided unless otherwise specified in Data Sheet-A.

Construction of guard cage and supply of other accessories required for satisfactory and safe operation of lift shall be included in BIDDER's scope. BIDDER shall also supply and install safety devices such as automatic stop equipment, over speed governor etc. all complete as per Manufacturer's recommendation.

All the embedments for the lift structure, approach platforms at landing levels and the lift supporting structure including lift car shall be hot dip galvanized.

12.08.12

Doors

(a) Clean Outdoors

Steel flue shall be provided with clean outdoor having clear dimension of 650 mm x 825 mm at appropriate location with proper access for operating the door. The edges of the doors shall be of hollow steel construction with inner plate of 8 mm and outer plate of 6 mm with suitable infill of stiffeners. The hollow space shall be filled up with insulation of the type used around flues. In addition around the door openings, removable type of insulation similar to the insulation provided around flue shall be provided. The door shall be of hinged type and provided with locking device in addition to Swivel studs with wing nuts on the remaining 3 edges. The door shall be openable both from inside and outside.

(b) Removable Cladding / Access Door

Opening provided near the base of the wind shield for the purposes of flue can erection shall be closed using a removable type of colour coated galvanized aluminium sheet min. 0.6 mm thick on structural steel framing which also can be dismantled if required. Within the cladding area, an access door of size 2100x1000 mm shall be provided with sill level at 300 mm from paved level. This door shall be similar to hollow steel doors provided in other buildings. The door shall be provided with a minimum two coats of acid and alkali resistant paint conforming to IS:158 type I to give a DFT of 75 microns. The outside surface shall be provided with 2 coats of zinc red oxide primer and two coats of synthetic enamel paint. The door shall be provided with suitable locking arrangement.

12.08.13

Chimney Roof & Roof Drainage

The chimney roof shall be of RCC slab suitably supported on MS beams. The roof shall be sloped towards rain water down take catch pits covered with CI gratings. The roof shall be designed to cater for differential movement between the shell and the liners including circumferential expansion and contraction due to temperature variation. 1200 mm high RCC parapet formed by windshield shall be provided at roof level.

The roof shall be provided with a hatch for access from the flue support platform. The hatch shall have a single leaf door with a minimum thickness of 6 mm to cover a clear



opening of 750 mm x 1000 mm. The hatch shall include the door, metal curb, draft seal, spring latch, hold-open device and all hardware. All joints shall be welded and ground smooth. The curb shall be 300 mm in height with a suitable lip which shall include a continuous neoprene seal strip to make the hatch air-tight when the cover is closed. The hatch shall be openable from both inside and outside and shall be provided with automatic hold open arm, easy one hand release and spring latch. All the items of the door shall be of stainless steel of grade 316 L and then painted with heavy duty acid and heat resistant paint as per requirement of IS:158.

The roof slab and the inside surfaces of the wind shield shall be lined with 20 mm thick acid proof tiles conforming to IS: 4457. Bedding mortar (of average thickness 10 mm) shall conform to IS: 4832 Part I and shall be of potassium silicate base.

Roof slab structure shall be designed to serve as a lifting platform for supporting the weight of flues during erection.

The tiles shall be pointed using a phenolic based resin cement mortar conforming to IS:4832 (Part-2).

The bedding mortar and phenolic based resin mortar should be acid-proof, chemical resistant and water-proof. They shall in particular be resistant to sulphuric acid and hydrochloric acid.

CI/UPVC rain water pipes shall be provided within the interior of the shell to remove rain water collected on the roof. Rain water inlet into the pipes at roof level shall be provided with CI grating. The rain water down take pipe shall be led to a manhole chamber inside wind shield suitably lined with acid / alkali resistant brick lining. From the manhole the drainage water shall be led out of the chimney wind shield through CI pipe to a manhole chamber outside wind shield and from there to Effluent Treatment Plant.

12.08.14 Acid Drains and Manholes

In the event of flue gas condensation within the flues acids may be drained out at the base with the provision of stainless steel pipes connected to the soot hopper outlets in each flue located outside the wind shield.

The stainless steel pipes, bends, bolts, fixing sleeves, collecting sumps, etc., shall conform to relevant IS codes or BS:3605, and shall be procured, supplied, fabricated and erected in position by the BIDDER.

The sumps shall be of adequate size suitable lined with acid resistant bricks and provided with a RC roof and a heavy duty manhole cover. The effluent shall be led by means of stoneware pipe to the nearest manhole of effluent treatment system.

12.08.15 Louvres

Air outlet louvers shall be provided as per the requirement of this specification near the top. The louver fins shall be of Z-shape in cross section and made from anodized aluminium plates of a minimum thickness of 4 mm. The frame work supporting the louver fins shall be made from extruded aluminium sections of minimum thickness 6 mm. The louvers shall be mounted in the form of panels. The fins shall be closely spaced to cut off any driving rain entering the chimney wind shield. All panels must be



approved by the EMPLOYER before installation. To outside face of louvers ST mesh on SST mesh frame work shall be provided.

12.08.16 Enclosure Walls

Reinforced concrete walls shall be provided at grade level around staircase & elevator to make the enclosed space air tight and free from ash & dust for the comfort & safety of personal, who are required to go into the chimney for maintenance and inspection purpose. Height of enclosure wall shall be 10m (minimum) or as per the requirement of client. All materials for the design of walls shall be as per relevant IS codes.

12.08.17 Maintenance Provisions

The outer face of the chimney wind shield shall be provided with adequate number of stainless steel insert plates of grade 316 L at the top to enable fixing and supporting painter's trolleys and other accessories. The insert plates shall be provided with suitable number of threaded holes and nuts welded at the rear end to enable bolting of the assembly when desired.

Similar arrangement shall be made at each platform level but to fix the painting trolley arrangement directly on the platforms for the purpose of painting the internals of the chimney.

A lifting beam shall be provided at platform level to cater for lifting of materials during maintenance and painting. The size and capacity of the lifting beams shall be as indicated in relevant drawings.

12.08.18 Chimney Painting

The entire inside surface of the shell for full height shall be painted with two coats of acid and heat resistant black bitumen paint over a coat of compatible primer as per IS:158 with total DFT 150microns.

The top 50m of the outside surface of the shell shall be painted with 4-coats of acid and heat resistant paint (Polyurethane) over a coat of primer with total DFT 150 microns in alternate bands of signal red and white colour.

The remaining portion of the outside surface of the shell shall be painted with 4-coats of Synthetic enamel paint (IS:5410) with total DFT 150 microns over a coat of primer in alternate bands of signal red and white colour.

12.08.19 Pressurised Ventilation

Pressurised ventilation shall be provided in the annular space between concrete shell windshield and flue to maintain positive pressure with respect to flue gas by means of centrifugal fans, filters, gravity dampers, short ducting etc. Positive pressure shall be not less than 1mmWC. Pressure equalisation chambers with all necessary accessories shall be provided wherever doors / hatches are provided in the shell.

Design, supply and Construction of ventilation system including supply of other accessories, spares etc, required for satisfactory and safe functioning of ventilation system shall be included in BIDDER's scope.



13.00.00 NATURAL DRAUGHT COOLING TOWER**13.01.00 Scope**

This specification covers the general requirements of engineering by the Bidder, of material, design, supply, manufacture, construction, testing and commissioning for satisfactory performance of natural draught cooling tower including all associated appurtenances and equipment as well as electrical works.

13.02.00 Codes and Standards

- | | | |
|----|--------------------------|--|
| a. | IS:383 | Coarse and fine aggregates from natural sources for concrete. |
| b. | IS:432 | Mild steel and medium tensile steel and bars (parts 1 & 2) and hard drawn steel wire for concrete reinforcement. |
| c. | IS:702 | Industrial bitumen. |
| d. | IS:780 | Sluice valves for water works purposes (50-300mm size). |
| e. | IS:1536 | Centrifugally cast (spun) iron pressure pipes for water, gas and sewage. |
| f. | IS:1554 | PVC insulated (Heavy duty) electric cables. (part 1) |
| g. | IS:1785
(parts 1&2) | Plain hard drawn steel wire for pre-stressed concrete. |
| h. | IS:1786 | High strength deformed steel bars and wires for concrete reinforcement. |
| i. | IS:1834 | Hot applied sealing compound for joint in concrete. |
| j. | IS:1838
(part 1) | Preformed fillers for expansion joint in concrete pavements and structures (non-extruding and resilient type: Part-1 bitumen impregnated fibre). |
| k. | IS:2062 | Steel for general structural purposes. |
| l. | IS:2267 | Polystyrene moulding and extrusion materials. |
| m. | IS:3042 | Single faced sluice gates (200-1200 mm size) |
| n. | IS:3589 | Seamless or electrically welded steel pipes for water, gas and sewage (168.3mm - 2032mm). |
| o. | IS:5620 | Recommendations for structural design criteria for low head slide gates. |
| p. | IS:5666 | Etch primer. |
| q. | IS:8112
(Part 1 to 4) | 43 Grade Ordinary Portland Cement |



r.	IS:9862	Ready mixed paint brushing, bituminous, black, lead free acid, alkali, water and chlorine resisting.
s.	IS 12330	Sulphate resistant Portland cement
t.	IS:456	Code of practice for plain and reinforced concrete.
u.	IS:800	Code of practice for general construction in steel.
v.	IS:875 (part 1 to part 5)	Code of practice for design loads (other than earthquakes for building and structures).
w.	IS:1080	Code of practice for design and construction of simple spread foundations.
x.	IS:1343	Code of practice for pre-stressed concrete.
y.	IS:1893	Criteria for earthquake resistant design of structures.
z.	IS:2210	Criteria for the design of reinforced concrete shell structures and folded plates.
aa.	IS:2309	Code of practice for the protection of buildings and allied structures against lightning.
bb.	IS:2395 (parts 1&2)	Code of practice for painting, concrete, masonry and plaster surfaces.
cc.	IS:2502	Code of practice for bending and fixing bars for concrete reinforcement.
dd.	IS:2629	Recommended practice for hot dip galvanising on iron and steel.
ee.	IS:2633	Methods of testing uniformity of coating of zinc coated articles.
ff.	IS:2751	Recommended practice for welding of mild steel bars for reinforced concrete construction.
gg.	IS:2950 (part 1)	Code for practice for design and construction of raft foundation.
hh.	IS:3043	Code of practice for earthing.
ii.	IS:3370	Code of practice for concrete structures for storage
jj.	IS:4558	Code of practice for under drainage of lined canals.
kk.	IS:11504	Criteria for structural design of reinforced concrete natural draft cooling towers.
ll.	IS:12200	Code of practice for provision of water stops at transverse contraction joints in masonry and concrete dams.
mm.	IS:1549 (2004)	Guide lines for improving the cyclonic resistance of low rise house and other buildings
nn.	BS:4485	Specification for water cooling towers. (All parts)



oo. VGB Structural design of cooling towers.
Guidelines

pp. ATC - 105 Specification for Cooling Towers - Cooling Tower
Institute - USA

13.02.01 Wherever the above Standards are in conflict with the stipulations of this specification, decision of the OWNER / OWNER CONSULTANT will dictate.

13.03.00 General

The cooling tower shall be capable of cooling the rated quantity of water through the specified thermal range at the design wet bulb temperature and it shall conform to the other design parameters as stipulated in Mechanical section of this document.

The scope of the Contractor's work for a cooling tower shall include preparation of detailed designs and construction drawings and execution of work including but not limited to supply of all material for foundations, shell, basin, fill with its supporting structure, staircases/walkways/platforms with handrailing, cold water outlet channels including gates, screens with handling arrangement, painting, access doors, water distribution system, aviation warning system, lighting, lightning protection system, with their associated hardware, etc., complete all as required to give satisfactory performance and as stipulated in various clauses in this document.

CONTRACTOR shall submit detailed design calculations and construction drawings to OWNER for obtaining his approval prior to commencement of work at site.

In the event, Contractor has quoted in collaboration with another firm (either Indian or foreign), each and every drawing and design calculations submitted shall bear collaborator's seal and signature indicating their approval. Contractor shall also furnish, along with the offer, back-up guarantee for the performance of the cooling tower from the collaborator.

For NDCT, three dimensional finite element analysis of the entire system consisting of tower shell, column supports, pedestal, pile cap, pile and soil mass shall be carried out in a single step so that effect of soil structure interaction can be taken care.

13.04.00 Grade of Concrete & Steel

Minimum grade of concrete to be used for all the structural elements shall be as specified below:

<u>STRUCTURAL CONCRETE</u>	<u>Minimum Grade</u>
a) Foundations	M30
b) Basin	M35
c) Shell	M35
d) Precast works (Fill Supporting Structure)	M35
e) Diagonal columns	M40



Minimum cement content of all RCC structures shall be 360 kg/cum.

Cement to be used for entire NDCT structure including pile cap is sulphate resistant cement with C_3A content between 5 to 8%.

Materials, design, construction and workmanship of prestressed concrete members shall be in accordance with IS:1343.

Reinforcement shall conform to grade Fe 500/550 as per IS:1786 except noted otherwise.

Steel wires for prestressing shall conform to IS: 1785 (part 1&2).

Fusion bonded epoxy coated reinforcement steel, with coating conforming to IS : 13620 shall be used for entire NDCT structure including pile and pile cap.

Steel for all structural steel works shall conform to IS: 2062.

All exposed steel work shall be protected by hot dip galvanising. The minimum coating of zinc shall be 910 gm/sq.m. and comply with the requirements of IS:2629 and IS:2633. In addition, galvanizing shall be followed by the application of one coat of etching primer conforming to IS:5666 and 2 coats of bituminous paint conforming to IS:9862.

As per IS 456, the environmental exposure condition of “very severe” shall be considered for entire NDCT structure below & above ground level including pile cap & Pile. Also, requirement of table 4 & 5 of IS 456 shall be satisfied depending on exposure condition.

13.05.00 Design Criteria

13.05.01 Wind Load

Basic wind speed as well as factors $K_1/K_2/K_3$ shall be taken from IS:875. Additional intensification factor to be considered due to natural turbulence in the incident wind / induced turbulence owing to nearby tall structures shall be 1.43 or based on wind tunnel studies, whichever is higher. Further cyclonic risk factor as per IS:15498 shall also be considered.

Steady pressure distribution around the shell for the wind loading shall be as per clause 5.1.3 of IS:11504, for cooling towers not more than 120m in height and not more than 100 m in base diameter. The value obtained shall be increased by 10 percent to take into account any geometrical imperfections.

For cooling towers of height more than 120m and/or more than 100m in base diameter, wind pressure distribution shall be as determined by model test in a wind tunnel.

13.05.02 Earthquake Load

The coefficient of horizontal acceleration shall be for the appropriate seismic



zone as per IS:1893. Earthquake forces in all the three directions (x,y,z) shall be considered in the analysis of NDCT.

13.05.03 Dead Load

For assessing the self weight of the structure, the specific weight of the concrete shell shall be taken as 25kN per cubic meter. All other dead loads shall be assessed as per IS:875.

The loadings brought upon the shell by permanent fixings shall be minimised to the greatest extent possible. However, when these attachments are made, their effect upon the structure shall be thoroughly investigated.

13.05.04 Constructional Loadings

Contractor shall be fully responsible for ensuring safety by checking the designs, for all the temporary construction loadings as applicable.

13.05.05 Miscellaneous Loads

Effect on account of the following loads shall also be investigated.

- a) Thermal restraint loads
- b) Subsoil deformation

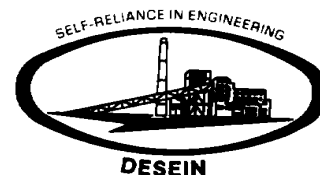
13.05.06 Load Combinations and Permissible Stresses

All structural components except Fill Supporting Structures, Stairs and Platforms of the cooling tower shall be designed by both working stress and limit state method. The following load / load combinations shall be followed in addition to codal requirement:

- a. For working stress method
 - i. $DL + WL$
 - ii. $DL + 1/3WL + EQ$
- b. For limit state method
 - i. $1.5DL + 1.5WL$
 - ii. $0.9DL + 1.5WL$
 - iii. $1.5DL + 1.5EQ + 0.5WL$
 - iv. $0.9DL + 1.5EQ + 0.5 WL$

where DL = Dead Load
 WL = Wind Load
 EQ = Earthquake Load

While considering the above load combinations, the appropriate stress resultants shall be superimposed most unfavourable. Stress due to subsoil deformation, temperature and constructional loads shall also be added to arrive at the most unfavourable load



combination.

Since cooling tower is more a wind predominant structure, increase in permissible stresses as per B – 2.3 Annexure – B of IS 456, shall not be allowed. Increase in safe bearing capacity for design of foundation under wind load combination shall also not be permitted.

13.06.00

Foundations

The design and construction of cooling tower foundations shall be in accordance with the requirements stipulated in IS:1080/IS:2911/IS:2950 as applicable. Continuous foundations shall be provided for cooling towers more than 75 m in height.

The foundation structure shall be designed for loads and load combinations indicated above of this specification and shall in addition consider the following:

- a. Thermally induced local loading where supply culverts pass through the foundation structure without structural isolation.
- b. Cold water basin floor loading.
- c. Surcharge load of 25 KN per sq.metre at the ground / grade level.
- d. In case of subsoil with varying degrees of stiffness, non uniform settlements due to the varying stiffnesses have to be determined. The redistribution of stress resulting from interaction effects between the subsoil and the shell structure have to be taken into account.

For the load combination 0.9DL+1.5WL, uplift of the foundation, not exceeding a sector of 15 degrees is permitted, provided the foundation is a continuous annular raft foundation.

No pressure relief valve shall be provided in any part including foundation & basin slab of NDCT.

13.07.00

Basin and Cold Water Outlet

The basin shall be divided into two compartments to facilitate complete isolation of one half of the basin for the purpose of cleaning and maintenance, while the other half is in service.

The basin floor of each compartment shall be sloped with a slope not less than 1 in 120 towards a collecting sump for effectively draining the water to permit desilting/desludging. To minimise obstructions to the flow of water, only the columns supporting the Fill structure shall be projected above the basin floor. Water shall be drained from the sump into a drain chamber outside the basin by CI drain pipe embedded below the basin floor. Suitable sluice gates of the rising spindle type conforming to IS:3042 or sluice valve conforming to IS:780 shall be provided in the drain chamber. Drainage arrangement from drain chamber to suitable location shall also be made.

A minimum of 300mm free board shall be provided for the basin over the maximum design water level. The basin wall shall project a minimum of 500 mm over the surrounding grade level. Plinth protection of 1 m wide shall be provided all round the



building.

7 m wide paving around NDCT tower from racker column shall also be provided

Each compartment of the basin shall be provided with a concrete channel for outlet of cold water. Cold water outlet channels shall be provided with the stoplogs and screens.

Stoplogs shall be fabricated out of structural steel plate and rolled sections and provided with rubber seals to prevent leakage. Stoplogs shall be hot dip galvanised and provided with etch primer and bituminous paint.

Screens shall be out of 8 gauge 25mm clear opening SS crimped wire netting welded to frame of structural steel section/flats. Frame work shall be hot dip galvanised and provided with etch primer and bituminous painting. Two numbers of screens shall be supplied per tower.

Suitable arrangement of monorails with hoist shall be provided for handling stoplogs and screens.

The basin walls, floor slab and outlet channels shall be of reinforced concrete construction. The design and construction of these water retaining structures shall be in accordance with IS:3370 with provision of construction/contraction and expansion joints. Resistance to cracking shall be checked as per cl. 4.5.2 and cl. 4.5.3 of IS:3370 (PART 2): 2009. Minimum thickness of structural concrete elements shall be 230mm. Basin walls shall also be designed for condition of external surcharge load along with ground water table and basin being empty.

The basin and channel walls shall be designed for a minimum surcharge load of 25kN per sq.m. to allow for construction plant operating in the vicinity.

External pressure due to earth and ground water shall not be relied upon to reduce the effect of the internal water pressure, but account shall be taken of the ground water pressure when considering buoyancy or stresses in the empty water retaining structure.

Construction of the basin and channel shall be watertight with the provision of minimum 230mm wide approved quality PVC ribbed water stops with central bulb of minimum 10mm at all construction joints and expansion joints.

The water retaining structures shall be tested for water-tightness in accordance with IS:3370, without the backfill. Any rectification measures required to satisfy the test criteria shall be executed by the CONTRACTOR at his own cost all as per the directions of the OWNER.

All round Cold water basin, plinth protection in M20, 150 mm thick shall be provided for a width of 3000 mm, sloping away from the basin. Dry rubble soling of 150 mm thick shall be provided below the concrete paving. Suitable drain shall also be provided along plinth protection with a sum at appropriate location.

In any case, the basin slab shall not be used to give lateral resistant to piles below main tower shell structure.



13.08.00 Tower Shell**13.08.01 Shell Geometry**

The shape of the tower shall be hyperboloid in vertical section and circular in plan.

13.08.02 Thickness

The tower shell shall have a minimum thickness of 250mm.

A minimum factor of safety of five (5) against buckling shall be ensured at all levels.

13.08.03 Design

The tower shell shall be designed based on bending analysis and as per elastic theory for thin shells.

Consideration shall also be given to the following effects of temperature and moisture variations that occur in the shell :

- a) The strain resulting from a temperature gradient across the shell thickness.
- b) The strain resulting from moisture content variations through the shell thickness, and
- c) The strain resulting from variations of conditions (a) and (b) caused by rain and sunshine on one side of the tower or partial operation of the tower.

The effect of stress concentration due to any fixtures shall be taken into account.

13.08.04 Reinforcement and Cover

Reinforcement shall be provided on each face of the shell in both directions.

The shell reinforcement of high yield strength deformed bars, in either direction and on each face shall be not less than 0.15% of the concrete cross-sectional area. Further, the circumferential reinforcement on each face shall be not less than 0.20% of the concrete cross sectional area in the top one third height of the shell.

The maximum spacing of reinforcement in either direction on each face shall be restricted to twice the thickness of the shell or 200 mm whichever is less. Further, not more than one third of the reinforcement at any level or section shall be lapped.

Minimum lap lengths in shell shall be 1.3 times L_d where L_d is the development length as described in cl 26.2.1 of IS:456.

Column reinforcing bars shall be carefully anchored in the shell and in the foundations. The minimum anchorage length in the shell shall be $2L_d$ or 80 times the diameter of the bar.



Minimum size of deformed bars used shall be 8mm for circumferential reinforcement and 10mm for Meridional steel.

The concrete cover shall be 50 mm minimum. However, the clear cover shall not be less than 1.5 dia of bars. (for severe exposure condition)

13.08.05 Tolerances

The shell should be constructed within the dimensional tolerances stipulated in clause 7.3 of IS:11504. Permissible allowances for survey inaccuracies while checking of shell geometry shall be as per clause 7.4 of IS:11504.

13.09.00 Fill

PVC splash type bars fill arrangement shall be provided.

The fill shall be adequately supported to minimize sag, possibility of dislodgment and damage as a consequence of induced vibrations in the fill.

The supporting hanger rods/tubes shall be of stainless steel conforming to ASTM 316L.

Density of fill to be considered for the design of fill supporting structure shall be 100 kg/m³ minimum

13.10.00 Fill Supporting Structure

Fill supporting structure with its foundation shall be of reinforced concrete construction.

Fill supporting structure, where formed of precast concrete units, shall be made stable by the use of either bolted, cast-in-situ or glued joints. Members of precast concrete structure shall be either pre-stressed or reinforced concrete units. Where bolts are used for permanent connections, they shall be of a material highly resistant to corrosion or fully protected by embedment or coating.

The columns and beams supporting the Fill shall be adequately braced in all directions. The design of all members shall be checked for resistance to buckling.

The stability of the Fill support structure shall be checked for its capacity to resist a horizontal force of 2% of self-weight, the Fill and supporting structure, assumed to act horizontally in any direction. The structure shall also be checked for the appropriate earthquake loading as applicable.

During erection, care shall be taken to ensure the stability of the potentially unstable, partially completed structure.

The Fill support structure members shall be designed to cater for the likely loads to be imposed thereon, including:

- a) temporary loadings : they are subjected to during handling, transportation and erection at normal design stresses.



- b) loading due to scaffolding for the erection crew if so envisaged to be supported.
- c) loading of personnel during maintenance of pipe work and sprinkler system. The appropriate parts of the pack support structure shall be designed to support a point load of 1.5 kN at any position.

Reinforced concrete members shall be designed for self-weight and permanent loadings by working stress method with allowable stresses limited to 80% of those specified in IS:456.

13.11.00 Maintenance Access Facilities

All areas of the cooling tower requiring inspection, cleaning, repairs and/or adjustment shall be easily and safely accessible by suitable provision of stairs, platforms etc., as per the requirement described herein below :

13.11.01 Stairs

The tower shall be provided with two numbers external RC staircase, leading to a heavy duty door giving access to the distribution system. Staircase shall be minimum 1000 mm wide (clear), with landings of minimum width of 1000 mm at not more than 2500 mm height intervals unless approved otherwise. The steps shall have a rise of about 125 mm and tread of about 250 mm. Anti-skid nosing at each step shall be provided.

All access doors shall be of size minimum 1200 mm wide and 2100 mm height, minimum.

13.11.02 Platforms

Reinforced concrete platform of 1.2m clear width shall be provided around the circumference at the top of the cooling tower on the outside for fixing aviation warning beacons.

13.11.03 Walkways

RCC walkways having a minimum clear width of 1.50 m shall be provided around the circumference inside the cooling tower above the Fill to provide access for inspection and maintenance of all hot water distribution pipes/ducts and nozzles. Walkways shall be supported independent of the Fill structure.

13.11.04 Ladders

Four numbers of hot dip galvanised mild steel rung ladders with etch primer and bituminous painting shall be provided equally spaced around the circumference of the tower.

Safety cage for the ladder shall be provided for the ladders located on the external surface of the tower upto the throat level.



Ladders shall be continued along the inside surface of the tower upto the top of the tower and then on to the top platform. At the throat level a reinforced concrete landing of 1.2m minimum width shall be provided with a hot dip galvanised steel access door for the interchange.

Two of the ladders leading to the platform at the top of the tower shall have approach from the reinforced concrete stairs. Other two ladders shall be directly from the ground level itself.

13.11.05 Handrailings

Handrailing shall be provided on both sides at the edges of platforms, landing slabs, sides of stairs, walkways and cold water outlet channel.

Handrailing shall be of hot dip galvanised construction and shall be out of 32 mm NB pipes of medium class conforming to IS:1239 and provided with etch primer and bituminous painting.

Handrailing shall be 1300 mm high with two intermediate rails at 450mm and 900 mm with the top rail at 1300 mm above the surface of slab/steps. Handrail posts shall be spaced at not more than 1.5 m centres.

A toe plate of hot dip galvanised mild steel of size 65 mm x 8 mm shall be provided for all handrailing.

13.12.00 Painting To Concrete Surfaces

Painting to the concrete surfaces specified herein below shall be with 100% solid high build Polyurethane coating of 2000 microns over compatible primer as per manufacturer's requirement. The prior approval of manufacturer shall be obtained from OWNER for painting of cooling tower.

- a) Basin floor
- b) Internal surfaces of basin wall including the partition wall.
- c) Diagonal columns over complete height.
- d) Fill support columns and beams for the full height and length respectively.
- e) Internal and external surfaces of hot water duct.
- f) Internal and external surfaces of cold water channel upto the terminal point.
- g) Internal and external surfaces of the drain chamber.
- h) Complete internal surface of the shell. For external surface at least 1 metre on the shell at top.

The preparation of concrete surface for painting shall conform to IS:2395(Part-I). For new surface, it is preferable that the surface left unpainted for as long as possible to allow drying. Before painting, the surface shall be thoroughly brushed to



remove all dirt and remains of loose or powdered materials.

All other exposed concrete surfaces shall be given two coat of water proof cement paint of approved make and colour after the surfaces are rubbed down suitably and as approved by the OWNER. All concrete surfaces which are in contact with the earth shall be applied with anticorrosive coat system.

13.13.00 Water Distribution System

The arrangement of water distribution system shall permit a satisfactory distribution of water over the whole area of the tower at all reasonable loads.

Hot water Inlet pipe shall conform to the requirements of Class I of IS:3589 and steel shall conform to IS:2062.

The distribution troughs/pipes shall be independently supported from the structures and shall be easily removable. Provision shall also be made for easy flushing or cleaning of all troughs/pipes.

13.14.00 Lightning Protection

The cooling tower shall be protected against damage by lightning. The installation work shall conform to the requirements of IS:2309.

CONTRACTOR's scope of work includes supply and installation of the complete system upto and including the earth electrodes.

For detailed specification of lightning protection system refer electrical section of this document..

During the construction of tower, temporary lightning protection shall be maintained by connecting the reinforcement bars to two grounding conductors of size 50 mm x 6 mm.

Lighting protection requirement shall be provided as per mentioned specification of vol. IV 4.

13.15.00 Aviation Warning System, Lighting and Power Receptacles

Refer Volume-III (Mechanical Section)

TECHNICAL DATA SHEET- A

13.22.00 Specific Technical Requirements

a) Number of cooling towers 2 nos

13.22.01 1. Design & Operating Conditions

Refer Volume-III (Mechanical Section)



13.23.00 Features

Refer Volume-III (Mechanical Section)

13.24.00 Elevations**13.24.01** Grade level in mTo be decided during
during detail
engineering.**13.24.02** Sill level in m

-do-

13.24.03 Basin depth in m

-do-

13.25.00 Materials of Construction

Refer Volume-III (Mechanical Section)

13.26.00 Structural Concrete (Minimum Grade)

a)	Foundations	M30
b)	Basin	M35
c)	Diagonal Columns	M40
d)	Shell	M35
e)	Precast works (Fill Supporting Structure)	M35

13.27.00 Type of Cement

- a) Below ground
 - (i) Sulphate Resisting Portland cement
with C₃A content 5 to 8% conforming to IS:12330
- b) Above ground
 - (i) Sulphate Resisting Portland cement
with C₃A content 5 to 8% conforming to IS:12330

13.28.00 Primary Loads**13.28.01 Wind load (as per IS 875)**

- a) Basic wind speed 50 m /sec
- b) Factor K1 1.08
- c) Terrain category factor K2
Varies w.r.t. height Category 1



	d)	Factor K3	1.0
	e)	Intensification factor	as per spec.
13.28.02		Earth quake load.	
	a)	Zone as per IS:1893	III
	b)	Importance factor	as per code
13.2900		Foundation Criteria	
		Ground water table	At grade level.
		Type of foundation	Pile
		Safe bearing capacity of soil	As per soil report
		Type of pile	As per soil report
		Pile capacity and termination depth belowground level	As per soil report
		Minimum thickness of basin base slab	250mm
		Pressure relief valves in the basin floor slab	Not to be provided
13.3000		Water Distribution Channel	
		Design Pressure	
13.3100		Performance Test	
		Method	
		Instrumentation	



14.00.00 COAL HANDLING SYSTEM**14.01.00 External Coal Handling System From Coal Berth -3 to Plant Area:**

External coal handling system will consist of conventional conveyor gallery from JTB-3, MCC / Electrical building in NCTPS Stage II. And thereafter pipe conveyor system upto Ennore SEZ.

The pipe conveyor will cross the Buckingham canal, Back water area, railway line, transmission line, oil pipe line & roads before entering Power plant area covering a distance of about 5-6 km.

The pipe conveyors will be supported on cross-over, trestles, piers etc. At the crossing of roads the cross-over should maintain 8.0m clear height from the crest of road and at the railway crossing the cross over should maintain the clear height from the top of rail as per railway norms. Bridge shall be provided for crossing of pipe conveyor, Sea water pipes (CT Make Up & Desal make up) and cables near canal crossing, road crossing & railway crossing.

Considering the declaration of Buckingham Canal & Pulicat back water as National Inland Waterway and Formation of terminal etc., at the crossing of Buckingham canal and Pulicat back water the span of the supporting cross-over and height of the pipe conveyors shall be decided with reference to the Water Resource Department, PWD guideline.

Necessary statutory clearances / approval from Railways / Highway Authority / PWD shall be obtained by Contractor at his own cost.

14.02.00 In-Plant Coal Handling System Necessary For The Proposed Plant:

In-plant coal handling will consist of junction towers (underground and above ground), sampling towers, underground / semi ground tunnels, pent house, conveyor galleries, trestles, MCC room, stacker reclaimer foundation, coal stock pile, stock yard drainage, water sprinkler system, dust extraction / suppression system, pump house, cable tray, cable support, underground cable trenches with RCC cover, rails etc.

14.02.01 Over Ground Transfer Points

Over ground TPs shall have RCC foundations and pedestals at (+) 500 mm above ground level. The superstructure for TPs shall be of steel framed structures with adequate bracing arrangement. The ground floor and all intermediate floors shall be of RCC and will be finished with IPS and metallic hardener having proper slopes for drainage. The roofs shall be of flat RCC slab with water proof treatment and drainage slope. The cladding shall be of permanently color coated zinc alum sheets of minimum thickness 0.6mm TCT. Plinth protection and storm water drain shall be provided around the structure suitably.

Roof and all intermediate floors will be of reinforced concrete minimum 150mm thick of concrete M25 grade. All grade floors will be 200mm thick of M-20 grade concrete with nominal reinforcement of 8 Tor @ 250 c/c top and bottom both ways over PCC, soling over sand cushion of required suitable thickness. However the codal regulations shall also be compiled with. Continuous handrails shall be provided for the staircase with 32 mm NB GI pipe hand rails.



TPs shall be provided with independent steel staircase with steps of MS grating from ground to highest operating floor level. The clear width of the stair shall be minimum 1.0m. The tread shall be atleast 250 mm and riser shall be uniform throughout the height and shall not exceed 180 mm. However the codal regulations shall also be complied with. Continuous handrails shall be provided for the staircase.

Drive units shall be directly supported on floor beams from suitable structural steel stools and not on concrete floors. All transfer houses shall be provided with adequate no. of windows and doors. The window area shall not be less than 10 percent of total wall area. Maintenance platform inside / outside of transfer house may have chequered plate floors with horizontal floor bracing.

Transfer points / crusher house with metal sheeting as cladding shall have 1.5m high 230 mm brick enclosure from finished ground floor around the building. Brick wall shall be inside face of sheeting with a lapping of 150mm.

Adequate floor washing arrangements shall be made in all TPS.

Provision of floor dust collection and removal by chute shall be made.

The Transfer house structure shall be so designed that transverse deflection at places where conveyor gallery meet, should be equal to the respective transverse deflection of conveyor trestles.

Monorail / beams for electric / manual hoists shall also be provided in junction tower wherever required.

Suitable overhead RCC / syntax water tank shall be provided for service / potable water system.

Piling shall be used for junction tower foundation.

14.02.02

Crusher House and Crusher Foundation

The crusher house will be braced structural steel structure with one brick thick (230 mm) upto 1.5 m from ground level and above 1.5 m the cladding shall be of pre-coated colour steel sheet. Floor and roofs shall be of RCC supported on steel beams. Roof will be provided with roof water proofing treatment in line with architectural requirements mentioned elsewhere in the specification. Adequate steel doors and windows shall be provided for natural lighting and ventilation as mentioned for TPs.

The crusher house shall have an independent staircase for general use like other TPs. In addition, it shall have a separate open staircase for fire escape suitably located to comply with fire fighting norms. An open elevator up to the top floor shall also be provided at suitable location at periphery of the building.

The crusher shall be supported on RCC M30 deck slab of designed thickness mounted on vibration isolation system of GERB comprising springs and viscous dampers, which in turn supported on steel girders. The framework of girders shall be an integral part of the building, which houses the machine. There shall be isolation gap between building floor slab and



crusher deck slab. Suitable provision for maintenance platforms for springs and damper shall be provided.

Monorail / beams for electric / manual hoists shall also be provided in crusher house wherever required.

Piling shall be used for crusher house foundation.

14.02.02 Crusher Foundation

Detailed dynamic analysis shall be done for the top deck together with springs and dampers and the natural frequencies and amplitudes of vibration shall be determined. A mathematical model of the top deck shall be formulated with three dimensional beam / plate finite elements for the purpose of analysis with the spring idealized with vertical and horizontal stiffnesses. The mass of the machine together with that of the top deck shall be considered for the analysis.

Natural frequencies up to at least 10% above the operating speed shall be determined and three frequencies shall be checked against the design criteria.

Forced response dynamic analysis shall be carried out for the operating condition unbalance forces using a sinusoidal forcing function. Unbalance forces as given by this specification shall be used for his purpose.

14.02.03 Isolation Efficiency

The vibration isolation system shall be designed for about 95% isolation efficiency.

14.02.04 De-Coupling

A ratio of the least 10 (ten) shall be ensured between the stiffness of the supporting structure and the stiffness of the spring system in the vertical direction of achieve de-coupling between the two (the stiffness of the spring system being lower). This ensures that dynamic analysis of the supporting structure need not be carried out.

14.02.05 Frequency Criteria

The frequency criterion has already been laid down implicitly foundation isolation efficiency criteria and de-coupling required.

The bending mode frequency of the top deck shall be atleast 20% above the operating speed.

14.02.06 Unbalance Forces

Unbalance forces arising out of all the following cases shall be considered for checking the design and amplitudes.

- (a) Balance quality grade Q40 as per VDI 2060-1966.
- (b) One hammer broken condition. The missing hammer shall be assumed to be closest to the crusher non-drive end of the crusher.



- (c) Three hammers broken condition. All the three hammers broken shall be assumed to be from the same suspension bar and located at the non-drive end of the crusher.

14.02.07 Amplitude Criteria

The calculated amplitudes (mean to peak values) shall not exceed following limits under the specified conditions:

1) Operating speed of 750 RPM.

- (a) 150 microns for an unbalance force arising out of balance quality grade Q40 as per VDI 2060-1966.
- (b) 300 microns in case of a one hammer broken condition.
- (c) Amplitudes need not be checked for a three hammer broken condition.

2) Operating speed of 450 RPM.

- (a) 200 microns for an imbalance force arising out of balance quality grade Q-40 as per VDI-2060-1966.
- (b) 300 microns in case of a one hammer broken condition.
- (c) Amplitudes need not be checked for three hammer broken condition.

For intermediate operating speed between 450 to 750 RPM, the amplitude limits can be linearly interpolated.

The amplitude limits mentioned above are in both vertical and horizontal directions. The amplitude shall be calculated at critical points on the top surface of the RCC deck. The amplitudes shall be checked for the most unfavourable superposition of modes in any direction. However, phase difference between the maximum amplitude occurring in different directions due to the rotating vector may be considered while superimposing the modes.

14.02.08 Transient Resonance

Transient resonance, which may occur during the start-up or coasting down condition of the crusher, shall be checked and the amplitudes in such a condition should not exceed one – and half times those at operating speed for each design condition.

14.02.09 Strength Criteria

The following criteria shall apply for the design of top deck:

Dead loads, live loads, seismic loads and dynamic loads shall be considered for the design. The most unfavourable combination shall considered for design.

Seismic loads shall be assumed to act together with dynamic loads for a one millimeter eccentricity in the rotor. However, seismic loads and dynamic loads arising out of hammer



breakage need not be considered together.

Fatigue shall be considered while designing for dynamic forces. A fatigue factor of 2.0 shall be used on all dynamic forces to arrive at the equivalent static force for the purpose of design.

Working stress method shall be used for the design of RCC deck. In survival condition, 10% overstressing may be permitted.

The RCC top deck shall be atleast of M30 grade of concrete as per IS:456-2000.

Fatigue need not be considered for the three hammer broken condition.

For calculating unbalance forces, the heaviest hammer (plain or toothed) shall not exceed the value specified by the manufacturer.

Loss of contact of foundations at founding level shall not be allowed for any combination of loads.

14.03.00 Coal Handling Control / Electrical Building

The control room for the coal handling plant shall be RCC framed structure with brick masonry walls or steel framed structures with brick masonry walls and RCC roof designed to be completely dust proof. Large glazed windows shall be provided for a complete view of the coal handling plant from the control room. Provision for fire escape stair shall be provided in accordance with TAC requirement. The building shall be provided with toilet/urinals at various locations as per owner's requirement. Air conditioning to be provided for the operators / working area suitably. All windows & doors shall be aluminium only.

14.04.00 Foundations of Stacker-cum-Reclaimer

Foundations for Stacker - Reclaimer shall be designed as a continuous wall or RCC framed structures (in longitudinal and transverse direction). Lateral tie beams shall be provided between two rail supporting elements shall be provided at a regular interval of min. 3.0m center. The foundation shall be designed for the most critical combination of loads as prescribed by the manufacture. The foundation shall also be designed for deflection and settlement as required by the manufacturer.

Piling shall be used for Stacker-cum-Reclaimer foundation.

14.05.00 Over ground Galleries & Trestles

Overhead conveyor galleries shall be structural steel consisting of two vertical girders braced at top and bottom and having rigid jointed portal frame at both ends. Common end portal shall not be used for two adjacent conveyor spans. Roof truss shall be provided at upper node points of latticed girder to form an enclosure. The maximum span of standard gallery shall be 24 m unless higher span is required due to the site condition, which shall be subjected to approval of the engineer. The gallery should as far as possible be erected as a box section keeping all the vertical and horizontal bracing tied in proper position. The Gallery should be checked for all erection stresses that are likely to develop during handling and erection and if required, temporary strengthening of gallery members during erection



shall be made.

The galleries for double stream conveyors shall have one central walkway and two side walkways of clear width 1100 mm and 800 mm respectively. Single stream conveyors shall have two side walkways. Both sides of walkways shall be provided with pipe handrails all along the conveyor. Walkway shall be of chequered plate with anti-skid arrangement. Conveyor gallery having slopes greater than 10 degree, stepped walkways of tread 250 mm of chequered plates with nosing and toe guard shall be provided all along the conveyor.

Each conveyor gallery shall be provided with seal plate (throughout gallery length) of minimum 12 gauge thickness. The conveyor galleries shall be designed for adequate ventilation and natural lighting.

Suitable floor washing arrangement shall be made in the conveyor gallery with down comers. Down comer shall lead the water/coal slurry to pit (of 2.0 cum capacity) to allow settling of coal, which will further discharge into RCC drains.

Cross-over with chequered plate platform, ladder and hand railing for crossing over the conveyors shall be provided at approximately every 100 M intervals of conveyor. Cross over shall be preferably located over four legged rigid trestle locations.

Location of trestles shall be decided carefully so that there is no interference with underground and over ground structures, tunnels, trenches, drains etc. However at the location where overhead conveyor gallery crosses road/ rail line, The minimum clearance of 8.0 m above road crest and 10 m above rail shall be provided. Necessary statutory clearances / approval from Railways / Highway Authority / PWD shall be obtained by Contractor at his own cost.

For calculation of coal load on moving conveyor, a multiplication factor 1.6 shall be used to take care of inertial force, casual overburden and impact factor etc.

Thus coal load per unit length of each moving conveyor shall be

$$1.6 \times \frac{\text{Rated Capacity of Conveyor System}}{\text{Conveyor Belt Speed}} \times \frac{1200}{800}$$

It should be noted that for structural design, Unit weight of coal shall be assumed as 1200 Kgs/ Cu M. instead of 800 Kgs/Cu M. considered for system sizing purpose.

Conveyor gallery shall be designed considering both conveyors operating simultaneously.

In between transfer house / buildings four legged trestles shall be place at a maximum interval of 90m. The arrangement shall be such as to ensure that force in the longitudinal direction (i.e along conv. Length) of conveyor gallery of length not more than 90m will be transferred to four legged trestle. Two legged trestles at regular interval may be placed between four legged trestles. The end supports resting on the four legged trestles can have either ends hinged or one hinged and the other on slide type. Slide type support shall be with PTFE bearing to allow both rotation and logitudnal movement.

End of conveyor gallery which will be supported over transfer points, shall be so detailed



that only vertical reaction is transferred from gallery and no horizontal force in longitudinal direction is transferred from gallery to transfer points and vice-versa.

Transverse deflection of trestle due to wind /seismic load shall not exceed trestle height/1000 as stipulated in IS 11592. Longitudinal deflection of trestle shall also be within limits.

All RC trestle pedestals should be raised to (+) 0.5 m above FGL. Trestles shall be of structural steel braced adequately and provided at suitable locations.

Piling shall be used for trestles foundation.

14.06.00

Coal stock yard

Entire area shall be excavated to average 250mm depth and given slope of 1 in 100 in suitable directions as approved. Entire excavated area shall be watered and rammed and compacted with 10 tonne roller. 100mm thick river sand cushion layer shall be placed over compacted subgrade and consolidated by flooding 150mm thick boulder soling shall be laid over sand cushion, joints filled with river sand. Over boulder soling, 75mm thick lean concrete (1:4:8) shall be spread. Final layer shall be minimum 150mm thick RCC slab of grade M20 having double layer reinforcement. Slope is to be maintained through all layers. The coal stack area shall be graded with suitable slope for draining the surface by longitudinal drains shall be of RCC trenches with graded gravel packets at about 1.5m intervals on the sides to trap coal dust. Weepholes shall be provided in the trench walls at the locations of filter pockets for collecting surface water in the longitudinal rains for discharging into the main surface drain to be provided by the Contractor by the side of the road. Retaining wall 1m depth to be provided around the crushed coal pile to prevent coal spillage into the drain. Necessary control joints in the grade slab shall be considered to avoid temperature and shrinkage cracks in the slab. The design of coal stack area should take care of loads arising due to dozer movement and shall be designed to sustains coal water pressure.

Suitable soiling with mortar cover shall be provided between rails for stacker reclaimer and reclaimer machines.

14.07.00

Cable gallery

Cable galleries shall made of tubular steel sections. The tubular steel sections shall be of circular/ rectangular/ square shape. The circular steel tube shall conform to IS:1161 and rectangular/ square steel sections shall confirm to IS:4923. The steel structures using tubular sections shall be designed and fabricated as per IS:806 – “Code of practice for use of steel tubes in general building construction.” And EN 1993-1-8:2005.

14.08.00

Drainage of CHP Area

Suitable drainage arrangement shall be provided for underground structures if any with sump pumps. Coal stockyards and surrounding areas also shall have suitable surface drainage facilities.

14.09.00

Road / Rail Crossings:



Hume pipes of appropriate size of class NP2 shall be provided under the roads and rail tracks for pipe and cable crossings at the required locations.

14.10.00 Loadings

For different loads on structures, latest IS:875 shall be followed. All structures shall be designed for dead load, live equipment load (including impact / vibration), erection load etc. in addition, seismic load (as per IS:1893) or wind load (as per IS:875) shall also be considered.

The following minimum live loads shall be adopted for design of buildings / structures.

- | | | |
|--|---|---|
| a) Flat Roof | : | Accessible roof – 200 Kg/m ²
(150kg/m ² for L.L. & 50 Kg/m ² for dust load) |
| | : | Non-accessible roof–125 Kg/m ² (75 Kg/m ² for L.L. & 50 Kg/m ² for dust load). |
| b) Inclined roof (including roof for Conveyor Galleries) | : | A per IS:875 for live load plus 50 Kg/m ² for dust load to be Considered on plan area. |
| c) All building floors & stairs | : | 500 Kg/m ² |
| d) Toilet rooms | : | 200 KG/m ² |
| e) Walkways of Conveyor Galleries | : | 300 Kg/m ² (including Coal dust load) or a concentrated load of 300 Kg. at centre whichever is critical. |
| f) M.C.C. Floor | : | 1000 Kg/m ² |
| g) Equipment Load | : | As applicable |
| h) Tunnels under roads, culverts per & its allied Structures including hume pipes. | : | Class “A” or class “70R” as IRC Standard whichever is higher or load due to bulldozers. |
| i) Underground basement tunnel trenches etc. | : | A surcharge pressure of 2T/m ² . |
| j) Tunnel | : | The loading shall be highest of the following: |



- i) Two bulldozers (maximum capacity indigenously available) operating side by side without any coal heap over reclaim tunnel & surroundings and uncrushed coal storage area.
- ii) Two bulldozers operating side by side over a coal heap of 4.0m.
- k) Rail loads : As per I.R.S. including latest amendments

All structures under influence of Railway load shall be designed as per provisions of Indian railway code of practices and the contractor shall get the designs and drawings of structures approved by railway authorities also.

- l) Other loads : As per IS:875 or as decided by the Purchaser
- m) Seismic Loads : For design purpose the values for Seismic Zone – III as per IS:1893 shall be considered. Importance factors for all buildings shall be considered as 1.75. In addition, provision contained in IS:4326 & 13920 shall also be considered.
- n) Wind Load : As per IS:875. Basic wind speed = 50 m/s.
- o) Minimum thickness of walls / raft for all underground structures will be as follows:

Minimum thickness of R.C.C. raft at bottom shall be 600 mm. Minimum thickness of RCC side walls shall be 600 mm at bottom and 300 mm at top.

- p) A minimum safety factor of 1.2 against uplift of due to underground water shall be ensured during execution and after execution, considering dead weight of the structure to be 0.9 times only, ground water table at adjoining formation level and soil wedge angle of not more than 15 degrees. Also FOS against uplift to be taken as 1.0, considering the dead weight of structure and soil resting on side projections, if any in the vertical plane. Inclined wedge action of soil shall not be considered in this case.
- q) For Underground structures, earth pressure to be considered for design shall be due to earth pressure at rest (K_0) condition only. Earth pressure due to surcharge intensity of Railway Loads (where applicable) or Uniformly Distributed Load (UDL) of intensity 2T/sqm whichever is critical, shall be considered in the design.



- r) For design of coal hoppers, bulk density and angle of repose for coal shall be considered as 1.2 MT/cum and 31 deg. respectively.
- s) For deep underground structures, minimum clear cover for reinforcement shall be 50 mm unless otherwise stated.

For Design of Liquid storage structures , IS: 3370 (Part-I to IV) (Latest) shall be followed. Face of the structure in contact with the water shall be designed as uncracked section.

For design of underground structures/ foundations, ground water table shall be assumed at the formation level (i.e adjoining ground level).

Face of the structure in contact with earth shall be designed with crack width limiting to 0.1 mm

- t) Water proofing of underground structures shall be done as /clause 6.11 of this specification.



15.00.00 ASH HANDLING SYSTEM**15.01.00 Specification for Civil, Structural and Architectural Works will be referred**

Ash handling system shall comprise of fly ash handling systems, ash slurry pump house, ash water pump house, ash compressor house, ash pipe supports including culverts, bridges etc. upto the specified terminal points.

Works shall include dry bottom ash hopper supports, scraper chain conveyor foundation & supports, transition chute foundation & supports, seal trough foundation & supports, clinker grinder, jet pump supports & foundation, bottom ash pipe supports, bottom ash extraction equipment supports, ash slurry pipe supports from ash slurry pump house to ash pond, surge tank, dry fly ash & bottom ash pipe supports, supporting structure for buffer hopper, fly ash silos, wet fly ash slurry pipe & supports from silo to ash dyke. Development of silo area (including paving, fencing, access roads, office block and watchman cabin), pump and equipment foundations, Vacuum pump foundation, BA over flow sump, drain sumps, compressor house, MCC room, RCC thrust blocks, 100 tonne electronic weigh bridge and other buildings, supporting structure facilities required to complete the system upto terminal points.

Structural arrangement, pile foundation system, if required, cladding interior and exterior finishes, area paving etc., shall be as per total plant building design and finish schedule furnished elsewhere in the specification.

Pipelines shall be supported on RCC structures and pile foundations, if required. Pipes shall be suitable anchored with RCC pedestals to resist lateral and vertical movements. For crossing of the pipe lines and maintenance road with nallahs, canals etc. and / or for road / rail crossings, pipes shall be laid through RCC box culverts / bridges. Road leading to silos shall have minimum width of 7.5m with both sides berms 1m and drains.

The compressors and their associated equipments shall be housed in RCC building along with requisite MCC room. Crane / monorail of adequate capacity shall also be provided in all pump / compressor house.

15.02.00 Following facilities shall be provided as per Vol. III of specification.

- i) Ash slurry sump with pump house.
- ii) Ash water sump house with pump house.
- iii) MCC Room / Control room.
- iv) Pipe Rack / trenches etc. in the plant area.
- v) RCC paving in Silo area.
- vi) Office block of minimum 50 sqm and Watchman cabin of minimum 5 sqm area shall be provided.
- vii) Dry ash storage silo area shall have compound wall all around with gate.
- viii) The following densities shall be considered for design of coal bunkers :



- (a) For volume calculation of fly ash silo : 750 kg/cum
- (b) For load calculation of both types of silos : 1600 g/cum
- ix) The concrete ash silo shall be designed generally as per the criteria laid down in IS:4995 (Part I&II). The static pressure calculated at rest shall be multiplied by an over pressure factor of 1.35 for the top 1/3rd portion and by a factor of 1.75 for the bottom 2/3rd portion. Special attention shall be given in assessing the effect of hot temperature of ash on the concrete wall. Temperature of ash shall be taken as 150°C.
- x) Clarifier.
- xi) Ash compressor house.
- xii) Fly ash silos, water wash arrangement lorries shall be provided in the flyash silo area



16.00.00 PLANT WATER PRE-TREATMENT AND DM PLANT SYSTEM

16.01.00 Sea water pre-treatment for CW and DM plant will generally consist of the following facilities / buildings, which will be located in two areas. However, the bidder may add / delete / club facilities as per approved system:

1. Sea Water Intake & Water Treatment In NCTPS Stage – II

- Sea water intake pump house including channel & forebay.

2. Water Treatment Facility Located In Ennore SEZ

- Cascade aerator & stilling chamber.
- Inlet channels.
- Filtered sea water tank & pump house.
- Dual Media Filters
- UF building.
- UF permeate tank & pump house.
- Cartridge filter building.
- RO Stage I building.
- RO Stage I Permeate tank & Pump house.
- Reject sump.
- Filtered water tank & pumphouse.
- RO stage II building including MB.
- DM building for SAC, SBA & MB chain.
- RO Stage II permeate tank & DM water storage tanks.
- Pump foundations in water treatment area.
- Electro chlorination building for CW system.
- Regeneration & bulk chemical storage area.
- CPU regeneration (if separate).
- Neutralization pit.

The stilling chamber and aerator shall be of RCC structure. The chambers and water channels shall be watertight structures.

The high rate solid contact clarifier shall be of RCC construction. The watertight circular RCC structures shall have continuous platform with handrail along periphery at the top. At least 3 nos. ladder shall be provided at the outside and vertical rungs shall be provided inside for maintenance.

The sludge sump (reject sump) shall be an underground RCC pit, part of the pit shall be cover with RCC slab for supporting the vertical sludge pumps. The sludge pumps shall be housed in a pump house with RCC roof supported on RCC columns and beams. There will be hoist and monorail of required capacity for handling of pumps. Capacity of reject sump is indicated in Vol. III & Composite Water Scheme.

Filtered sea water tank/ UF permeate tank/ RO Stage I Permeate tank/ Filtered water tank shall be of water tight RCC construction having two compartments covered with RCC roof. It will be aboveground. Vent pipes shall be provided at roof. The tanks including RCC roof shall be designed as uncracked structure as per IS:3370. The pump house structure shall be of RCC framed construction with RCC flat roof. Side cladding shall be



of brick masonry. Cranes & hoists as indicated in Vol. III shall be provided for handling pumps. The building shall be complete with doors, windows and rolling shutter. Floor and other finishes shall comply with architectural specification. Capacity of the tanks is indicated in Vol. III & Composite Water Scheme.

Pretreatment chemical house shall be a two storied concrete RCC framed building. Clear height of 5.5m (minimum) shall be provided in each floor. The building shall be complete with brick work, plastering, doors, windows, rolling shutters, flooring, plaster, painting, roofing etc. and all other items necessary for satisfactory performance of the building. The building shall have toilets and the scope shall include supply and fixing W.C.'s urinals, showers and all plumbing sanitary and other work required for satisfactory completion of the above building. The tanks and parts of floor slabs shall be lined with acid / alkali proof tile lining. Monorails of adequate capacities shall be provided as per functional requirements. The building shall be provided with adequate toilet facilities.

Near the chemical house there will be a single storeyed RCC framed building housing the electro-chlorination plant. Hoists with monorail shall be provided for normal plant operation and for handling of equipment during erection / maintenance. The building shall be complete with brick work, plastering, doors, windows, rolling shutters, flooring and painting, roofing etc. and all other items necessary for safety and satisfactory performance of the building. Brick masonry wall with suitable windows and doors will be provided in accordance with architectural specification.

Water treatment facility including RO stage I, UF building, Electrochlorination & Sea water intake PH, and DM plant building in Ennore SEZ will be RCC framed building having brickwork cladding on all four sides. Ground floor shall house MCC and control room. Control Room which shall have false ceiling and shall be air-conditioned. Central chemical laboratory and office rooms etc. shall be at first floor level, which will also have false ceiling and air condition provision. Equipment foundations & trenches in ground floor slab shall be of reinforced cement concrete. Ground floor shall have ironite flooring except at locations, which may come in contact with acid/ alkali where acid / alkali resistant tile lining with epoxy mortar shall be provided. Effluent drain trenches shall also have acid proof lining at inside surfaces of drain as well as soffit of pre-cast removable covers. The building shall be provided with adequate toilet facilities. The lab shall be provided separately in NCTPS & Ennore SEZ.

Floor where acid / alkali spillage are expected shall be isolated with RCC kerb. Neutralizing pit shall be of watertight RCC construction. Internal surfaces shall be lined with acid / alkali proof lining with matching epoxy mortar. Neutralization pit shall be designed as uncracked section. Ceiling as well as floor, supporting pumps shall be given epoxy lining.

Acid / alkali storage tanks shall be resting on RCC saddles on elevated RCC platform. The platform shall be supported on RCC beams, columns with RCC foundations. The saddles along with platforms shall be lined with acid/alkali proof tiles.

Acid / alkali storage and handling area shall also be lined with acid / alkali resistant tiles.

The DM storage tanks shall rest on thoroughly compacted fill of gravel, coarse sand or other stable material topped with minimum 75 mm thick compacted crushed stone, screenings, fine gravel, clean sand or similar material mixed in hot asphalt (8 to 10



percent by volume) and rolled or compacted. This filling shall be confined within a RCC ring wall with foundation. The tank shall have complete bearing with the foundation fill. All backfilling within tank foundation shall be done by compacted sand. Degasser shall be supported on RCC framed structure. Minimum 2.5m wide RCC pavements shall be provided around each outdoor equipment.

The capacity of all tanks and equipment shall be as per VOL III.



17.00.00 OUT DOOR CIVIL WORKS**17.01.00 Roads**

Access within the plant site shall be provided by a system of roadways.

All building shall be approached by access road, which shall either be single or double lane road depending upon the functional requirement. Access roads shall also be provided to areas such as transformer areas, steam generator area and other equipment area shown in the plot plan, where access is necessary for inspection, operation and maintenance.

Roads shall be three types: Type I, Type II and Type III. All the roads shall be of reinforced cement concrete with M30 grade concrete.

Type I roads shall consist of 7.5 metres wide RCC paved carriageway with 2.5m wide hard shoulders. The main plant access road and a portion of the main plant complex circumferential road shall be Type I.

Type II roads shall consist of one 4.0 metre wide RCC paved carriageway with 1.5m wide hard shoulders with drains on either side. A turning area at blind ends shall be provided. Plant areas where infrequent access is needed shall be served by Type II roads. Examples of Type II roads are the lay-down area roads.

Type III roads shall be 3.0m wide with 1.0m wide hard shoulders on either side. Type III roads shall be provided along the plant boundary for access for security and maintenance.

Top of all the shoulders shall be finished with interlocking tiles. 80 mm thick M20 grade PCC with minimum 100 mm thick screed sand cushion layer shall be provided below the interlocking tiles.

Road leading to main gate of power house shall be double lane road with suitable berm. Minimum thickness of road shall be 25 cm and width of 7.5m each.

All Type I roads shall have a minimum turning radius of 25 meters and for Type II and Type III roads shall have a minimum turning radius of 7 meter.

Bollards shall be provided along side of all type of roads near equipment which requires protection. Spare duct bank shall be provided under all type roads spaced at 100m intervals.

Sign Boards shall be provided for vehicle management and shall meet the Indian standards. All sign boards shall be dual worded in both English and the local Indian language. Finished top (crest) of roads shall be 250 mm above the surrounding grade level. Geometric design of road shall be in accordance with IRC:73. The ruling gradient for roads in longitudinal direction shall not exceed 1 in 25.

Main roads shall be designed for movement of heaviest equipment of the plant.

The shoulder shall be laid with slope of 1 in 30.



Top level of parking area shall be flushed with crown connecting roads with cross slope. Parking area shall be provided rigid pavements & shall be provided with anti skid tiles.

On either side of type I and type II roads and on one side of type III roads, open drains shall be provided. Minimum clear width of drains shall be 600mm. The drains shall be designed and built using RCC. Drainage lines and other underground services shall be located at least 1m clear from the edge of the roads. All service and utility lines crossing the roads shall be taken through NP3 class RCC pipe designed for impact loading. All culverts carrying storm water shall be cast in place RCC box culverts.

No underground service piping except for drainage and sewage system shall run directly below the road (including upto 1 m. from edge of road) along its longitudinal direction.

Surface drainage of roads shall be provided by giving proper longitudinal slopes and cross falls.

The Minimum thickness of RCC road pavement, Sub-base and minimum reinforcements shall be as follows.

Type I & II roads –

250mm thick with 8mm TOR at 250mm c/c both ways at top and bottom (double layer)

Sub – base : 2 layers of granular sub base plus 100mm thick PCC.

Type III roads –

150mm thick with 8mm TOR at 250mm c/c both ways (Single layer) Sub – base : 250mm thick granular sub base plus 100mm thick PCC The sub-grade shall be compacted to atleast 98% Proctor Dry Density. The actual thickness road pavement, sub-base thickness, reinforcement etc., shall be provided as per design and functional requirements.

Roads shall be designed for IRC and MOST standards (MORTH). Minimum thickness of basement slab and walls and drains to be 200mm thick with 2 layers of reinforcement.

A Separation membrane shall be used between Concrete and sub-base. Membrane shall be impermeable. Plastic sheeting shall be 125microns thick laid flat without creases.

The drainage system shall be designed for precipitation intensity of minimum 80mm per hour.

The joints, dowel bars, tie bars, joint pillars, ceiling compounds tolerances curing shall be as per latest IRC MOST standards.

17.02.00

Surface Drainage

- a) All the paved and unpaved areas shall be adequately drained. The surface drainage system shall be designed for surface washings and / or rain / fire water as the case may be. Unpaved open areas shall be drained through RCC drains and connected to main storm drains.
- b) The paved area shall be sloped towards the drains with a minimum slope of 1 in 100. The maximum drainage travel extent shall be limited to 10 metres.



- c) The surface drainage from uncontaminated area shall be connected to nearest open storm water drains through rectangular drains. Contaminated area surface drainage shall be collected through separate network.
- d) The interconnecting pipes and rectangular drains shall be sized for carrying the design discharge when running full.
- e) The rectangular drains shall be minimum 450 mm wide of RCC construction. The pipes for water drainage system shall be of RCC class NP2 conforming to IS:458 with minimum size of 150 mm NB. However, for road crossings etc., pipe of class NP3 SHALL BE PROVIDED. For rail crossings, pipes conforming to railway loading standards shall be provided. If sufficient clearance cannot be provided between the top of the pipe and road top, the pipes shall be encased in RCC. Minimum clear width of drain shall be 300 mm.
- f) The maximum velocity for pipe drains and open drains shall be limited to 2.4m/sec and 1.8m/sec respectively. However, minimum velocity for self cleansing of 0.6m/sec shall be ensured. Slope of drain shall not be milder than 1 in 1000.
- g) Minimum earth cover of 450 mm shall be provided over drainage pipes in paved areas.
- h) RCC Garland drains minimum 300mm wide shall be provided allround the building to lead away roof drainage to plant drainage system. Plinth protection in PCC grade 1:2:4 shall be provided between brick wall and drain with appropriate slope.

17.03.00**Storm Water Drainage System**

- a) The plant storm water drainage system shall take into account the topography of the plant area, area drainage patterns and intensity of rainfall etc. The drainage system shall be designed for a precipitation intensity equal to hourly rainfall for a return period of 1 in 50 years. However, storm frequency of 100 years return period shall be applied for Coal Storage area. These values shall be based on the recommendations of Indian metrological department (IMD).
- b) All storm water drainage shall preferably be through open storm water drains. These shall be provided on both sides of the roads and shall be designed to drain the appropriate catchment area including road surface, open and covered area etc. The drains shall be minimum 300 mm wide at the base.
- c) All open drains shall be rectangular in cross section & in RCC. In the main plant block, rectangular section RCC drains in minimum M25 grade concrete shall be provided. The thickness of side and bottom shall be minimum 125 mm or as per design considerations whichever is higher.
- d) The pipes for water drainage system shall be concrete pipes of class NP2 conforming to IS:458. However, for road crossings etc. higher strength pipe of class NP3 shall be provided. Diameter of pipes used for drainage / culverts shall be between 300 mm to 600mm. Beyond 600mm, box drains / culverts shall be provided. Water way RCC culvert shall not be less than 1m.



- e) Surface drains shall normally have a bed slope not milder than 1 in 1000 along longitudinal direction and RCC pipes shall have such slopes so as to have effective discharge. The maximum velocity for pipe drains and open drains shall be limited to 2.4 m/sec and 1.8 m/sec respectively. However, minimum velocity for self cleansing of 0.6m/sec shall be ensured at peak flow condition (i.e. 3 times average flow) for pipes flowing at half full.
- f) Suitable manholes shall be provided to piped drainage lines at every 30m intervals, at junctions and at change of gradient, alignment and diameter of pipe and shall be of masonry or RCC construction. Minimum size of manholes shall be 1.0m x 1.0m. All manholes shall be designed considering maintenance, inspection and cleaning of pipes. Easy accessibility and safety shall also be given due consideration.
- g) The cushion over the pipes for storm culverts shall be minimum 600mm. Where less cushion is available, pipe shall be cased in RCC m-15. Suitable RCC or masonry structures shall be provided at drops / falls to prevent scouring or damage to surface.
- h) Invert of drainage pipe / drain shall be decided in such a way that the water can easily be discharged above the high water level in water course outside the plant boundary to which the storm water is to be led.
- i) The storm water drainage for the contaminated area such as coal stack pile, fuel oil area, oil skids etc. shall be designed separately and the discharge shall be led separately for treatment and disposal.

17.04.00**Paving**

Sufficient pavement areas adjacent to administrative building, canteen building, service building, fire station and first aid centre shall be provided for parking facilities.

Concrete paving of grade M-20 with suitable underbed shall be provided in the entire area from Transformer yard to full length up to 5m beyond chimney.

The inside area of fuel oil dyke shall be provided with concrete paving.

The transformer yard, covering area between A row of the turbine building upto the fencing of transformer yard shall be provided with minimum 100 mm thick PCC paving of nominal mix 1 : 2 : 4 of grade M-20. Paving shall be laid over 150mm thick compacted rubble soling and laid to slope towards drains.

Entire switch yard shall be provided with 75mm paving of 20 to 40mm size stone aggregate on top and 75mm paving of 20 mm stone aggregate below. Before laying the paving, the ground surface shall be treated with antiweed chemicals as per manufacturer recommendations.

For auxiliary transformer yard, paving of stone aggregate of 75 mm thick using 20 mm size aggregate shall be provided after compacting the under bed and treating with anti-weed chemicals.



The RCC paving in specified areas shall comprise of following layers :

1. 255 mm thick approved quality boulders shall be placed on well compacted soil underneath. The interstices of the boulders shall be filled with sand. This layer shall be compacted to the satisfaction of engineer.
2. 100 mm thick M15 grade plain cement concrete on layer mentioned (1) above.
3. M20 grade reinforced concrete with nominal reinforcement 8 tor @ 250 c/c both ways top and bottom having thickness 200mm shall be laid over the layer mentioned (2) above.

17.05.00

Security cum Time Office Complex :

Approximate area of this complex shall be a minimum of 400 sqm. The dormitory shall be constructed in the first floor for security persons.

The complete security gate complex consisting of Gates, Guard House, Wicket gate, time and security office, speed breakers, traffic barriers and parking/waiting space shall be carefully planned such that they function smoothly, specifically at the time of Change of shift.

The whole complex shall be aesthetically pleasing merging with the architecture of the compound wall.

Time and Security Office shall have adequate area to house the security staff and time office staff in addition to the record room, waiting rooms, toilet facilities etc.

In Time office, separate dress change rooms shall be provided for ladies and gents.

In addition to inside toilet, another toilet with entry from outside shall be provided for the use of visitors.

Speed brakers shall be provided both out side and inside the compound with manually operated traffic barriers.

Emergency Gate shall be provided in addition to the main gate. Main Gate shall have two separate entry one for project site and other for fly ash silo trucks movement. Gate latches shall be arranged for padlocking with the padlock accessible from both sides of the gate. Padlocks shall be provided and the key shall be in accordance with the Purchaser's requirements.

Cantilever type motorized sliding gate shall be furnished at the main plant entrance. The gate shall be operable by push button at both the guard house and the security office in the Administration / Plant Service area, and by card reader or key switches.

The road near the main entrance to be widened additionally by 7.5 m on either sides of the road for parking of vehicles for security check.

Suitable median arrangements to be made.

Security cabin to be provided on either sides of the road near the main gate.



Elevated platform either in steel structure / Brick work or RCC to carry out checking on Lorries/Tippers by security personnel while the vehicles getting in or going out from the premises.

17.06.00**Fencing**

- a. Minimum 3m height fencing above toe wall shall be provided around transformer yard and other areas where fencing is necessary due to statutory requirements.
- b. Fencing shall comprise 3.0m height PVC coated galvanized chain link fencing of minimum 8 gauge including PVC coating of mesh size 75 mm and galvanized concertina.
- c. Galvanized barbed of a height of 0.6m shall be provided above the chain link fence. The diameter of steel wire for chain link fencing excluding PVC coating shall not be less than 12 gauges.
- d. MS entry gate shall be provided for all fenced areas. Top of toe wall shall be min. 200 mm above formation level.
- e. Removable type of fencing shall be provided at suitable location to permit entry and exit of equipment.

17.07.00**Watch Tower**

Watch Tower shall be constructed with RCC frame work. Floor of tower shall be at least 6m above graded level.

Covered area for each tower shall be approx. 25 sqm. Number of towers shall be such that the complete periphery of the plant can be watched from the towers but in no case the number of towers shall be less than twelve.

Walking platform of 1.5m wide with hand rails shall be provided all round of watch cabin.

17.08.00**Plinth Protection**

All buildings shall be provided with 1000 mm wide and 100 mm thick PCC of grade M-20 for plinth protection all round with surface drain of required size and slope, to suite storm water quantity. Plinth protection shall be laid over prepared subgrade and base formed with broken brickbats or rubble laid to a thickness of 150 mm.

17.09.00**Waste Water Treatment and Drainage System**

Waste water treatment plant will receive plant waste water except RO plant reject and CT blow down. The treated water and the solid waste will be used within the plant.

Guard pond will hold the RO Stage-I reject, clarifier sludge treated water from oil water separator and this will be pumped to the sea. For this return water to the sea temperature shall not exceed 5°C over the inlet temperature of cold water.



The cooling tower blow down shall be pumped to sea with the help of blow down pumps installed in the CW pump house.

All necessary civil / structural works for waste water treatment plant shall be under scope of contractor. The major source of plant waste water are Boiler blow down, coal mill area drainage, PH and boiler area oily effluent drain, F.O. pump house oil drainage, boiler and ESP area floor washing drains, water pre-treatment plant effluent, fly ash silo area effluent, transformer yard waste water etc. Design and construction of channels / settling pits / sumps / pipe lines / separators shall be done by the Bidder.

The description of some of the major structures / components covered under the waste water system package is given below:

Coal pile area run-off will be led to settling pond. Earthen settling pond with two (2) compartments will be provided. Top of earthen dyke shall be 500 mm above surrounding finished grade level to restrict ingress of storm water from adjacent areas. Capacity of pond shall be determined on the basis of inlet drain invert. The side slopes and bottom will be protected with PCC blocks of minimum 75 mm thickness with suitable under-bed having interstices filled with cement-sand mortar. At the downstream of pond RCC overflow weir and sump shall be provided. Design and detailing of pond shall be as per good engineering practice so as to satisfy functional requirement as specified in the specification. Necessary sluice gates with hoisting arrangement shall be provided in inlet drain carrying coal pile area run-off so that one settling pond can be operative while the other one under maintenance.

The Power House and boiler area service water waste shall be collected in a RCC underground oily waste retention pit. Necessary pumps and supporting floor / maintenance area shall be provided. This effluent will finally be treated in TPI (tilted plate interceptor). Foundation from TPI shall be designed as per IS:456. RO Stage-II reject shall be led to fire water sump and green belt development. In case the fire sump is full then arrangement shall also be made to lead this to the guard pond.

Oil storage and handling area run-off will be collected to a conventional baffled oil water separator. This oil water separator will be an underground RCC structure having RCC baffle wall and overflow sump for collecting water at outlet end. An oil collection pit shall be provided on sidewall of oil water separator. This structure shall be designed as un-cracked section as per IS:3370. The structure shall project at least 300 mm above finished grade level. Handrail shall also be provided around the pit. The overflow sump shall be covered at top over which pumps will be installed.

RCC sump for collecting boiler blow down and ESP area floor washing is included in scope of bidder. Civil work for outlet of this effluent shall also be under scope of this package.

Guard Pond (effluent monitoring pond and equalization basin) sized for holding waste water from RO Stage-I reject, clarifier sludge, treated water from oil water separator and boiler blow down as per capacity indicated in Vol. III & Composite Water Scheme. The guard pond (earthen) shall have adequate capacity as per design requirement with 750 mm free board. Top of earthen dyke shall be 500 mm above finished grade level. The pond shall be of such construction as to prevent pollution of ground water by seepage of



any wastewater having side slopes and bottom lined with minimum 250 micron LDPE for minimizing seepage loss. Over the LDPE lining, PCC blocks of minimum 75 mm thickness having interstices filled with cement-sand mortar shall be provided. Filter media shall be suitably designed and provided below liner.

17.10.00 Sanitary Sewerage System

Sanitary Sewerage System shall be carried out as specified in VOL – III (Mechanical Spec.).

The treated water to be provided with necessary pumps and distribution lines for reutilization for Horticulture / Green belt development.

The sewerage system of foul water from toilet shall include layout and laying of sewers up to a sewage treatment plant together with all fittings and fixture and inclusive of ancillary works such as connections, installation of man-hole and inspection chambers. The construction of a sewage treatment plant of adequate capacity including settling tank, lifting stations, pump house and all other necessary provisions shall be made for treating the sewage water by using the latest CAACO technology developed by M/s. Central Leather Research Institute, Taramani, Chennai.

The domestic effluent from the plant shall be collected and treated in proper STP to meet the prescribed BIS standard before being discharged or reutilized for green belt development.

17.10.01 Sanitary Sewer Drains

- a) Sanitary sewers shall be designed for a minimum self cleansing velocity of 0.75 m/sec. and the maximum velocity shall not exceed 2.5m/sec.
- b) Very hot (over 60⁰C) water shall be first cooled down to less than 60⁰C in collecting basin by mixing with cold water before connecting to storm drainage system.
- c) The maximum temperature, quality, quantity and location of drain water of individual equipment shall be tabulated and furnished to the Owner's representative.
- d) All underground piping below concrete slab shall be cast iron of minimum 100mm dia and for outdoors it shall be reinforced concrete pipe of minimum 200mm diameter. In buried piping system manholes shall be placed at every change in direction and at every SOM (max.) interval in straight run. Suitable clean outs shall be provided for buried piping under floor slab.
- e) Following minimum drainage slope shall be provided:
 - Pipes of diameter less than 200mm : 1 (vertical) : 100 (horizontal)
 - Pipes of diameter 200mm & more: 1 (vertical): 200 (horizontal).



18.00.00 ARCHITECTURE**18.01.00 Architectural Concepts**

- a) Layout of the plant area shall have definite hierarchy of road network depending upon its usage, aesthetic, visual sensibilities for creating road vistas, focal points, building back drops, building frames. General layout shall be evolved taking over the basis of landform & local climate & due consideration shall be given to orientation & wind direction. The resulting built mass shall present a definite image width in distinct vocabulary in the form of landmarks, nodes & skyline.
- b) Main Plant Building shall be architecturally treated in such a way that it retains a monumental scale, yet presents a pleasing composition of mass and void with suitable and functionally designed projections and recesses. The overall impact of the building shall be one of aesthetically unified architectural composition having a comprehensible scale, blending tonal values with the surroundings and taking full consideration of the climatic conditions, the building orientation and the existing structures nearby.
- c) All other buildings and structures shall be architecturally treated in such a way so as to be in complete harmony with the main plant, surrounding structures and environment. Local architectural characters and materials may be judiciously imbibed. The building shall be designed initiating an architectural control common to all buildings. The architectural control shall be clearly spelt out in terms of scale, man & form.
- d) Overall colour scheme and finish schedule of the plant and other buildings shall be designed judiciously and in a comprehensive manner taking into account the mass and void of buildings, its façade, equipments, exposed structural elements, piping, trestles, bus ducts and other service elements.
- e) Overall emphasis shall be on developing an eco-friendly architecture, merging with the nature with its own sustainable energy management systems.

The scheme shall be conceptually finalized in totality including that of equipments so that the proper coordination with other agencies can be taken up at appropriate time.

18.02.00 Architectural Design

- i) Natural light shall be used to the maximum extent specially in the form of north light / sky light. For adequate light & ventilation, National Building code recommendations shall be followed.
- ii) Entrance canopies, chajjas (projections, recesses) over openable windows and door openings on exterior facades shall be provided.
- iii) All the buildings shall be architecturally designed to meet the National Building Code (SP:7) norms and local building bye laws, wherever applicable.



- iv) Architectural design and detailing aspects of all the buildings shall be rendered through professional services of an Architect. Statutory requirements and any clearances from local authority may be required to be met with, wherever essential. The Architect Consultant shall be of National / International repute having experience in similar kind of works. The consultant shall evolve the design philosophy based on Employer's guidelines and shall present it in the form of presentation drawings, Prospective views, 3-D Models & detail drawings.
- v) Minimum 1000mm high (from floor / roof level) hand railing shall be provided around all floor / roof openings, projections / balconies, walkways, platforms, steel stairs, etc., All handrails and ladder pipes (except at operating floors) shall be 50mm nominal bore MS pipes (medium class) conforming to IS:1161 and shall be galvanized as per IS:4736 treated with etch primer and finished with suitable paint. All rungs and ladders shall also be galvanized. Minimum weight of galvanizing shall be 610 g/sqm.

All stairs and around all floor openings at operating floors, 1000mm high hand railing with 32 NB (polished) stainless steel pipe shall be provided.
- vi) All stairs shall have a maximum riser height of 180mm and a minimum tread width of 250mm. Minimum clear width of stair shall be 1200mm unless specified otherwise.
- vii) All buildings having metal cladding shall be provided with a 150mm high RCC toe curb at the edge of the floor along the metal cladding. 900mm high hand railing shall be provided on this RCC curb, wherever required from the safety point of view.
- viii) In all buildings, structures, suitable arrangement for draining out water collected from equipment blow downs, leakages, floor washings, fire fighting, etc., shall be provided for each floor. All the drains shall be suitably covered with grating or pre-cast RCC panels.
- ix) RCC staircase shall be provided for all RCC construction buildings and structural steel staircase for all structural steel buildings.
- x) Parapet, Chajjas over window and door heads, architectural facias, projections, etc., shall be provided with drip course in cement sand mortar 1:3. Cut lintels over windows/doors shall be avoided as far as possible.
- xi) All fire exits shall be painted with P.O red/signal red colour shade which shall not be used anywhere except to indicate emergency or safety measure. Fire safety norms shall be followed as per National Building Codes and fire safety requirements for providing fire exits, escape stairs and firefighting equipment. In detailing of all buildings, fire safety requirements conforming to IS:1641 and IS:1642 shall be followed.
- xii) All windows shall be provided with granite slab at sill level.
- xiii) All grills of window shall be of 12 mm square MS bars both ways.



18.03.00 Interior Design

A comprehensive interior design scheme shall be conceived with the intention of projecting a definite theme and aesthetic appearance to inside working environment. It shall take into account the multidisciplinary engineering activities involving power plant technology, and architectural and civil engineering for a smooth control hierarchy and man machine interface. All the design aspects such as flooring, false ceiling, furniture, colour scheme, equipment design, layout, illumination, fire fighting, acoustics and ergonomics requirements shall be detailed out so as to present an overall unified aesthetic spatial appearance. The areas to be undertaken for this interior design process shall be control room complex including common control room, computer room, conference rooms and office areas in the main plant building and the following aspects shall be reviewed and evaluated for design. Furniture to be supplied by Bidder for the control room complex shall be as specified under C&I specification.

- a) Layout, keeping in view the main-machine interface and suitable ergonomic practices.
- b) Integration of civil engineering with architecture and interior design.
- c) Illumination levels, noise levels, electromagnetic interference levels, taking into account the equipment and furniture.
- d) Comfort and safety requirements such as air conditioning, fire fighting, fire escapes etc.
- e) Microprocessors based control system to control the functional requirements.

The above design philosophy put into practice shall be detailed out through presentation drawings, perspective views, scale models, detail drawing etc.

18.04.00 Sealant

Silicone sealant or polyurethane shall be used in all expansion joints exposed to weather. All joints around exterior doors, windows and expansion joints shall be sealed for proper water tightness.

18.05.00 Damp Proof Course

50mm thick 1:1.5:3 concrete with water proofing admixture shall be used. Water proofing compound shall be of Zydex, Sikka or similar make.

18.06.00 Finishing Schedule

The minimum quality of finishes used for various building areas are furnished in Table-1 in this section. The Bidder is at liberty to use superior finishes provided all specific requirements for the finish specified in Part-B of Vol-VI.

18.07.00 Roof Drainage and Water Proofing

- a) Roof drainage system shall be provided for quick and efficient draining of rainwater from roof to avoid seepage and damage to roof. The runoff gradient for the roof shall



not be less than 1 in 100. Roof drainage system shall consist of roof drain heads, rain water down comer and fixtures. System shall be designed to handle design rainfall for the specific site and shall be in accordance to stipulations of IS: 1742 and IS: 2527. The down comer pipes shall be suitably concealed with masonry work, cement concrete or sheeting to match with the exterior finish.

- b) Multiple drains (minimum 2) shall be provided for all roof areas. Any roof more than 8 m above grade shall have access from within the building for cleaning of roof drains.
- c) Rain water down comers shall be of HDPE pipe conforming to IS: 4984.
- d) All roofs shall be provided extra with heavy duty water proofing treatment comprising of
 - i. Seven courses as per IS:1346 using layer of Type 2 Grade 1 glass fire based felt as per IS:7193.
 - ii. 50mm thick foam concrete as per IS:6598 for thermal insulation
 - iii. 15mm thick cement sand plaster 1:4.
 - iv. Chequered cement tiles as per IS:13801 shall be provided over water proofing treatment in areas where movement of personnel is expected. Minimum width of pathway if provided only locally shall be 1000mm.
 - v. In areas such as roof of control room bay of Main Turbine building, Bunker bay roof etc where handling of equipment is anticipated, the foam concrete may be replaced by 40mm screed concrete.

Roof water proofing may also be provided using high solid content liquid applied elastomeric water proofing membrane with separate wearing course as per ASTM C-898. Thickness of the membrane shall be a minimum of 1.5mm. The treatment includes application of polymerized mortar over sloped roof to achieve a smooth surface and a primer coat. Wearing course shall be 40mm screed of 1:2:4 concrete as above cast in panels of 1.2m x 1.2m and reinforced with 0.56mm dia galvanised chicken wire mesh and joints sealed using sealing compound. Accessible roof shall be provided with chequered tiles as above.

18.08.00 Sumps

In case of underground structures, sumps with pumping arrangement shall be provided at suitable location to collect and pump out any incidental water collection to nearest storm water drain.

18.09.00 Master Key System

An appropriate serviceable and functional master key system for the whole plant shall be installed. The elaboration of the system itself shall be made in close co-ordination with the Purchaser / Consultant and only after obtaining the Purchaser's / Consultant's approval in writing shall the order of production be placed.

The following requirements shall be met:



- The general master key shall operate all locks
- The main key shall open all locks of one building
- The single key shall open the lock of a single room.

Necessary attention shall be paid to later extensions of the master key system, which shall be suitable for the entire plant including all final stages.

The profile cylinders shall be sea water resistant and shall suit all plant requirements. The material of the cylinders shall be of massive brass, nickel-plated with six security pawl studs.

The keys shall be made of material approved by the Purchaser / Consultant and shall have an engraved indication of the applicable key system and the building or door number.

Keys shall be supplied in the following numbers:

General Master Key	10	nos
Main keys	10	nos. per each building
Single keys	3	nos. per each door

18.10.00 Land Scaping

18.10.01 General

This specification covers broadly the requirements for Landscaping within the Plant Area. Landscaping shall be carried out after all the underground utilities and drainage systems are in place. The entire work of Landscaping shall be carried out to the satisfaction of the Engineer and in a manner to have a beautiful appearance after completion of the work.

The Landscaping plan should be prepared by the Contractor with the help of an expert Horticulturist with due considerations for Fauna & Flora grown in the adjacent area and which can sustain in the saline climate. Minimum 30% of total plant area shall be covered with grass belt. In addition approximately 1000sqm of area shall be landscaped with Korean grass & latest flowers.

The area to be landscaped shall be clearly demarcated at site and got approved by the Engineer and the work shall be divided into following sub-items.

18.10.02 Preparing Lawn

For this purpose, within the area where lawn is to be grown, about 0.3 metres of earth shall be removed. This shall be replaced by fresh garden soil and manure in 2: 1 proportions spread evenly. Fresh garden soil & manure shall be mixed thoroughly, well watered and left overnight. The next morning, the variety of lawn, as per approved plan and as directed by the Engineer, shall be planted. This grass shall be maintained till well established by watering, weeding, clipping, rolling etc.

18.10.03 Preparing Clipped Hedges



Hedges made of Casuarinas or other shrubs as directed by the Engineer shall form borders between footpath and the Lawns. The shrubs shall be planted at spacing not more than 25 cms.

18.10.04 Planting Shrubs

Shrubs with coloured leaves like Crotons, Acalyphia, Oleander and Bougainvillea, Hibiscus etc. which are perennial shall be planted in clusters or over a large area at locations and in the manner as per approved layout. These shrubs shall be planted at spacing of not more than 25 cms.

18.10.05 Installation of Watering System

Shall prepare and submit watering System for the Plants. Drip Irrigation and Spray System consisting of complete PVC pipe with all necessary bends, Tees, Elbows etc., valves, stop cocks, Spray nozzles, sufficient lengths of Flexible PVC pipes to reach all area of Landscaping shall be included in the watering system. Only after approval of the Engineer, the Contractor shall commence the work.

TABLE – 1

FINISHING SCHEDULE

Building / Area	Wall (Internal)	Floor Finish	Ceiling	Roof Treatment	Wall (External)
STG Building					
Ground Floor: Maintenance Bay & Unloading Areas	Acrylic washable distemper	Granolithic with non-metallic hardner	Acrylic washable distemper	--	Water proof cement based paint
Ground floor general area	Acrylic washable distemper	Granolithic Flooring	Acrylic washable distemper	--	Water proof cement based paint
Mezzanine Floor	Acrylic washable distemper	Granolithic flooring	Acrylic washable Distemper	--	Water proof cement based paint
Operating Floor					
1. Operating Area, laydown area	Acrylic washer distemper	Corborandum tiles	Acrylic washable Distemper	--	Water proof cement based paint
2. General circulation and movement	Acrylic washable	Glazed vitrified ceramic tiles	Acrylic washable	--	Water proof cement based



Building / Area	Wall (Internal)	Floor Finish	Ceiling	Roof Treatment	Wall (External)
areas	distemper		Distemper		paint
3. Heater area	Acrylic washable distemper	Heavy duty cement concrete tiles (Carborandum)	Acrylic washable Distemper	--	Water proof cement based paint
Toilet	Dadoing for 2100 mm high + Acrylic distemper	Heavy duty ceramic tiles (Anti skid only.)	Acrylic washable Distemper	--	Water proof cement based paint
Roof	--	--	--	Elastomeric water proofing treatment	--
Clarified Water Pump House	Acrylic washable distemper	Granolithic floor with non metallic floor hardener	Acrylic washable Distemper	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Cooling Water Pump House	Acrylic washable distemper	Granolithic floor with non metallic floor hardener	Acrylic washable Distemper	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Raw Water Pump House	Acrylic washable distemper	Granolithic floor with non metallic floor hardener	Acrylic washable Distemper	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Control Rooms	Acrylic emulsion paint	Glaze vitrified ceramic tiles	Aluminum Fall Ceiling	--	Water proof cement based paint
MCC Switchgear Room	Acrylic washable distemper	Granolithic floor with non metallic floor hardener & top finished with epoxy coating.	Acrylic washable Distemper	--	Water proof cement based paint
Office Areas	Acrylic emulsion paint	Heavy duty Ceramic Tiles	Acrylic washable Distemper	--	Water proof cement based paint



Building / Area	Wall (Internal)	Floor Finish	Ceiling	Roof Treatment	Wall (External)
Cable Vaults	Acrylic Distemper	Granolithic floor with non metallic floor hardener	Acrylic washable Distemper	--	Water proof cement based paint
Control & Switchgear Building					
Central Control Room	Acrylic emulsion paint	Anti skid Marbonite tiles	Aluminum False Ceiling	Elastomeric water proofing treatment	Water proof cement based paint
Electronic cubicle room & Computer room	Acrylic emulsion paint	Heavy duty ceramic tiles	Aluminum False Ceiling	--	Water proof cement based paint
Conference room, senior executive room	Acrylic emulsion paint	Glazed vitrified ceramic tiles	Aluminum False Ceiling	--	Water proof cement based paint
Switchgear room	Acrylic washable distemper	Granolithic with non-metallic floor hardner & top finished with epoxy coating.	Acrylic washable Distemper	--	Water proof cement based paint
Battery Room	Acid & alkali tile dadoing for 2100 mm high + Chlorinated rubber based paint	Acid / alkali resistant tiles	Chlorinated rubber based paint	--	Water proof cement based paint
SWAS room	Acrylic washable distemper	Glazed vitrified ceramic tiles for dry areas. For wet areas Heat resistant, anti skid of approved brand & colour.	Acrylic washable Distemper	--	--
Office, Common areas & corridor	Acrylic emulsion paint	Heavy duty ceramic tiles	Acrylic washable Distemper	--	Water proof cement based paint



Building / Area	Wall (Internal)	Floor Finish	Ceiling	Roof Treatment	Wall (External)
Cable Room	Acrylic distemper	Granolithic with non-metallic floor hardener	Acrylic washable Distemper	--	Water proof cement based paint
Toilet	Dadoing for 2100 mm high + Acrylic distemper	Heavy duty matt finish ceramic tiles	Acrylic washable Distemper	--	Water proof cement based paint
Lockers room	Acrylic washable distemper	Kota stone.	Acrylic washable Distemper	--	--
Pantry	Dadoing for 2100 mm high + Acrylic distemper	Heavy duty matt finish ceramic tiles	Acrylic washable Distemper	--	--
Laboratory room	Acrylic washable distemper	Heavy duty ceramic tiles	Acrylic washable Distemper	--	Water proof cement based paint
Records Room	Acrylic washable distemper	Kota stone.	Acrylic washable Distemper	--	--
Other Buildings					
Bunker Building	Acrylic washable distemper	Granolithic floor with non metallic floor hardener	Acrylic washable Distemper	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Chlorination Building	Chlorinated rubber based paint	Acid / Alkali resistant tiles	Acrylic washable Distemper	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Chemical House	Acid/Alkali resistant tiles on wall up to 1500mm height & chemical resistant paint above.	Acid/Alkali resistant Tiles	Chemical resistant paint	Heavy duty water proofing as per IS:1346	Water proof cement based paint



Building / Area	Wall (Internal)	Floor Finish	Ceiling	Roof Treatment	Wall (External)
DM Plant	Chlorinated rubber based paint	Granolithic	Chlorinated rubber based paint	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Switchyard control room building and GIS building	Acrylic emulsion paint in control room and Acrylic washable distemper in other areas.	Epoxy finish on Granolithic flooring.	Aluminum false ceiling in control room and acrylic distemper in other areas.	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Workshop	Acrylic washable distemper in Workshop area and Acrylic emulsion paint in office area.	Granolithic flooring with non metallic floor hardener in workshop area and Heavy duty ceramic tiles in office area.	Acrylic washable Distemper	Heavy duty water proofing as per IS:1346	Water proof cement based paint
Canteen : General Area Kitchen	Dadoing for 2100mm high & Acrylic Distemper above. Dadoing for 2100mm high & Acrylic Distemper above.	Heavy Duty Ceramic Tiles Terrazo Tiles	Acrylic washable Distemper Acrylic washable Distemper	Elastomeric water proofing treatment	Granular finish Granular finish
Fire Station Building	Acrylic washable distemper	Vitrified ceramic tiles in office, dormitory area & granolithic flooring with non metallic floor hardener in parking & equipment store area.	Acrylic washable Distemper	Heavy duty water proofing as per IS:1346	Water proof cement based Paint
Service building: Ent.	Acrylic	20mm thick polished granite	Acrylic washable	Roof of service	Granular finish (Vineratex) or



Building / Area	Wall (Internal)	Floor Finish	Ceiling	Roof Treatment	Wall (External)
Lobby, Reception, Portico, staircase & Lift Area	emulsion paint	stone	Distemper	building shall be provided with Elastomeric water proofing treatment	equivalent.
Office area	Acrylic emulsion paint	Glaze vitrified Ceramic tiles	Acrylic washable Distemper		
Administrative Building : Ent. Lobby, Reception, Portico, staircase & Lift Area	Acrylic emulsion paint	20mm thick polished granite stone	Acrylic washable Distemper	Roof of service building shall be provided with Elastomeric water proofing treatment	Granular finish (Vineratex) or equivalent.
Office area	Acrylic emulsion paint	Glaze vitrified Ceramic tiles	Acrylic washable Distemper		



PART – B

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19.00.00 SURVEYING WORKS

19.01.00 Site survey shall be carried out to establish the ground levels and to determine any existing structures, roads etc. The site survey shall be carried out before the commencement of the work and if required during the progress of the work. The site survey shall be carried out at a grid spacing of 5-10 meters and at every change of level of the existing ground to produce contour drawings at 0.25 meter intervals. Scale of the contour drawings shall be as directed by the Consultant. A specialized firm approved by the Consultant shall carry out the site survey.

A single grid pillar will be given by the client with coordinates in plant area. Necessary grids shall be made by EPC contractor including demarcating the structures, buildings and jungle clearance including Cutting, uprooting and removing of trees and dispose the same away from the site etc.

19.02.00 Survey points (reference points and bench marks) : The contractor shall construct adequate number of reference points and bench marks for marking the setting out lines and levels. The reference points shall consist of suitable metal plates set in 400 mm x 400 mm x 500 mm precast concrete plinth (grid pillars) and inscribed with the exact level. All levels of the benchmarks shall be related to the agreed datum. The exposed numbers, locations, co-ordinates and level of the reference points and bench marks shall be plotted on drawings and approved by the consultant prior to the commencement of the work. Survey points in solid walls shall be stainless steel plates or cast iron, fixed firmly and sufficiently deep in the walls.

19.03.00 Safeguard of survey points: The survey points shall not be removed from their position without the permission of the Consultant. If any of the survey points are damaged or lost due to any work carried out near their location, the contractor shall be responsible for replacing such survey points.

19.04.00 Surveying during the progress of work : The contractor shall assist the consultant at any time when checking survey points, setting out, checking construction items and erection parts. The contractor shall provide and arrange the following:

- i) Provision and maintenance of survey instruments and accessories.
- ii) Provision of skilled personnel, supply of all material required for the survey exposing covered survey points.
- iii) Shifting of any machinery used for construction out of the sight lines.
- iv) Stopping all drilling, blasting, driving and any other works.
- v) Causing soil vibrations and stopping during instrument observations.
- vi) Removing all obstructive accumulation of water.
- vii) Taking all necessary safety precautions.



- viii) Furnishing any marking material requested by the Consultant in connection with control surveys.
- ix) Providing additional survey points in accordance with the consultant's instruction.



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20.00.00 EXCAVATION AND FILLING

20.01.00 The works to be provided by the bidder in respect of excavation in all types of soils including shoring, dewatering, filling around foundations and to grade, compaction of fills and approaches, protective fencing, lighting, etc. relevant to structures and locations shall be as detailed below:

- a. Furnish all labour, supervision, services including facilities as required under statutory labour regulations, materials, equipment, tools and plants, transportation etc. required for the work.
- b. Prepare and submit working drawings showing the approaches, slopes, beams, shoring, sumps for dewatering including drains and outfall for drainage, space for temporary stacking of spoils, disposal area, fencing etc. and all other details as may be required by the Engineer.
- c. To carryout sampling and testing and submit to the Engineer, results of soil compaction tests if required by the Engineer to assess the degree of compaction.

20.02.00 All works should conform to the requirements of the latest revision of relevant ISS.

IS: 3764 : Indian Standard for Safety Code for excavation work.

IS:1200 (PartI) : Indian Standard Methods of Measurement of Building and Civil Engineering work, Part-I: Earthwork.

20.03.00 The bidder should carryout the work as per the approved drawings. All materials required for the work shall be of best commercial variety. Borrow materials for back- filling shall be excavated from approved locations and shall consist of material, free from roots, vegetation, decayed organic matter, harmful salts and chemicals free from lumps and clods. If specified, clean graded sand free from harmful and deleterious material from approved quarries, shall be used as fill material.

20.04.00 Quality Control: The bidder shall establish and maintain quality control for the various aspects of the work, method, material and equipment used. The quality control operation shall include but not be limited to the following items of work:

- | | | | | |
|----|--------------------------|---|-----|---------------------------------------|
| a. | Lines, Levels and Grades | : | i. | Periodic surveys. |
| | | : | ii. | Establishment of markers, boards etc. |
| b. | Back-filling | : | i. | Checking the quality of fill material |
| | | | ii. | Checking moisture content |



of

the backfill.

- iii. Checking the degree of compaction.

20.05.00 Excavation

- a. Setting out : The contractor should submit to the Engineer, detailed drawings of the excavation work to be executed by him showing the dimensions as per drawings and specification adding his proposals of slopes, shoring, approaches, dewatering sumps, beams etc. Upon Engineer's approval the contractor should set out the work from the control points furnished by the Engineer and fix permanent points and markers for ease of future checking. These permanent points and markers shall be fixed at intervals prescribed by the Engineer. The contractor should proceed with the work after Engineer's approval. It should be noted that this checking by the Engineer prior to start of the work shall in no way absolve the contractor of his responsibility of carrying out the work to true lines, levels and grades as per drawing and subsequent corrections, if necessary, should be carried out by the contractor free of cost to the Purchaser.
- b. Clearing and Grubbing etc.: The area to be excavated shall be cleared out of fences, trees, logs, stumps, bush, vegetation, rubbish, slush etc. and leveled up. Trees upto 300mm girth shall be uprooted. Trees above 300mm girth to be cut, shall be approved by the Engineer and then marked. Felling of trees shall include taking out roots upto 600mm below ground level or 150mm below formation level whichever is lower. After the tree is cut and roots taken out, the pit-holes formed shall be filled with good earth in 250 mm layers and consolidated. The trees shall be cut in suitable pieces as instructed by the Engineer.
Before earthwork is started, all the spoils/ unserviceable materials and rubbish shall be burned or removed from the site to approved disposal areas as specified. Useful materials, saleable timber, firewood etc. shall be the property of the Purchaser and shall be stacked properly at the work site in a manner as directed by the Engineer.
- c. Excavation in all kinds of soil: The excavation in all kinds of soil including old (from demolished structures) underground RCC / CC / brick masonry shall be carried out as per the approved proposal, modified and corrected where necessary by the Engineer. The work shall be carried out in a workman like manner without endangering the safety of nearby structures/ services or works and without causing hindrances to other activities in the area. As the excavation reaches the required dimensions, lines, levels and grades, the work shall be checked by the Engineer thoroughly and the balance work shall be carried out carefully to avoid any over-excavation. On completion, the work shall be finally checked and approved by the Engineer. In certain cases, where deterioration of the ground, upheaval, slips etc. are expected, the Engineer may order to suspend the work at any stage and instruct the contractor to carryout the balance work just before the foundation work of the structure can be started.



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- d. Disposal: The excavated soils shall be disposed of within 5 km crow fly distance from site as directed by the Engineer-in-charge in any or all the following manners:
 - i) By using it for backfilling straightway.
 - ii) By stacking it temporarily for use in backfilling at a later date during execution.
 - iii) By either spreading or spreading and compacting at designated disposal areas.
 - iv) By selecting the useful material and stacking it nearly in areas designated by the Engineer for use in backfilling by some other agency.
- e. Disposal of Surplus: The surplus material from excavation shall be carried away from the excavation site to designated disposal area selected by the Engineer. All goods excavated from the pits and all assorted materials of dismantled structures shall be the property of the Purchaser.

20.06.00 Back-filling

- a. The backfilling material shall consist of materials, approved by the Engineer obtained directly from nearby areas where excavation work by the same agency is in progress, from temporary stacks of excavated spoils or from borrow pits from selected areas designated by the Engineer. The material shall be free from lumps and clods, roots and vegetations, harmful salts and chemicals, organic materials, etc. In certain locations, the Engineer may direct sand fillings. The sand should be clean, well graded and be of quality normally acceptable for use in concrete.
- b. Filling and compaction in pits and trenches around structures: As soon as the work in foundations has been accepted and measured, the space around the foundation structures in pits and trenches shall be cleared of all debris, brick bats, mortar droppings etc. and filled with earth in layers not exceeding 250 mm in loose thickness; each layer being watered, rammed and properly compacted to the satisfaction of the Engineer. Earth shall be rammed with approved mechanized compaction machine. Usually, no manual compaction shall be allowed unless specifically permitted by the Engineer. The final surface shall be trimmed and leveled to proper profile as desired by the Engineer. Since the degree of compaction depends on the moisture content of the soil, a close watch should be kept on it and corrections done to optimize the moisture content.

The backfilling shall be carried out at optimum moisture content to achieve 95% of Proctor's Maximum Dry Density.



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- c. Plinth filling: The plinth shall be filled with earth in layers not exceeding 250 mm in loose thickness, watered and compacted with approved compaction machine or manually, if specifically permitted by the Engineer. When the filling reaches the finished level, the surface shall be flooded with water for at least 24 hours, allowed to dry and then rammed and compacted, in order to avoid any settlement at a later stage. The finished level of the filling shall be trimmed to the slopes intended to be given to the floor.
- d. Filling in trenches for water pipes and drains: Earth used for filling shall be free from salts, organic or other foreign matter. All clods of earth shall be broken or removed. Where excavated material is mostly rock, the boulders shall be broken into pieces not bigger than 150 mm size in any direction, mixed with fine material consisting of disintegrated rock, moorum or earth as available, so as to fill up the voids as far as possible and then the mixture used for filling. Filling in trenches for pipes and drains shall be commenced as soon as the joints of pipes and drains have been tested and passed. Where the trenches are excavated in soil, the filling shall be done with earth on the sides and top of pipes in layers not exceeding 150 mm, watered, rammed and compacted taking care that no damage is caused to the pipe below.
- e. Filling in disposal area: Surplus material from excavation which is not required for backfilling shall be disposed of in designated disposal areas within the lead for disposal as 5 km crow fly distance from the site. The spoils shall not be dumped haphazardly but should be spread in layers approximately 250 mm thick when loose and compacted with the help of compacting equipment. In wide areas, rollers shall be employed and compaction done to the satisfaction of the Engineer at the optimum moisture content which shall be checked and controlled by the contractor.
In certain cases, the Engineer may direct disposal without compaction which can be done by tipping the spoils from a high bench neatly maintaining always a proper level and grade of the bench.

20.07.00 Approaches and Fencing : The contractor should provide and maintain proper approaches for workmen and for inspection. The roads and approaches around the excavated pits should be kept clear at all times so that there is no hindrance to the movement of men, material and equipment of various agencies connected with the plant. Sturdy and elegant fencing shall be provided around the top edge of the excavation as well as the bottom of the fill at the surplus disposal area where dumping from a high bench is in progress.

20.08.00 Lighting: Full scale area lighting should be provided if night work is permitted or directed by the Engineer. If no night work is in progress, red warning lights should be provided at the corners of the excavated pit and the edges of the filling.

20.09.00 Testing and Acceptance Criteria

- a. Excavation: On „completion of excavation, the dimensions of the pits shall be checked as per the drawings. After the pits are completely dewatered, the



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work shall be accepted after all undercuts have been set right and all over excavations filled back to required lines, levels and grades by placing ordinary concrete of M10 proportion and/ or richer and/ or by compacted earth, as directed by the Engineer. Over excavation of the sides shall be made good while carrying out the back-filling. The excavation work shall be accepted after the above requirements are fulfilled and all temporary approaches encroaching inside the required dimension of the excavation have been removed.

- b. Back-filling: The degree of compaction required shall be as per IS standards and the actual method of measuring the compaction achieved shall be as decided by the Engineer. The work of back-filling shall be accepted after the Engineer is satisfied with the degree of compaction achieved.



21.00.00 CEMENT, CONCRETE & ALLIED WORKS

- 21.01.00**
- a. This part covers the requirements for general use of Plain and Reinforced Cement Concrete work in structures at all locations, cast-in-situ or precast, including all incidental items of work not shown or specified but reasonably implied or necessary for the completion of the work. The special requirements of structures covered in latest IS:456 should also be complied with.
 - b. The works to be provided by Contractor:
 - i) Furnish all labour, supervision services including facilities as required under statutory labour regulations, materials, forms, templates, supports, scaffolds, approaches, aids, construction equipment, tools and plant, transportation etc. required for the work.
 - ii) Prepare progressively detailed drawings and bar bending schedules for reinforcement bars showing the positions and details of spacers, supports, chairs, hangers etc.
 - iii) Submit shop drawings for various inserts, anchors, anchor bolts, pipe sleeves, embedment, hangers, openings, frames etc.
 - iv) Submit detailed drawings of supports, templates, hangers etc. required for installation of various embedment like inserts, anchor bolts, pipe sleeves, joint seals, hangers, openings, frames etc.
 - v) Submit detailed schemes of all operations required for executing the work e.g. material handling, concrete mixing, placement of concrete, compaction, curing, services, approaches etc.
 - vi) Design and submit concrete mix designs required to be adopted on the job.
 - vii) Provide all incidental items necessary for successful completion of the work in accordance with the drawings, specifications and schedule of Items.
 - ix) Supply of specialized materials as directed by the Engineer with a guarantee in approved performa for satisfactory performance.
 - x) Furnish samples and submit for approval the results of various properties of the following materials :
 - a. The various ingredients of concrete including concrete
 - b. Embedments.
 - c. Joint seals

21.02.00 AFTER AWARD OF WORK ACTIVITIES

The following information and data including samples where necessary shall be submitted by the bidder progressively during the execution of contract:



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a) **Programme of execution and Requirement of Materials.**

Within 15 days of the award of contract ,the Contractor will submit a Master Programme for completion of the work giving month wise requirements of materials , particularly mentioning in details the materials which are to be supplied by the Owner and for the procurement of which the help of the Owner is required as per the terms and conditions of the contract.

The master Programme may have to be reviewed and updated by the Contractor quarterly or at more frequent intervals as may be directed by the engineer depending on the emergency of the work.

b) **Samples**

Materials supplied by the Contractor have to be tested for physical, chemical and other properties in the reputed labs at their cost. If found unsuitable, the same shall be removed from the site. Samples of the following materials & any other materials proposed to be used, shall be submitted as directed by the Engineer in Charge, in sufficient quantities free of cost for approval. Approved samples will be preserved by the Engineer in Charge for future reference. The approval of the Engineer in Charge shall not, in any way, relieve the Contractor of his responsibility of supplying materials of specified qualities :-

- i) Coarse and fine aggregates.
- ii) Cement.
- iii) Reinforcement.
- iv) Admixtures.
- v) Plywood for formwork.
- vi) Embedded & anchorage materials as may be desired by the Engineer in Charge.
- vii) Joint sealing strips and other waterproofing materials.
- viii) Joint filling compounds.
- ix) Foundation quality Rubber Pads.

c) **Design mix**

Design mix as per the specification giving proportions of the ingredients, sources of aggregates & cement, along with test results of trial mixes as per relevant I.S. is to be submitted to the Engineer in Charge for his approval before it can be used on the works.



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- a. Detailed drawings & designs of formworks to be used.
- b. Detailed drawings for templates & temporary supports for embedments.
- c. Test reports for cement, reinforcement steel and formwork including inspection reports.
- d. Any other data as per specification.

d) Conformity with Design

The contractor should prepare check lists in approved performa which will be known as “Pour Cards” listing out all items of work involved. The contractor should inform the Engineer, sufficiently in advance, whenever any particular pour is ready for concreting. He shall accord all necessary help & assistance to the Engineer for checking required in the pour. On satisfying himself that all details are in accordance to the drawings & specifications, the Engineer will give written permission allowing the contractor to commence placement of concrete. Details of all instructions issued by the Engineer and the records of compliance by the contractor; deviations allowed by the Engineer and any other relevant information should be written on accompanying sheets attached to the Pour Cards. These sheets, termed as “Progress Cards” shall be prepared by the contractor on approved performa. The Pour Cards alongwith accompaniments shall be handed over to the Engineer before starting placement of concrete. One of mix designs developed by the contractor as per the I.S. specifications and established to the satisfaction of the Engineer by trial mixes shall be permitted to be used by the Engineer, the choice being dictated by the requirements of designs and workability. The methods of mixing, conveyance, placement, vibration, finishing, curing, protection and testing of concrete shall be to Engineer’s approval.

21.03.00 Materials to be Used / Supplied by Contractor

All materials whether to be incorporated in the work or used temporarily for the construction shall conform to the relevant IS Specifications unless stated otherwise and be of best approved quality.

a) Cement

Ordinary Portland Cement (OPC) Grade 43 shall be used for all structures.

For mix design, water cement ratio would not exceed 0.45. Chloride free sulphonated melamine based super plasticizer conforming to IS:9103 shall be added as per mix design and manufacturer’s instructions to improve workability.

b) Coarse Aggregate

Aggregate of sizes ranging between 4.75 mm & 150 mm shall be termed as Coarse Aggregate. Only Coarse Aggregate from approved quarries & conforming to IS: 383 shall be allowed to be used on the works. Aggregates shall be washed to make it free



from deleterious materials, if necessary.

c) **Fine Aggregate**

Aggregate smaller than 4.75 mm and within the grading limits and other requirements set in IS: 383 is termed as Fine Aggregate or Sand. Only Fine Aggregate from approved sources and conforming to the above IS Specification shall be allowed to be used on works.

In certain cases crushed stone sand may be added to natural sand in order to achieve the required grading with prior approval of the Owner.

Crushed stone sand alone may be used only with the prior approval of the Engineer for filling and PCC works only.

d) **Water**

Water for use in Concrete shall be clear and free from injurious oils, acids, alkalis, organic matter, salt, silts and other impurities. Normally potable water is found to be suitable. Generally, IS: 3550 shall be followed for routine tests. In case of doubt, the acceptance test for water shall be as per IS: 3025 and Table-I of IS: 456.

f) **Admixture**

Only admixtures of approved quality shall be used when directed or permitted by the Engineer in Charge. The admixture shall conform to IS: 9103.

g) **Reinforcement**

The reinforcement used shall be cold worked steel high strength deformed bars of grade Fe 415/Fe 500 conforming to IS:1786- latest.

21.04.00 Storage of Materials

a) All materials shall be so stored as to prevent deterioration or intrusion of foreign matter and to ensure the preservation of their quality and fitness for the work. Any material, which has deteriorated or has been damaged or is otherwise considered defective by the Engineer, shall not be used for concrete and shall be removed from site immediately. The contractor shall maintain up-to-date accounts of receipt, issue and balance (stack wise) of all materials. Storage of materials shall conform to IS: 4082.

b) Cement shall be stored off the ground in dry, leak proof, well-ventilated ware-house at the works in such a manner as to prevent deterioration due to moisture or intrusion of foreign matter. Sufficient space of storage with open passages should be arranged between stacks. Cement shall be stored in easily countable stacks with consignment identification marks arranged in the order



of their receipts at site. Sub standards or partly set cement shall not be used; and shall be removed from the site, as soon as it is detected.

- c) Aggregates shall be stored on planks or on steel plates or on concrete or masonry surface. Each size shall be kept separated with wooden or steel or concrete or masonry bulk-heads or in stockpiles stacks and sufficient care shall be taken to prevent the material at the edges of the stockpiles from getting intermixed. Stacks of fine & coarse aggregate shall be kept sufficiently apart with proper arrangement of drainage. The aggregates shall be stored in easily measurable stacks of suitable depths
- d) Reinforcing shall be stored consignment –wise and size wise off the ground & under cover. It shall be protected from rusting, oil, grease and distortions.

The stacks shall be easily measurable. Steel needed for immediate use shall only be removed from storage.

21.05.00 Quality Control

Contractor shall establish and maintain quality control for different items of work and material to assure compliance with spec requirements and submit to the Engineer. The quality control operation shall include but not be limited to the following items of work :-

- a) Admixture : Type, quantity, physical and chemical properties that affect strength, workability and durability of concrete. For air entraining admixtures, doses to be adjusted to maintain air contents within desirable limits.
- b) Aggregates : Physical, chemical, mineralogical qualities, grading moisture contents and impurities.
- c) Water : Impurities test.
- d) Cement : Test to satisfy relevant IS: Specification
- e) Form work : Material shape, dimension, lines, elevation, surface finish, adequacy of form, ties, bracing and shoring & coating.
- f) Reinforcement : Shape, Dimension, Length of splices, clearances ties and supports. Quality & requirement of welded splices. Material test and certificates to satisfy relevant IS: Specification.
- g) Grade of concrete : Usage & Mixdesign. Testing of all



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- h) Batching & Mixing : properties.
Types & capacities of Plant, concrete mixtures and transportation equipment.
- i) Joints : Location of joints, water stops and filler quality and materials. Dimension of joints, shape of joints material and splices.
- j) Embedments & anchorage items : Material, shape, location and setting.
- k) Placing : Preparation, rate of pouring, weather limitations, time intervals between mixing and placing and between two successive lifts, covering over dry or wet surfaces, cleaning and preparation of surfaces on which concrete is to be placed, application mortar, slurry for proper bond, prevention of cold joints and type of chutes or conveyors.
- l) Compaction : Number of vibrators, their prime movers, frequency and amplitude of vibration, diameters and weight of vibrators, duration of vibrator, hands spreading and rodding and tamping.
- m) Setting of base : Lines, elevation and bedding mortar.
- n) Concrete finishes : Repair of surface defects, screening, floating, steel trowelling and brooming special finishes.
- o) Curing : Methods and length of time.

21.06.00 Concrete Mix Requirements

Concrete mix requirements shall be in accordance with IS:456 and as supplemented and modified herein or by other best possible standards.

- a) Washing & Screening of coarse and fine aggregates shall be carried out to remove fines, dirt and other deleterious material.
- b) Admixture: All concrete shall be designed for normal rate of setting and hardening at normal temperature and humidity under different climatic conditions. Admixtures shall be used in accordance with IS:456 to modify the rate of hardening, to improve workability or an aid to control quality. The Engineer reserves the right to order for laboratory test or use test data or



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other satisfactory reference before granting approval. The admixture shall be used strictly in accordance with the manufacturer's directions and/or as desired by the Engineer.

- c) Concrete grades to be used in different parts of work shall be as shown on the drawings or as per the Engineer's instructions. In case of liquid retaining structures, IS:3370 shall be followed.
- d) Proportioning of ingredients of concrete shall be made either with preliminary test by designing the concrete mix ["Design Mix Concrete"] or without preliminary test adopting nominal concrete mix. Design mix concrete shall be used on all concrete works while nominal mix concrete, as per IS:456, may be used as shown on drawings and approved by the Engineer. In all cases, the proportioning of ingredients and works control shall be in accordance with IS:456 and its adequacy after obtaining Engineer's approval.

e) **Mix Design Concrete**

Concrete mixes shall be designed by the contractor to achieve the strength, durability and workability economically with various ingredients. In general the design will keep in view the following considerations :

- i) Consistent with the various other requirements of the mix, the quantity of water should be kept at the lowest possible level.
- ii) The nominal maximum size of coarse aggregate shall be as large as possible within the limits specified.
- iii) The various fractions of coarse and fine aggregates should be mixed in such a proportion as to produce the best possible combined internal grading giving the densest and most workable mix.
- iv) Chemical admixtures may be used to modify the rate of hardening and improve workability.
- v) The finished concrete should have adequate durability in all conditions, to withstand satisfactorily the weather and other destructive agencies which it is expected to be subjected to in actual service.
- vi) Concrete Mix Design and Strength requirements

Concrete mix design shall be as per IS: 10262 and SP: 23. The strength requirements of both design mix and nominal mix concrete where ordinary Portland cement or Portland slag cement is used shall be as per Table-2 of IS: 456. All other relevant clauses of IS: 456 shall also apply.



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f) **Minimum cement content**

The minimum cement content for each grade of concrete shall be as shown below:

TABLE – I
MINIMUM CEMENT CONTENT & MAX. W / C RATIO SPECIFIED
FOR
DIFFERENT GRADES OF CONCRETE

Sl no	Class	Conc Grade	Min cement content (kg/cum)	Water, Cement ratio	Slump (mm)
1	Plain concrete used for lean concrete.	M 10	-	-	-
2	Backfill	M 15	250	0.5	-
3	Pavement around buildings, Base plate encasement, etc.	M 20	320	0.55	80+/20
4	Precast concrete	M 25	365	0.50	Do
5	a) Reinforced concrete for structural work in foundation & superstructure	M 30	365 - 400	0.45	Do
	b) Reinforced concrete for Water retaining structure (not in contact with sea water)	M 30	365 - 400	0.45	Do
6	TG top deck, Chimney shell & structures coming in contact of sea water.	M 35	365-400	0.45	Do
7	Natural draft Cooling Tower	M 40	365-400	0.40	Do

The Engineer will always have the unquestionable right to revise the minimum cement content as decided above, if, in his opinion, there is any chance of deterioration of quality on account of use of lower cement content or any other reason.

g) **Water-Cement Ratio**

The choice of water-cement ratio in designing a concrete mix will depend on

- i) The requirement of strength.
- ii) The requirement of durability.

i) **Strength Requirements of Concrete Mix**

In case of 'Design Mix Concrete', the water-cement ratio of such value as to give



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acceptable test results as per IS: 456 will be selected by trial and error. The values of water- cement ratios for different grade and mix designs will have to be established after conducting sufficiently large number of preliminary tests in the laboratory to the satisfaction of the Engineer.

Frequent checks on test will have to be carried out and the water-cement ratios will be revised if the tests produce unsatisfactory results. Notwithstanding anything stated above the Contractor's responsibility to produce satisfactory test results and to bear all the consequences in case of default remains unaltered.

In case of nominal mix concrete, the maximum water-cement ratio for different grades of concrete is specified in Table-9 of IS: 456 and no tests are necessary. The acceptance test criteria for nominal mix concrete shall be as per IS: 456.

ii) **Durability**

Tables 4 of IS:456 give the maximum water-cement ratio permissible from the point of view of durability of concrete subjected to adverse exposure to weather, sulphate attacks and contact with harmful chemicals. Impermeability may also be an important consideration.

Whenever the water-cement ratio dictated by durability consideration is lower than that required from strength criterion, the former shall be adopted.

In general the water cement ratio between 0.4 and 0.45 will be desirable to satisfy the durability requirement and from the consideration of Impermeability of concrete. The contractor may propose lower water cement ratio as mentioned above by addition of a suitable plasticizer / super- plasticizer.

h) **Workability**

The degree of workability necessary to allow the concrete to be well consolidated and to be worked into the corners of formwork and around the reinforcement and embedment's and to give the required surface finish shall depend on the type and nature of structure and shall be based on experience and tests. The limits of consistency for various types of structures as per Table – I With the permission of the Engineer. For any grade of concrete, if the water has to be increased in special cases, cement shall also be increased proportionately to keep the ratio of water to cement same as adopted in trial mix design for each grade of concrete. No extra payment will be made for this additional cement.

The workability of concrete shall be checked at frequent intervals by slump tests. Alternatively where facilities exist or if required by the Engineer, the compacting factor test in accordance with IS: 1199 and Clause 7 of IS: 456 shall be carried out.

i) **Size of Coarse Aggregates**

The maximum size of coarse aggregates for different locations shall be as follows



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unless otherwise directed by the Engineer:

Reinforced concrete except foundation mm	-	20
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Ordinary Plain concrete and Reinforced concrete foundations mm	-	40
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Grading of coarse aggregates for a particular size shall conform to relevant I.S. Codes and shall also be such as to produce a dense concrete of the specified proportions, strength and consistency that will work readily into position without segregation.

Coarse aggregate will normally be separated into the following sizes and stacked separately in properly designed stockpiles 40 mm to 20 mm and 20 mm to 5 mm.

In certain cases it may be necessary to further split the 20 mm to 5 mm fraction into 20 mm to 10 mm and 10 mm to 5 mm fractions.

This separation of aggregates in different size fractions is necessary so that they may be remixed in the desired proportion to arrive at a correct internal

j) **Mixing of Concrete**

Concrete shall always be mixed in mechanical mixer for unimportant works & in small quantities. Water shall not normally be charged into the drum of the mixer until all the cement and aggregates constituting the batch are already in the drum and mixed for atleast one minute. Mixing of each batch shall be continued for uniform distribution of the materials with uniform mass in colour and consistency, but in no case shall mixing be done for less than 2 minutes and at least 40 revolutions after all the materials and water are in the drum. When absorbent aggregates are used or when the mix is very dry, the

mixing time shall be extended as needed. Mixers shall not be loaded above their rated capacity as this prevents thorough mixing.

Controlled concrete only in batching plant shall be used for power house column foundation, TG, boiler foundation, ID, FD, PA, BFP foundations, chimney cooling tower & other important structure.

The entire contents of the drum shall be discharged before the ingredients for the next batch are fed into the drum. No partly set or remixed or excessively wet concrete shall be used. Such concrete shall be immediately removed from site. Each time, the work stops, the mixer shall be thoroughly cleaned & when the next mixing commences, the first batch shall have 10% additional cement at no extra cost to the Purchaser to allow for loss in the drum.

Regular checks on mixer efficiency shall be carried out as per IS: 4634 on all mixers employed at site. Only those mixers whose efficiencies are within the tolerances



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specified in IS: 1791 should be allowed to be employed. Batching Plant where used shall conform to IS: 4925.

Hand mixing if permitted shall be carried out on a water-tight platform and care shall be taken to ensure that mixing is continued until the mass is uniform in colour and consistency. In case of hand-mixing, 10% extra cement shall be added to each batch at no extra cost to the Purchaser.

Crushed ice shall be used in concrete mixing water so as to maintain temperature in the concrete in the top decks of the machine foundations. Arrangements for standby weight batching plant and equipment shall also be made available by the contractor for continuous pour of concrete.

k) **Conveying Concrete**

Concrete shall be handled and conveyed from the place of mixing to the place of laying as rapidly as practicable by approved means, concrete shall be placed and compacted in the final position before the initial setting of the cement starts. Concrete should be conveyed in such a way preventing segregation or loss of any of the ingredients. For long distance haulage, agitator cars of approved design should be used. If, in spite of all precautions, segregation does occur during transport, the concrete shall be properly remixed before placement. During very hot weather, concrete shall be transported in deep container to reduce the rate of loss of water by evaporation or loss of heat. If necessary, the container should be covered. Conveying equipment for concrete shall be well maintained and thoroughly cleaned before commencement of concrete mixing. Such equipment shall be kept free from set concrete.

l) **Placing and Compacting of Concrete**

- i) Relevant I.S. Code should be followed for the procedure of surface preparation, placement, consolidation, curing, finishes, repairs and maintenance of concrete. Concrete shall be placed against the surfaces of formwork or construction joint in concrete or masonry. The surface against which concrete is placed shall be cleaned thoroughly. Old construction joint has to be roughened by wire brushing, chipping sand blasting or any other approved means for proper bond. All cuttings, dirt, oil, foreign and deleterious material, laitance, etc. shall be removed by air water jetting or water at high pressure.

Concrete shall be placed in the formwork by approved means, and shall not be dropped from a height or handled in a manner which may cause segregation.

- ii) Construction joint shall be kept moist for at least 72 hours prior to placement. Concrete shall be placed always against moist surface but never on pools of water. In case the foundation cannot be dewatered completely, special procedure and precaution shall be adopted. Formwork shall be cleaned thoroughly and smeared lightly with form oil or grease of approved quality



just prior to placement.

- iii) 12 mm thick mortar with less w/c ratio as that of the concrete being placed and cement slurry shall be spread thoroughly on the construction joint just prior to placement of concrete. The concrete shall be spread, and thoroughly compacted without segregation and thoroughly worked around shape. Vibrators shall not be used for pushing concrete into adjoining areas. Vibrators must be operated by experienced workmen and the work carried out as per relevant IS. In thin members with heavy congestion of reinforcement of other embedments, where effective use of internal vibrator is, doubtful, in addition to immersion vibrators, the contractor should employ form vibrators conforming to IS: 4656. For slabs and other similar structures, the contractor should employ screed vibrator as per IS: 2506. Care must be taken to ensure that the inserts, fixtures, reinforcement and formwork are not displaced or distorted during placing and consolidation of concrete.
- iv) The rate of placement of concrete shall be such that no cold joint is formed; and fresh concrete is placed always against green, plastic and workable concrete. No concrete shall be placed in open during rains. During rainy season, no placement in the open should be attempted unless sufficient tarpaulins or other similar protective arrangement for completely covering the still green concrete from rain is kept at site of placement. If there has been any sign of washing of cement and sand, the entire affected concrete shall be removed immediately. Suitable precautions shall be taken in advance to guard against rains before leaving the fresh concrete unattended. No accumulation of water shall be permitted on or around freshly laid concrete.
- v) Slabs, beams and similar members shall be poured in one operation. Moulding, throating, drip course, etc., shall be poured as per the drawings. Holes shall be provided and bolts, sleeves, anchors, fastenings or other fixtures shall be embedded in concrete as per drawings. In case the forms or supports get displaced during or immediately after the placement and bring the concrete surface out of alignment beyond tolerance limits, the Engineer may direct to remove the portion and reconstruct or repair the same.

The Engineer shall decide upon the time interval between two placements of concrete of different ages coming in contact with each other, taking in consideration the degree of maturity of the older concrete, shrinkage, heat dissipation and the ability of the older concrete to withstand the load imposed upon it by the fresh placement. Once the concrete is deposited, consolidated and finished in its final position, it shall not be disturbed.

21.07.00

a) Construction Joints

Concrete structure shall be completed by continuous pouring in one operation. However, due to practical limitation of methods and equipment and certain design considerations, construction joints are formed by discontinuing concrete at certain predetermined stages. These joints shall be formed as per drawings. Vertical construction joints shall be made with rigid



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stop-board forms having slots for allowing passage of reinforcement rods and any other embedments and fixtures that may be shown. For water retaining structures and leak proof buildings suitable and approved water stops shall be installed at the construction joints as per clause 12.4 of IS:456. Where the location of the joints are not specified, it shall be in accordance with the following :-

- i) In a column, the joint shall be formed 75 mm below the lowest soffit of the beam framing into it.
- ii) Concrete in a beam shall preferably be placed without a joint, but if provision of a joint is unavoidable, the joint shall be vertical and at the middle of the span.
- iii) A joint in a suspended floor slab shall be vertical and at the middle of the span and at right angles to the principal reinforcement.
- iv) Feather-edges in concrete shall be avoided while forming a joint.
- v) A construction joint should preferably be placed in a low stress zone and at right angles to the direction of the principal stress.

b) Cold Joint

An advancing face of a concrete pour, which could not be covered by fresh concrete before expiry of initial setting time (due to an unscheduled stoppage or delay on account of breakdown in plant, inclement weather, low rate of placement or any other reason), is called a cold joint. The contractor should always remain vigilant to avoid cold joints. If however, a cold joint is formed due to unavoidable reasons, the following procedure shall be adopted for treating it :-

- i) If the concrete is so green that it can be removed manually and if vibrators can penetrate the surface without much effort, fresh concrete should be placed directly against the old surface. The old concrete should be covered by fresh concrete as quickly as possible, and the joint thoroughly and systematically vibrated.
- ii) In case concrete has hardened a bit more than (a) but can still be easily removed by a light hand pick, the surface shall be raked thoroughly and the loose concrete removed completely without disturbing the rest of the concrete in depth. A rich mortar layer 12 mm in thickness, shall be placed on the cold joint, fresh concrete shall be placed on the mortar layer and the joint shall be thoroughly and systematically vibrated penetrating the vibrator deep into the old layer of concrete.
- iii) In case the concrete at the joint has become so stiff that it cannot be remolded and mortar or slurry does not rise inspite of extensive vibration, the



joint shall be left to harden for atleast 12 to 24 hrs. It shall then be treated as a regular construction joint, after cutting the concrete to required shape and preparing the surface as indicated above.

c) Expansion And Isolation Joints

- i) Expansion and isolation joints in concrete structures shall be provided at specific places as shown on the drawings with materials and types of joints as specified. In case of liquid retaining structures, additional precautions shall be taken to prevent leakage of liquids as specified on the drawings. All materials should be procured from reliable manufacturers to the approval of the Engineer. Test certificates for the materials should be furnished. Joints shall be formed true to line, level, shape, dimension and quality as per drawings and specifications. Prior approval from concerned engineer of the method of forming the joints should be obtained.

- ii) Duraboard HD or its equivalent of approved manufacturer shall be used as filler for expansion joints. It must be durable and waterproof. It shall be compressible and possess a high degree of rebound. The dimensions of the board should be equal to that of the joint being formed. It should, preferably be manufactured in one piece, matching the dimension of the joint and not prepared by cutting to size smaller pieces from larger boards at site. At the exposed end, the joint shall be sealed with approved sealing compound to a depth of at least 25 mm after application of an approved primer. The sealing compound and the primer shall be applied as recommended by the manufacturer.

Commercial quality of expanded polystyrene products commonly used for thermal insulation can also be used as filler material in expansion joints. The thickness may vary from 12 mm to 50 mm. The material should be procured from reliable manufacturers as approved by the Engineer. The method of installation shall be similar to that recommended by the manufacturer for fixing on walls. A coat of bitumen paint shall be applied on the board against which concrete is placed.

- iii) Joint sealing strips shall be provided at the construction, expansion and isolation joints as a continuous diaphragm to contain the filler material and/or to exclude passage of water or any other material into or out of the structure. General strips shall be of PVC or otherwise as specified. Sealing strips shall not have any longitudinal joint; and shall be procured and installed in largest practicable lengths having a minimum number of transverse joints. The material shall be procured from reputed manufacturers having proven records of satisfactory supply of joint strips of similar make and shape for other jobs. The jointing procedure shall be as per the manufacturer's recommendations. The contractor should supply all labour and materials for installation including the material and tools required for jointing, testing, protection, etc. The joints in rubber seals shall be vulcanised as needed.



- iv) The minimum thickness of P.V.C. sealing strips shall be 3 mm and the minimum width 100mm. The actual size and shape shall be as shown in drawings. The material should be of good quality Polyvinyl Chloride highly resistant to tarring, abrasion and corrosion as well as to chemicals likely to come in contact with during use. The physical properties shall generally be as follows:
- i) Specific Gravity : 1.3 to 1.35
 - ii) Shore Hardness : 60A to 80A
 - iii) Tensile Strength : 10 to 15 N/Sq.mm
 - iv) Maximum Safe Continuous Temperature : 70 Deg.C
 - v) Ultimate Elongation : Not less than 275%
 - vi) Bitumen Compound: When directed, the gap in expansion joints shall be thoroughly cleaned and bitumen compound laid as per Manufacturer's recommendation. The compound to be used shall be of approved manufacture and shall conform to the requirements of IS: 1834.
 - vi) Isolation Joints: Strong and tough alkathene sheet or equivalent, about 1 mm in thickness shall be used in isolation joints. It shall be fixed by an approved adhesive compound on the cleaned surface of the already set concrete, to cover it fully. Fresh concrete shall be laid against the sheet, care being taken not to damage the sheet in any way.
 - vii) Rubber Pad: Hard foundation quality rubber pads of required thickness and shapes shall be put below machine or other foundations as shown on the drawings. The rubber shall have a unit weight of 1500 kg/cum, shore hardness – 65A to 70A and be of best quality of approved manufacture, durable, capable of absorbing vibration and must be chemically inert in contact with moist or dry earth or any other deleterious material expected under normal conditions.
 - viii) All foundation structures in contact with soil shall be coated with two coats of Bituminous coating.

21.08.00 Repairs, Finishes and Treatment of Concrete Surfaces

- a) Adequate and sound concrete surfaces, whether formed or unformed, should



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be obtained by employing a concrete mix of proper design, competent formwork, appropriate methods of handling, placing and consolidation by experienced workmen.

- b) Unsound concrete resulting from improper mix design, incompetent methods, equipment and form work, poor workmanship and protection shall not be accepted and shall be dismantled, removed and replaced by sound concrete at the contractor's cost. All concrete work shall be inspected by the contractor immediately after the forms are removed and he should promptly report of occurrence of any defects to the Engineer. All repair works should be carried out as per the instructions and in the presence of the Engineer. Generally, repair work shall consist of any or all of the following operations :
- i) Sack rubbing with mortar and stoning with carborundum stone.
 - ii) Cutting away the defective concrete to the required depth and shape.
 - iii) Cleaning of reinforcement and embedments.
 - iv) Roughening by sand blasting or chipping.
 - v) Installing additional reinforcement/ welded mesh fabric.
 - vi) Dry packing with stiff mortar.
 - vii) Plastering, guniting, shotcreting etc.
 - viii) Placing and compacting concrete in the void left by cutting out defective concrete.
 - ix) Grouting with cement sand slurry of 1:1 mix.

c) Finishing Unformed Surface

The requirement of finishes of formed surfaces are specified hereunder separately. The contractor should include for concrete, the provision of normal finishes in unformed surfaces which can be achieved by screeding, floating, trowelling etc. A few typical and common cases of treatment of concrete surface are cited below :

- i) Floor: For non-integral floor finish, the surface of reinforcement concrete slab shall be struck off at the specified levels and slopes and finished with a wooden float fairly smooth removing all laitance. No over trowelling, to obtain a very smooth surface, shall be done as it will prevent adequate bond with the subsequent finish. The surface shall be scored and marked to provide better bond.



For monolithic finish, concrete shall be compacted and struck off at the specified levels and slopes with a screed, preferably a vibrating type; and then floated with a wooden float. Steel trowelling should then started after the moisture film & shine disappeared from the surface; and after the concrete has hardened enough to prevent excess of fines and water to rise to the surface but not hard enough to prevent proper finishing of aberrations. Steel trowelling properly done should flatten and smoothen sandy surface left by wooden floats and produce a dense surface free from blemishes, ripples and trowel marks. A fine textured surface that is not slick and can be used where there is likelihood of spillage of oil or water should be obtained by trowelling the surface lightly with a circular motion after initial trowelling keeping the steel trowel flat on the surface. To provide a better grip, the floor should be marked in a regular geometric pattern after initial trowelling.

ii) Beams, Columns & Walls: If on any other concrete structure, it is intended to apply plaster or such concrete surfaces against which brick- work or other allied works are to be built, the contractor shall hack the surface adequately as soon as the form is stripped off so that proper bond can develop.

d) Protection and Curing of concrete: Newly placed concrete shall be protected from rain, sun and wind. Concrete placed below the ground level shall be protected against contamination from falling earth during and after placing. Concrete placed in ground containing deleterious substances, shall be protected from contact with such ground, or with water draining from such ground, during placing of concrete and for a period of at least three days. Steps shall be taken to protect immature concrete from damage by debris, excessive loadings, vibration, abrasion, mixing with earth or other deleterious materials, etc. that may impair the strength and durability of the concrete.

As soon as the concrete has hardened sufficiently, it shall be covered either with sand, hessian, canvas or similar materials and kept continuously wet for at least 14 (fourteen) days after final setting. Curing by continuous sprinkling of water shall be allowed if the Engineer is satisfied with the adequacy of the arrangements made by the contractor.

The contractor shall remain extremely vigilant and employ proper equipment and workmen under able supervision for curing. In case any lapse on the part of the contractor is noticed, the Engineer will inform the contractor verbally or in writing to correct the deficiency in curing. If no satisfactory action is taken by the contractor within 3 hours of issuance of such instruction, the Engineer will be at liberty either to employ sufficient means through any agency to make good the deficiency and recover the cost thereof from the contractor.

21.09.00

a) **Reinforcement**

The reinforcement used shall be cold worked steel high strength deformed



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bars of grade Fe 415/ Fe 500 conforming to IS:1786 – latest.

All steel for reinforcement shall be free from loose scales, oil, grease, paint or other harmful matters immediately before placing the concrete.

b) Bar Bending Schedules

The contractor shall submit to the Engineer bar bending schedules with working drawings showing clearly the arrangements proposed by the contractor. Upon receipt of the Engineer's final approval of the bar bending schedule and drawings, the contractor shall submit the final drawings with one reproducible print after incorporating necessary modifications or corrections, for final record and distribution. Approval of such detailed drawings shall not relieve the contractor of his responsibility for correctness nor of any of his obligations to meet the other requirements of the contractor.

c) Bending

Reinforcing steel shall be bent in accordance with the procedure specified in IS:2502. Bends and shapes shall comply strictly with the dimensions corresponding to the approved bar bending schedules which shall be rechecked by the contractor before any bending is done.

No reinforcement shall be bent when already in position in the work, without approval of the Engineer, whether or not it is partially embedded in concrete. Bars shall not be straightened in a manner that will injure the material. Re-bending can be done only if approved by the Engineer. Reinforcing bars shall be bent by machine or other approved means producing a gradual and even motion. All the bars shall be cold bent.

d) Placing in position

All reinforcements shall be accurately fixed and maintained in position as shown on the drawings. Bars at crossing points, shall be securely tied together by # 20G annealed soft iron PVC coated wire or by tack welding in case of bar larger than 25 mm dia. binders shall tightly embrace the bars with which they are intended to be in contact and shall be securely held. The vertical distance between successive layers of bars shall be maintained with FBEC spacer bars. They should be spaced such that the main bars do not sag perceptibly between adjacent spacers. Before actual placing, the contractor shall study the drawings thoroughly and inform the Engineer in case he feels that placement of certain bars is not possible due to congestion. In such cases, he should not start placing any bar before obtaining clearance from the Engineer.

e) Welding

Normal bond laps in reinforcement should be placed by lap or butt welding reinforcement bars. The work should be done with suitable safe guards as per



IS for welding of mild steel bars used in reinforced concrete construction as per IS: 2751 and IS: 456. Welded mesh fabrics to IS:566 shall also be as per drawings.

f) **Control**

The placing of reinforcements shall be completed well in advance of concrete pouring. Immediately before pouring, the reinforcement shall be examined by the Engineer for accuracy of placement and cleanliness. Necessary corrections as directed by him shall be carried out. Laps and anchorage lengths of reinforcing bars shall be as per IS: 456. If the bars in a lap are not of the same diameter, the smaller will guide the lap length. The laps shall be staggered as far as practicable. Arrangements for placing concrete shall be such that reinforcement in position do not have to bear extra load and get disturbed.

The cover for concrete over the reinforcements shall be as shown on the approved drawings. Where concrete blocks are used for ensuring the cover and positioning reinforcement, they shall be made of mortar not leaner than 1part cement to 2 parts sand by volume and cured in a pond for at least 14 days. The type, shape, size and location of the concrete blocks shall be as approved by the Engineer.

g) **Cold Weather Concreting**

When conditions are such that the ambient temperature may be expected to be 4.5 Deg.C or below during the placing and curing period, the work shall conform to the requirements of clause 13 of IS: 456 and IS: 7861.

h) **Hot Weather Concreting**

When depositing concrete in very hot weather, the contractor shall take all precautions as per IS: 7861 and stagger the work to the cooler parts of the day to ensure that the temperature of wet concrete used in massive structures does not exceed 38oC while placing.

21.10.00 Form Work

- a) The contractor shall prepare, before commencement of actual work, designs and working drawings for formwork and centering. The formwork shall conform to the shape, grade, lines, levels and dimensions as shown on the drawings. Materials used for the formwork inclusive of the supports and centering shall be capable of withstanding the working load and remain undistorted throughout the period it is left in service. All supports and scaffolds should be manufactured from structural or tubular steel.
- b) The centering shall be true to vertical, rigid and thoroughly braced both



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horizontally and diagonally. Rakes shall be used where forms are to support inclined members. The forms shall be sufficiently strong to carry without undue deformation, the dead weight of the concrete as a liquid as well as the working load. Where the concrete is vibrated, the formwork shall be strong enough to withstand the effects of vibration without appreciable deflection, bulging, distortion or loosening of its components. The joints in the formwork shall be sufficiently tight to prevent any leakage of slurry or mortar.

To achieve the desired rigidity, tie bolts, spacer blocks, tie wires and clamps shall be used but they must in no way impair the strength of concrete or cause stains or marks on the finished surface. Where there are chances of these fixtures being embedded, only mild steel or concrete of adequate strength shall be used. Bolts passing completely through liquid retaining walls/ slabs for the purpose of securing and aligning the formwork shall not be used.

- c) The formwork shall be such as to ensure a smooth uniform surface free from honeycombs, air bubbles, bulges, fins and other blemishes. Any blemish or defect found on the surface of the concrete must be brought to the notice of the Engineer immediately and rectified as directed by him.

For exposed interior and exterior concrete surfaces of beams, columns and wall, plywood or other approved form shall be thoroughly cleaned and tied together with approved corrosion-resistant devices. Rigid care shall be exercised in ensuring that all column forms are plumb and true and thoroughly cross braced to keep them so. All floor and beam centering shall be crowned not less than 8 mm in all directions for every 5 meters span. Bevelled forms 25 mm x 25 mm shall be fixed in the form-work at all corners to provide chamfering of the finished concrete edges. The formwork should lap and be secured sufficiently at the lift joints to prevent bulges and offsets.

Temporary openings for cleaning, inspection and for pouring concrete shall be provided at the base of vertical forms and at other places as necessary. The temporary openings shall be so formed that they can be conveniently closed when required, during pouring operations without leaving any mark on the concrete.

d) **Cleaning and Treatment of Forms**

All parts of the forms shall be thoroughly cleaned of old concrete, wood shavings, saw dust, dirt and dust sticking to them before they are fixed in position. All rubbish, loose concrete, chippings, shavings, saw dust etc. shall be scrupulously removed from the interior of the forms before concrete is poured. Compressed air jet and/ or water jet along with wire brushes, brooms etc. shall be used for cleaning. The inside surface of the formwork shall be treated with approved non-staining oil or other compound before it is placed in position. Care shall be taken that oil or other compound does not come in contact with reinforcing steel or construction joint surfaces. They shall not be allowed to accumulate at the bottom of the formwork.



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e) Design

The formwork shall be so designed and erected that the forms for slabs and the sides of beams, columns and walls are independent of the soffits of beams and can be removed without any strain to the concrete already placed or affecting the remaining formwork. Removing any props or reproping shall not be done except with the specific approval of the Engineer. If formwork for column is erected for the full height of the column, one side shall be left open and built up in sections, as placing of concrete progress. Wedges, spacer bolts, clamps or other suitable means shall be provided to allow accurate adjustment and alignment of the formwork and to allow it to be removed gradually without jarring the concrete.

f) Inspection of Forms

Casting of concrete shall start only after the formwork inspection and approval by the Engineer. The concreting shall start as early as possible within 3 days after the approval of the formwork; and during this period, the formwork shall be kept under constant vigilance against any interference. In case of delay beyond three days, a fresh approval from the Engineer shall be obtained.

g) Removal of Forms

Before removing any formwork, the contractor must notify the Engineer well in advance to enable him to inspect the concrete if he so desires. The contractor shall record the date on which concrete is placed in each part of the work; and the date on which the formwork is removed there from. The contractor shall be responsible for the safe removal of the formwork; and any work showing signs of damage through premature removal of formwork or loading shall be rejected and entirely reconstructed by him. Forms for various types of structural components shall not be removed before the minimum periods as detailed below:

SCHEDULE OF REMOVAL OF FORM

Part of Structure	Ordinary Portland Cement Concrete				Rapid Hardening Portland Cement Concrete			
	Temperature ($^{\circ}\text{C}$)				Temperature ($^{\circ}\text{C}$)			
	Above 40°C	40°C to 20°C	20°C to 5°C	Below 5°C	Above 40°C	40°C to 20°C	20°C to 5°C	Below 5°C
	Days	Days	Days	Days	Days	Days	Days	Days
a) Columns & Walls	2	1	1	Do not remove forms until	1	1	1	Do not remove forms
b) Beam sides	3	2	3		2	1	1	



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c) Slabs, 125 mm	10	7	8	site cured test specimen develop atleast 50% of the specified 28 days strength	7	4	5	until site cured test specimen develop atleast 50% of the specified 28 days strength
d) Slabs over 125 mm thick and soffit of minor beams	18	14	16		12	8	9	
e) Soffit of main beams	24	21	22		14	10	12	

Wherever exposed surfaces of concrete can be effectively sealed to prevent loss of water, the periods specified for temperature above 40oC can be reduced to those of the temperature range of 20oC to 40oC.

Construction joints in beams shall be located at the middle of span. In such cases, however, entire span of beam shall have to be kept supported by formwork till its removal for the portion of beam, cast at a later date.

h) Tolerance

The formwork shall be so made as to produce a finished concrete, true to shape, lines, levels, plumb and dimensions as shown on the drawings subject to the following tolerances :-

- | | | | |
|------|---------------------|---|---------------------------------|
| i) | Sectional dimension | - | ± 5 mm |
| ii) | Plumb | - | 1 in 1000 of height |
| iii) | Levels | - | ± 3 mm before any has taken |

deflection
place

The above tolerance shall be for local aberrations in the finished concrete surface, and should not be taken as tolerances for the entire structure taken as a whole or for the setting and alignment of formwork, which should be as accurate as possible. Any error, within the above tolerance limits if noticed in any lift of the structure after stripping of forms, shall be corrected in the subsequent work to bring back the surface of the structure to its true alignment.

i) Re-use of Forms

Before re-use, all forms shall be thoroughly scraped, cleaned, joints and planes examined and when necessary repaired and inside surface treated as specified hereinbefore. Formwork shall not be used / re-used if declared unfit or unserviceable by the Engineer.

j) Generally, the “ordinary” class formwork shall be used in places where



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ordinary surface finish is required and shall be composed of steel and/ or approved good quality partially seasoned timber. Plywood formwork shall be used in exposed surfaces, where a specially good finish is required and shall

be made of approved brand of heavy quality plywood to produce a perfectly uniform and smooth surface conforming to the shape described in the drawing with required grain texture on the concrete. Re-use may only be permitted after special inspection and approval by the Engineer. He may also permit utilization of used plywood for the “ordinary” class, if it is still in good condition. Ornamental formwork shall be used where ornamental and curved surfaces are required and shall be made of selected best quality well seasoned timber or plywood, which can be shaped correctly. The contractor shall leave all openings, grooves, chases, etc. in concrete work as shown on the drawings.

21.11.00 Anchor Bolts, Anchors, Sleeves, Inserts, Hangers / Conduits / Pipe and Other Misc.Embedded Fixtures

The contractor shall build into concrete work all the items as noted below; and shall embed them partly or fully as directed and secure the same as may be required. The materials shall be as specified and be of best quality available according to relevant IS and of approved manufacture to the satisfaction of the Engineer. Exposed surfaces of embedded materials shall be painted with one coat of approved anti-corrosive paint and/or bituminous paint. If welding is to be done subsequently on the exposed surfaces of embedded material, the paint shall be cleaned off the member to a minimum length of 50 mm beyond each side of the weld line. Necessary templates, jigs, fixtures, supports etc. shall be used as required items to be embedded shall be as follows:

- i) Inserts, hangers, anchors, frames around openings, manhole covers, Frames, floor clips, sleeves conduits and pipes.
- ii) Anchor bolts and plates for machinery, equipment and for structural steel work.
- iii) Dowel bars, etc. for concrete work
- iv) Lugs or plugs for door and window frames occurring in concrete work. v) Flashing and jointing in concrete work.
- vi) Any misc. embedments and fixture as may be required.
- vii) Convextra-GP2, Sikka grout or equivalent grouting material shall be used as per manufacturer’s specification.



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Correct location and alignment, as per drawings/instructions of all these embedded items shall be entirely the responsibility of the contractor.

21.12.00 Precast Concrete

The precast concrete shall be similar as for the cast-in-place concrete described herein. All precast work shall be carried out in a yard which shall be dry, properly levelled and having a hard and even surface. It shall be paved with concrete or masonry as needed; and provided with a layer of plaster (1:2 proportion) with smooth neat cement finish or a layer of M.S. sheeting. Casing shall be done on suitable vibrating table. The yard, lifting equipment, curing tank, finished material storage space etc. shall be designed such that the units are not lifted from the mould before 7 days of curing and can be removed for erection after 28 days of curing. The moulds shall be of steel or of timber lined with G.I. sheet metal.

Lifting hooks as necessary shall be embedded in correct position of the units to facilitate erection, even though they may not be shown on the drawings, and shall be burnt off and finished after erection. Precast concrete units, when ready, shall be transported to site by suitable means. Care shall be taken to ensure that no damage occurs during transportation. All adjustments, levelling and plumbing shall be done as per instructions of the Engineer. The contractor shall render all help with instruments, materials and men to the Engineer for checking the proper erection of the precast units.

21.13.00 Sampling and Testing

The contractor shall carry out all sampling and testing as per IS for the following items:

a) Cement

Representative samples shall be taken from each consignment of cement received from the manufacturer / supplier for carrying out the tests for fineness (by hand sieving), setting time and compressive strengths as per IS:269. Soundness Tests shall also be carried out as required. No cement from a particular consignment / batch shall be used on the works unless satisfactory 3 days and 7 days test results for compressive strength are known. These tests shall be of great importance as their results shall have bearing on the acceptance of concrete or otherwise.

b) Aggregates

The contractor shall carry out any or all the tests of aggregates as required as per IS:2386 Parts-I to VIII. The acceptance criteria of the samples tested shall be as per the relevant IS.

c) Water



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Water for concrete works shall be tested as per IS: 3550 by the contractor at regular intervals and whenever directed by the Engineer. The final acceptance criteria in case of doubt shall be as per IS: 3025 & IS: 456.

d) Admixture

Air Entraining Agents (AEA): Initially, before starting to use A.E.A., relationship between the percentage of air entrained and the cylinder cube crushing strength vis-à-vis quantity of A.E.A. used for all types of concrete shall be established by the contractor by carrying out sufficiently large number of tests. After than, at regular intervals and whenever directed by the Engineer, the contractor should check up, the actual percentages of air entrained and corresponding crushing strengths to correlate with the earlier test results. Tests for establishing the various properties of any other admixtures which may be required to be added shall be carried out by the contractor.

e) Concrete

The sampling of concrete, making the test specimens, curing and testing procedure etc. shall be as per IS:516 and IS:1199 with the size of specimen being 15 cm cubes. Normally, only compression tests shall be performed apart from other tests as per IS:516. Sampling procedure, frequency of sampling and test specimen shall conform to clause 15 of IS: 456. To control the consistency of concrete from every mixing plant, slump tests and / or compacting factor tests in accordance with IS:1199 and as per clause 7.3.7(f) above shall be carried out by the contractor every 2 hours. Slumps corresponding to the test specimens shall be recorded for reference. The acceptance criteria of concrete shall be in accordance with clause 16 of IS:456. Concrete work found unsuitable for acceptance shall have to be dismantled and replacement shall be done as per specification by the contractor. In the course of dismantling, if any damage is done to the embedded items or adjacent structures, the same shall be made good by the contractor to the satisfaction of the Engineer.

f) Concrete for Equipment or Steel Structures Foundations

Concrete for equipment foundation, whether principal or auxiliary, shall be poured continuously so that the structure becomes monolithic, particular care being exercised to see that the base slabs, if any, are of compact impervious construction. Tunnels, passages apertures and so forth shall be provided in accordance with the drawings for the installation of mechanical and electrical equipment, pipes or cables. The top elevation of the equipment foundations or parts shall be accurately cast to 20/50 mm (or more as may be specified on the drawings) above the level required for grouting and it shall be pneumatically chiseled of and well roughened just prior to the erection of the equipment concerned. All embedded anchor bolts or bolt sleeves shall be accurately and firmly set with the aid of approved templates, steel supports



and / or other accessories. For holding the embedded bolts or sleeves in the correct position during concreting, template shall have to be of steel of suitable section approved by the Engineer in Charge. Two (2) sets of templates shall have to be provided, one to hold the bottom and the other the top of the bolts or sleeves. The bottom template shall be securely and rigidly fixed by providing anchorage arrangement and by welding to the lowest part of the steel reinforcement and other structural supports. The top templates shall be securely fixed by tying with guy wires and turn buckle arrangement to firm and rigid adjoining structures and stagings.

Bolt pockets, where required, shall be cast with wooden taper wedges. These shall be withdrawn at an appropriate time when the concrete has set, the pockets cleaned roughened and then covered or blocked thoroughly to prevent debris getting into these. The exposed portions of bolts and embedded parts shall be kept well greased and adequately protected from damage throughout construction. Any damages found shall have to be corrected at the contractor cost. The Purchaser shall have the right to use the foundations, pads, piers, slabs, floors and all concrete work as needed for other works or equipment erected prior to its "Taking over".

21.14.00 Acceptance Criteria

- a) Standard deviation shall be based on test results; and determination of standard deviation shall conform to clause 14.5 of IS:456. The strength requirements and acceptance criteria shall conform to clause 15 of IS:456. Inspection of concrete work immediately after stripping the formwork and core test of structures shall conform to clause 16 of IS:456.
- b) **Load Test**
 - i) Load tests of structural members as required, when the strength of test specimen results fall below the required strength, as per „Load Test on Parts of Structures”, clause 17.6 of IS:456. If load testing is decided by the Engineer, the member under consideration shall be subjected to a test load equal to 1.25 (one and a quarter) times the specified live load used for design and this load shall be maintained for a period of 24 (twenty four) hours before removal. The detailed procedure of the test shall be decided by the Engineer. Load tests shall not be made until the structure is at least 56 days old.
 - ii) If the member shows evident failure, such changes as necessary to make the structure adequately strong, shall be made by the contractor, if permitted under Statutory Regulations to the discretion of the Engineer, the structure under test or a portion thereof may be retained as such without any modification by derating its load bearing capacity, provided the design criteria allows such derating.
 - iii) A reinforced concrete beam, floor or roof shall be deemed to have passed the test, if the maximum deflection at the end of 24 hours does not exceed the deflection given in clause 17.6 of IS: 456. If a portion of the structure is



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found to be unacceptable, it shall be dismantled and replaced by a new structure as specified. If, in the course of dismantling, any damage is done to the embedded items and or other adjacent structures, the same shall be made good, by the contractor to the satisfaction of the Engineer.

21.15.00 Tolerances

The permissible tolerances are as follows :-

- a) Tolerances for RC buildings
- b) Variation from the plumb
- c) In the lines and surfaces of columns, piers, walls and in arises 5mm per 2.5m but not more than 25mm.
- d) For exposed corner columns and other conspicuous lines:

In any bay upto 5m	=	5mm
In 10m or more	=	10mm
- e) Variation from the level or from the grades indicated on the drawings:
 - i. In slab soffits, ceilings, beam soffits and in arises:

In 2.5m	=	5mm
In any bay upto 5m	=	8mm
In 10m or more	=	15mm
 - ii. For exposed lintels, sills, parapets, horizontal grooves and other conspicuous lines:

In any bay upto 5m	=	5mm
In 10m or more	=	10mm
 - i. Variation of the linear buildings lines from established position in plan and related position in plan and related position of columns, walls and partitions.

In any bay upto 5m	=	10mm
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In 10m or more = 10mm

- ii. Variation in the sizes and locations of sleeves, openings in walls and floors = 5mm
(Except in the case of and for anchor bolts.)

- iii. Variation in cross-sectional dimensions of columns and beams the thickness of slabs and walls:-

Minus = 5mm
Plus = 10mm

- f) Footings

- g) Misplacement or eccentricity

2% of footing width in the direction of misplacement but not more than 50mm

- h) Reduction in thickness

Minus = 5% of specified thickness subject to a maximum of 50mm

- i) Variation in steps

In a flight of stairs

Rise = 3mm

Tread = 5mm

In consecutive steps

Rise = 1.5mm

Tread = 3mm

- j) Tolerances in other concrete structures

- i) Variation of the constructed linear outline from established position in plan.

In 5 m = 10mm
In 10m or more = 15mm



- ii. Variations of dimensions to individual structure features from established positions:

In 20m or more	=	25mm
In buried construction	=	50mm

- iii. Variation from plumb, from specified batter or from curved surfaces of all structures:

In 2.5 m	=	10mm
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In 5m	=	15mm
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In 10m or more	=	25mm
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In buried – construction	=	Twice the above amounts
--------------------------	---	-------------------------

- iv. Variation from level or grade indicated on drawings in slab, beams, soffits, horizontal grooves and visible arises:

In 2.5 m	=	5mm
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In 7.5m	=	10mm
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In buried – construction	=	Twice the above amounts
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- v. Variation in cross-sectional dimensions of columns, beams, buttresses, piers and similar members:

Minus	=	5mm
-------	---	-----

Plus	=	10mm
------	---	------

- vi. Variation in the thickness of slabs, walls, arch section and similar members:

Minus	=	5mm
-------	---	-----

Plus	=	10mm
------	---	------

- vii. Misplacement of eccentricity:
2% of footing width in the direction of misplacement but not more than 50mm.

- viii. Reduction in thickness:

5% of specified thickness subject to a maximum of 50mm.



Tolerance in other types of structures shall generally conform to those given in clause 2.4 or recommended practice for concrete formwork (ACI347).

ix. Tolerance in fixing anchor bolts shall be as follows:

(i) Anchor bolts without sleeves +1.5mm in plan.

(ii) Anchor bolts with sleeves +5.0mm in elevation.

a) For bolts upto and including 25mm dia +mm in all directions.

b) For bolts 32mm dia and above +3mm in all directions.

(iii) Embedded parts +5mm in all directions.

21.16.00 Special Conditions For Construction of TG Foundation

a) Scope

i) The work to be performed under this contract consists of providing all materials except those supplied by the Purchaser, shuttering, staging, inserts, construction equipment, labour and all incidental items not shown or specified but reasonably implied or necessary for the proper completion of the work, all in the strict accordance with the drawings, schedules and specifications and including revisions and amendments thereto and such detailed drawings as may be provided by the Consultant, during the execution of the work.

ii) It is not the intent to specify completely herein all the details of designs and construction of the structure. However, the structure shall conform in all respects to high standards of Engineering, design and workmanship and be capable of performing in continuous commercial operation upto contractors guarantee in the manner acceptable to the Purchaser / Consultant who will interpret the meaning of drawings and specifications and shall have the power to reject any work or materials which in his judgment are not in full accordance therewith.

b) Form Work

i) All forms shall be abundantly wetted on both sides before concrete is poured. The date of removing forms for each individual stage of construction shall be fixed by the Purchaser / Consultant.

ii) The minimum period for striking of formwork shall be as follows:-



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- Vertical sides of beams and pedestals -7 days
- Bottom of beams / slabs -28 days

However, the vertical faces shall be loosened after 24 hours of completion of concreting the supports.

- iii) Concrete surface shall not normally be patched or otherwise treated after the removal of forms. Where the surfaces exposed on stripping is not of a satisfactory nature, owing to the contractors failure to take necessary precaution before, during or after the concrete placing, such surfaces shall be worked and finished in accordance with the instructions of the Purchaser / Consultant at the cost of the contractor. The pores shall be filled in with a neat solution of cement and water applied by brush and when dry the surface shall be rubbed down with carborundum stone. The cost of the above treatment shall be deemed to be included in the unit rate entered by Contractor. The top surface of the T.G. deck shall be float-finished, unless otherwise specified to the required levels. There must be no surface grouting or treating which might draw the “fines” to the top. All shuttered surfaces shall be left as they strip without removing boards or panel markings.

Any serious honey combing will render the concrete work liable to rejection and cutting out and re-concreting wholly or partly as the Engineer in Charge directs. All costs involved in repairing defect shall be borne by the Contractor.

- iv) The arrangement and method for movement of workers during TG construction to various levels of the TG foundations shall be submitted well in the advance to the Engineer- in – Charge for his approval far taking up the work.
- v) In addition to the above paras the contractor shall also satisfy all other requirements for formwork mentioned in elsewhere in the specification.

c) Staging

- i) The entire staging for supporting the formwork, walkways and platforms for placing concreting equipments such as vibrators, etc., shall be of structural steel. The staging shall be designed for the worst combination of loading as specified hereinafter.
- ii) The Contractor shall submit 6 copies of design calculations and drawing to prove adequacy of the staging for approval of the consultant. On receipt of final approval, the contractor shall supply 20 copies of approved drawings for distribution.



- iii) The staging shall be so designed that no load from platforms are passed on to the formwork at the top.
- iv) All platforms, walkways etc. shall be clear of the formwork and at least 200 mm above it. The width of platforms and walkways shall be at least 1.2 metres for easy movement of labour both ways.
- v) The platforms shall be of planks or bamboo mats (clamped with steel strips suitably stiffened to avoid springing).
- vi) The form work and staging shall be designed for a live load of 1000 Kg/m².
- vii) Unit weight of green concrete shall be considered as 2500kg/m³ for design of form work and staging.
- viii) The staging shall be braced in both the directions.
- ix) The staging shall be supported on rigid surfaces at ground level.
- x) The staging shall be sufficiently rigid to prevent any distortion in the form work.

d) Special Precautions

- i) The contractor shall take all precautions to ensure concreting of TG Raft, columns and TG Deck in one pour each. Concreting shall be continuous and no break in concreting shall be permitted.
- ii) The Contractor shall prepare a scheme for concreting giving details of number of mixers, labour, vibrators, pouring schedule and obtain prior approval of the same from the Engineer-in-Charge before starting of concreting.
- iii) The Contractor shall ensure that at no time the temperature of the green concrete exceeds 38 degree C by taking proper precautions. If required, ice shall be added to control the temperature at no additional cost to the Purchaser.
- vi) Approved “Retarders” shall be used by the contractor in the proportions specified by the manufacturer for total concreting work of the TG foundation. The cost of the admixture shall be included in unit rates quoted by the Bidders.



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e) Test for Soundness of Concrete

- i) After completion of the construction of TG foundations and all major machine foundation like BFP, FANS, MILLS etc, the contractor shall get the Deck and column tested for soundness of concrete by “ultrasonic wave – non – destructive test method” from reputed research institutions like “SERC-CHENNAI” “CWPRI – POONA”, at no extra cost to the Purchaser.
- ii) If the test report calls for any rectification in the concrete works by way of pressure grouting or otherwise, the same shall be carried out by the contractor at no additional cost to the Purchaser to the satisfaction of the Purchaser / Consultant
- iii) If require, any additional test to establish the adequacy of the rectification works carried out shall be done by the contractor at no extra cost to the Purchaser

f) List of Codes and Standards

All work under this specification shall conform to the latest revisions of Indian Standard specifications and Codes of Practice:

IS : 216	-	Indian Standard Specification for Coal Tar Pitch
IS : 226	-	Indian Standard Specification for Structural Steel [Standard quality]
IS : 269	-	Indian Standard Specification for Ordinary, and Low Heat Portland Cement
IS : 383	-	Indian Standard Specification for Coarse and Fine Aggregates from Natural Sources for Concrete
IS : 432	-	Indian Standard Specification for Mild Steel and Medium Tensile Steel Bars and Hard Drawn Steel Wire for concrete Reinforcement
IS : 455	-	Indian Standard Specification for Slag Cement and Reinforced Concrete
IS : 456	-	Indian Standard Code of Practice for Plain and Reinforced Concrete
IS : 516	-	Indian Standard Specification for Methods of Test for Strength of Concrete.



IS : 1139	-	Indian Standard Specification for Hot Rolled Mild Steel and Medium Tensile Steel and High Yield Strength Steel Deformed Bars for concrete Reinforcement
IS : 1199	-	Indian Standard Specification for Methods of Sampling and Analysis of Concrete
IS : 1200	-	Indian Standard Specification for Method of measurement Cement Concrete Works. Part-II
IS : 1200	-	Indian Standard Specification for Method of Measurement of Part-V Formwork.
IS : 1332	-	Indian Standard Specification for Bitumen Felts for Waterproofing and Damp-proofing.
IS : 1489	-	Indian Standard Specification for Portland – Pozzolona Cement
IS : 1566	-	Indian Standard Specification for Methods of Sampling and Analysis of Concrete
IS : 1609	-	Code of Practice for Laying Damp proof Treatment using Bitumen felts
IS : 1786	-	Indian Standard Specification for Cold-twisted Steel Bars for Concrete Reinforcement
IS : 1791	-	Indian Standard Specification for Batch Type Concrete Mixers
IS: 2210	-	Indian Standard Specification FOR Design of Reinforced Concrete Shell Structures and Folded Plates.
IS : 2386	-	Indian Standard Specification for Methods of Test for Aggregates for Concrete – Part-I to VIII
IS : 2502	-	Indian Standard Code of Practice for Bending and Fixing of Bars for concrete reinforcement
IS : 2505	-	Indian Standard Specification for Concrete Vibrators, Immersion Type
IS: 2506	-	Indian Standard Specification for Screed Board Concrete Vibrators



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IS : 2514	-	Indian Standard Specification for Concrete Vibrating Tables
IS : 2722	-	Indian Standard Specification for Portable Swing weigh Batchers for Concrete (Single and Double Bucket type)
IS : 2770	-	Indian Standard Specification for Method of Testing Bond in Reinforced Concrete.
IS : 3025	-	Indian Standard Specification for Methods of Sampling and Test (Physical and Chemical) for Water used in Industry.
IS : 3201	-	Indian Standard Specification for Design and for construction of Precast Concrete Trusses.
IS : 3370	-	Indian Standard Specification for Code of Practice Concrete Structures for Storage of Liquids.
IS : 3550	-	Indian Standard Specification for Method of Test for Routine Control for Water used in Industry.
IS : 3558	-	Code of Practice for use of Immersion Vibrators for Consolidating Concrete
IS : 3590	-	Indian Standard Specification for Load Bearing Light Weight Concrete Blocks.
IS : 3696	-	Safety Code for Scaffolding and Ladders
IS : 3812	-	Indian Standard Specification for Fly Ash for use as Admixture for Concrete.
IS : 4031	-	Indian Standard Specification for Method of Tests for Hydraulic Cement.
IS : 4082	-	Indian Standard Specification for Recommendation on Stacking and Storage of Construction Materials at site.
IS : 4634	-	Indian Standard Specification for Method of Testing Performance of Batch-type Concrete Mixes.
IS : 4656	-	Indian Standard Specification for Form Vibrators for Concrete.



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IS : 4925	-	Indian Standard Specification for Concrete Batching and Mixing Plant.
IS : 4926	-	Indian Standard Specification for Ready Mixed Concrete.
IS : 4990	-	Indian Standard Specification for Plywood for Concrete Shuttering work.
IS : 4995	-	Indian Standard Specification for Design of Part-I&II Reinforced Concrete Bins for the Storage of Granular and Powdery Materials.
IS : 5512	-	Indian Standard Specification for Flow Table for use in Tests of Hydraulic Cement and Pozzolanic Materials.
IS : 5513	-	Indian Standard Specification for Vicat Apparatus.
IS : 5515	-	Indian Standard Specification for Compaction Factor Apparatus.
IS : 5751	-	Indian Standard Specification for Precast Concrete Coping Blocks.
IS : 5816	-	Indian Standard Specification for Method of Test for Splitting Tensile Strength of Concrete Cylinders.
IS : 5891	-	Indian Standard Specification for Hand Operated Concrete Mixers.
IS : 6452	-	Indian Standard Specification for High Alumina Cement for Structural Use.
IS : 6909	-	Indian Standard Specification for Supersulphated Cement.
IS : 6923	-	Indian Standard Specification for Method of Test for Performance of Screed Board Concrete Vibrators.
IS : 6925	-	Indian Standard Specification for Method of Test for Determination of Water Soluble Chloride in Concrete Admixtures.
IS : 7242	-	Indian Standard Specification for Concrete Spreaders.



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IS : 7246	-	Indian Standard Specification for Table Vibrators for Consolidating Concrete.
IS : 7251	-	Indian Standard Specification for Concrete Finishers.
IS : 7320	-	Indian Standard Specification for Concrete Slump Test Apparatus.
IS : 7861 Part-I&II	-	Indian Standard Specification for Recommended Practice for Extreme Weather Concreting
IS : 7969	-	Safety Code for Storage and Handling of Building Materials.
IS : 8041E	-	Indian Standard Specification for Rapid Hardening Portland Cement.
IS : 8112	-	Indian Standard Specification for high strength Ordinary Portland Cement.
IS : 8142	-	Indian Standard Specification for Determining Setting time of Concrete by Penetration Resistance.
IS : 8989	-	Safety Code for erection of Concrete Framed Structures.
IS : 9013	-	Indian Standard Specification for Method of Making, Curing and Determining Compressive Strength of Accelerated-cured Concrete Test Specimens.
IS : 9077	-	Code of Practice for Corrosion Protection of Steel Rails in RB and RCC Construction.
IS : 9103	-	Indian Standard Specification for Admixtures for Concrete.



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22.00.00 CONSTRUCTION OF SHELL BY SLIP-FORM METHOD

22.01.00 Shell construction has to be done by slip form shuttering technique. Type of slip form proposed should be indicated in the offer with sketches, drawings and construction statement as explained hereinafter. Number, type and capacities of jacks, the control system and achievable rate of progress in mm/hour should also be indicated. The chosen scheme shall be of a past proven design. A certified performance record of the scheme should be submitted with the offer to guarantee workability of the scheme both from execution time and safety point of view.

22.02.00 The Contractor should furnish a brief but comprehensive statement indicating the planning & programme and method of work to be followed, for the approval of Purchaser at the time of submitting tender. This statement shall include the following items:

- i) Type and description of slip-form equipment and its accessories
- ii) Design of scaffolding and staging
- iii) Description of materials including admixtures to be used for construction.
- iv) Manpower planning, construction spaces required, standby arrangement.
- v) Rate of slip-forming
- vi) Proposed workability requirement of concrete and type of cement and admixture to be used
- vii) Quality assurance programme
- viii) Method of transportation of material
- ix) Method of curing and rectification of defects
- x) Planned interruption, if proposed, and activities during planned interruption. Treatment of construction joint.
- xi) Contingency solution for unplanned interruption
- xii) Time of completion

22.03.00 While selecting the Contractor, due consideration will be given to the merit of the above mentioned statement proposed by the Contractor and minimum time of completion, apart from his past experience in such types of work as also technical and



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financial resources of the Contractor.

22.04.00

Notwithstanding what have been specified in earlier clauses, following guide lines are being presented which should be kept in view by intending Contractor, while quoting for slip form method of construction.

- a) Care to be taken to prevent dragging of concrete along with upward movement of the shuttering. For this purpose following steps are advisable.
 - i) Shutter plates have to be smooth and should be thoroughly Before fixing them in position all the surfaces which will be in contact with concrete to have a coat of epoxy paint.
 - ii) In areas where concrete thickness is 750 mm or more rate of should be such that minimum slipping of shuttering is 100 mm hour.
 - iii) Mix design should be so done that it will be self-lubricant at contact face of shutter and concrete and thus reduce friction cement of approved manufacturer (conforming to relevant specification) may be used for the purpose. An optimum ratio coarse / fine aggregate should be established to suit the depending on availability of aggregates.
 - iv) Mix design also should be so done that it has a slump of 50mm at the point where concrete is placed under an ambient temperature of around 40 Deg.C. This will also keep vibration by needle vibrators to required minimum. Slump should not drop down to zero in less than 45 min. Suitable retarding agent and plasticizer of approved manufacture may be added in mix to achieve this purpose. These admixtures to be properly identified by preliminary tests both for Performance and for compatibility with particular type of cement Proposed to be used.

Additional steps like spraying of water over the shutters and keeping down the temperature of coarse aggregates by continuous spraying of water over those may be resorted to if ambient temperature is more than 40°C.

- b) Care must be taken to prevent twist, which predominantly occur in the initial stages because of low slipping rate, in the horizontal plane of slip-form assembly. A thorough check on this aspect must be kept at every 15 minutes interval. One person should exclusively be assigned to this work together with rectifying any defect.
- c) Every endeavor has to be made so as not to occur any tilt in the shutter assembly. To achieve this following steps need be taken:
 - i) Performance of jacks has to be closely observed and any defective one needs immediate replacement. Difference in levels of opposite aggregates offer better performance in slip form technique. These help to keep down water



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/ cement ratio and also offers better lubrication between concrete and shutter surface. 40 mm down size of coarse aggregates should be preferably be used unless reinforcement detailing calls for lesser size aggregates.

- d) From the creep point of view, shrinkage as well as initial setting property of concrete, cement content should not preferably be more than 400 kg per cum of concrete.
- e) Minimum compressive strength (after 4 to 6 hours of mixing) of concrete immediately below the shutter as slip form proceeds should be between 0.1 to 0.2 Newton/sq.mm.
- f) Large diameter vibrator needles should not be used for vibrating concrete. Sizes of these needles should preferably be restricted to 25 mm diameter and to 40 mm diameter – only in exceptional cases. Atleast two nos. standby vibrator units should always be maintained on top of working deck at all times during the entire period of slip form operation.
- g) It is preferable to have membrane curing compounds sprayed on fresh surfaces emerging out of shutter panels for ensuring proper curing at greater heights.

In case such spraying is not envisaged then elaborate arrangement has to be made for adequate supply of water both on inside and outside vertical surfaces with spraying arrangement, necessary length of pipelines and pump of adequate head to serve the purpose. It is always advisable to have a standby pump for effective utilization of the system.

- h) If slip forming is carried out in summer, rate of slipping should be around 400 mm per hour. If lesser value is contemplated appropriate retarders should be specified.
- i) Exact number and capacity of jacks as well as spacing of yoke frames are to be determined taking into account various loadings including self weight of the system, dead and live loads on working and other platforms, horizontal load on formwork, wind load etc.

It is desirable that jacking system, based on which the entire slip form system works, should consist of jacks 3 tonne / 6 tonne capacity and a hydraulic pump with necessary pipe connections.

Spacing of yoke legs should preferably be kept within 2 meters to prevent overloading on jacks and consequent failure resulting in twist of the formwork.

Jacking rods should be of 25 mm diameter for 3 tonne jacks and 32 mm



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diameter for 6 tonne jacks.

- j) Atleast 30% spare jacks and jacking rods should be kept ready during the entire operation. It is obligatory to maintain spare hydraulic pump along with a set of loose pipes in perfect working condition on top of working deck.
- k) In sections where thickness is 500 mm or more it is prudent to go in for two nos. of jacks for each slip form yoke.
- l) For effective utility of this technique following areas need careful attention at the very conceptual stage:
 - i Detailed quality assurance programme
 - ii Advance planning and preparations
 - iii Arrangement for on site supervision and adequate access facilities.
- m) Construction methods including description and types of different equipment proposed to be used, structural arrangement and analysis of the system, description and type of different materials, planned interruptions, descriptions and frequency of various checks and tests for slip form technique as well as for material, method of preparing, transporting and pouring of concrete, solution for probable defects during slipping, sequence of operations during planned interruptions etc. should be prepared before hand by executing agency and to be approved by Engineer before starting the actual work.
- n) Placing and binding of reinforcement is also a very critical item and needs special attention. From practical considerations not more than two or three layers of horizontal steel can be tied at a time and this causes a definite limitation in placement of reinforcement.

Vertical reinforcements should be kept vertical by providing suitable holders within the slip form system.

- o) It is desirable to have a break of atleast one day for every two weeks of continuous operation. Such break should be utilized for various maintenance activities, removal of jacks rods etc.
- p) Numbers and locations of hoists for lifting concrete, reinforcement and other materials have to be planned well in advance. Capacity of hoists should be such as to match with hourly requirement of concrete and reinforcement. It felt necessary one hoist may be exclusively earmarked for transporting concrete.

For movement of personnel supervising the work a separate hoist must be arranged for.



- q) The system being operative round the clock it is obligatory to have adequate lighting arrangement both on various platform levels as well as on ground below. Arrangement has to be made for facilitating continuous upward movement of the entire system along with slip form.
- r) Winches for lifting men and material and mixers, if located within unsafe area around chimney, should be protected by adequate shelter from possible damage.
- s) Proper telecommunication system has to be established between the personnel working on top of chimney and control room below.
- t) A small laboratory should be maintained at site for testing different materials like cement, coarse and fine aggregates. A cube testing machine may also be installed at site for getting quick feed back results.

Apart from using plumb bobs, level and theodolite instruments for survey purpose arrangement should also be kept for lasers.

- u) In case of interruption in the course of slipping of formwork following measures should be taken:
 - i Provision of a key and additional reinforcement at the junction of new and old concrete.
 - ii Slip form system should be brought up freely to have a minimum overlap of 100 mm or so over previously cast concrete.
 - iii Washing of old concrete surface with compressed air and water jet and thereafter pouring a layer of neat cement grout.
 - iv Clearing of shuttering panels of loose materials, concrete etc. by compressed air and applying a coat of epoxy paint, if felt necessary by Engineer.
 - v Neatly preferable the interface of old or new concrete as soon as it comes out of shutter panel.
- v) It is preferable to suspend the construction work under high wind condition.
- w) It is of utmost importance that for effective implementation of this system an Engineer fully conversant with slip form technique with enough experience in planning and control of formwork should be in overall command of the site and he should be ably supported by well trained mid level supervisory staff, skilled workers and operators.



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- x) Operation of slip form method of construction is a continuous one and it demands continuous / intermediate inspection of accuracies in line, level, dimensions and position and immediate rectification of any noticed deviation. All these ask for personnel of high quality having constant vigilance over the construction activity.
- y) While all the activities in effective implementation of the work needs utmost care keeping safety of men and material in mind it is obligatory that all activities should be carried out under the guidance of a qualified and trained safety engineer.

Safety measures as listed below must be adhered to but should not be limited to only these:

- i. Safety helmets and belts to be provided to all supervising staff and workers.
- ii. Safety nets to be provided below both inside and outside platforms as instructed by Engineer.
- iii. Hand railing and toe guard to be provided around all openings and platforms.
- iv. Regular maintenance of equipment, checking of hoists, scaffolding etc.
- v. Passenger hoist must have multiple ropes with adequate factor of safety.
- vi. Emergency lights, coloured lamps to be provided in accordance with relevant Indian Standards and as supplemented in the specification and to be operative in case of sudden power failure emergency standby generator must be kept ready during the entire period of slip form method or construction.
- vii) Emergency vehicles, first aid facilities must be kept ready during the entire period of work.

22.05.00 Construction tolerances

The following shall be the limit of construction tolerances to be strictly adhered to by the contractor:

- Wall thickness : (+) 10mm
- Shell diameter : (+) 10mm for every 3m diameter without any abrupt changes but in no case more than (+) 40mm.
- Verticality : 1 in 1000 subject to a maximum of 75 mm.

In addition, no two points 10m apart vertically shall be more than 20mm out



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of plumb w.r.t. each other

22.06.00

Although deviations in general will not be encouraged, the Contractor, however may mention in his offer, the additions to or deviations from drawings/ technical conditions/schedule of items issued with the tender papers and any other special requirement implied with the adoption of the slip form method, which may include but need not be limited to the following items as applicable.

- i. Particular requirement of type and brand of cement, if any.
- ii. Special admixture to be added to concrete
- iii. Any change required in the geometry of the chimney including the shell thickness or side slope from that shown in the NIT drawing.
- iv. Any change/special requirement in the arrangement of reinforcement.
- v. Implications if any of necessary in-situ bending of rebars for /brackets etc. and straightening/cleaning of the same prior to casting of brackets.
- vi. Any additional constructional opening in the shell required at ground level for concreting.

All deviations from specification must be justified and tender price shall include all such variation / deviation. Such deviation without assigning any reason will be rejected.



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23.00.00 FABRICATION OF STRUCTURAL STEEL WORK

23.01.00 The details of fabrication, shop testing, painting and delivery to site of structural steel work including supply of all consumable stores, bolts, nuts, washers, electrodes and other materials as required including field connections are indicated below to be performed by the contractor:

- a) Preparation & submission of complete detailed fabrication drawings and erection marking drawings as required including design calculations.
- b) Furnish all materials, labour, tools & plant and all consumables required for fabrication and supply of all necessary bolts, nuts, washers, tie rods and welding electrodes for field connections.
- c) Furnish shop painting of all fabricated steelwork as specified.
- d) Suitably mark, bundle and pack for transport all fabricated materials.
- e) Prepare and furnish detailed bill of materials, dispatch lists (including bought out items) as required for fabrication of structural steelwork.
- f) Load and transport all fabricated steelwork to site with field connection materials.
- g) Maintain a fully equipped fabrication shop at site for modification and repairs as required.

No work under this specification will be provided by any agency other than the contractor, unless specifically mentioned otherwise elsewhere in the contract.

23.02.00 Codes and standards

The work should conform to the requirements of the following latest relevant Indian standard specifications and codes of practice:

- IS : 800 - Code of practice for general construction in steel.
- IS : 80 - Code of practice for use of cold formed light gauge steel structural members in general building construction.
- IS : 806 - Code of practice for use of steel tubes in general building construction.
- IS : 808 - Dimensions for rolled steel beams, channels and angle sections.



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IS : 813	-	Scheme of symbols for welding.
IS : 814	-	Covered electrodes for metal arc welding of carbon and carbon manganese steel.
IS : 815	-	Classification coding of covered electrodes for metal arc welding of mild steel and low alloy high tensile steel.
IS : 816	-	Code of practice for use of metal arc welding for general construction in mild steel.
IS: 228	-	Method of chemical analysis of pig Iron, cast Iron & plain carbon and low alloy steel
IS: 817	-	Code of practice for use of Structural steel in General building construction.
IS: 818	-	Code of practice for use of metal arc welding for general construction in mild steel.
IS: 819	-	Code of practice of resistance spot welding for light assemblies in mild steel.
IS:823	-	Code of practice for manual metal arc welding for mild steel.
IS: 919	-	Recommendations for limits and fits for engineering.
IS 1161	-	Specifications for steel tubes for structural purposes.
IS1181	-	Qualifying test for metal arc welders
IS1599	-	Method of bend tests for steel products other than sheet strip wire & tube.
IS 1731	-	Dimension for steel flats for structural & general engineering purposes.
IS 7205	-	Safety code for erection steel work.
IS2595	-	Code of practice for radiographic testing.
IS : 822	-	Code of practice for inspection of welds.
IS : 1182	-	Recommended practice for Radiographic Examination of fusion welded butt joints in steel plates.
IS : 1200 (Part - 8)	-	Method of measurement of steel work and iron work.



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IS : 1363 (Part - 1 to 3)	-	Hexagon head bolts, screws & nuts of product grade C.
IS : 1364 (Part - 1 to 5)	-	Hexagon head bolts, screws and nuts of product grade A&B.
IS : 1367 (Part - 1 to 18)	-	Technical supply conditions for threaded steel fasteners.
IS : 1608	-	Method for tensile testing of steel products.
IS : 1730	-	Dimensions for steel plate, sheet and strip for structural and general engineering purposes.
IS : 1852	-	Rolling and cutting tolerances for hot-rolled steel product.
IS : 1977	-	Structural steel (Ordinary quality)
IS : 2016	-	Plain washer
IS : 2062	-	Steel for general structural purposes.
IS : 3644 and	-	Code of practice for ultrasonic pulse echo testing by contact immersion method.
IS : 3757	-	High Strength Structural Bolt
IS : 4000	-	High strength bolts in steel structure
IS : 5369	-	General requirements for plain washers and lock washer.
IS : 6005	-	Code of practice for phosphating of iron and steel.
IS : 6649	-	Specification for hardened and tempered washers for high strength structural bolts and nuts.
IS : 6623	-	Specification for high strength structural nuts.
IS : 7215	-	Tolerances for fabrication of steel structures.
IS : 7280	-	Bare wire electrode for submerged arc welding
IS : 8500	-	Structural steel micro alloyed (medium & high strength quality).



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| IS : 8629 | - | Code of practice for protection of iron steel & structures (Part - I to III) from atmospheric corrosion. |
| IS : 9595 | - | Recommendation for metal arc welding of carbon manganese steels. |
| IS : 117 | - | Specification for ready mixed paint, brushing, finishing, exterior, semi-gloss, for general purposes. |
| IS : 128 | - | Specification for ready mixed paint, brushing, finishing, semi-gloss for general purposes, black. |
| IS : 1477 | - | Code of practice for painting of ferrous metal in building (Part - I & II). |
| IS : 2074 | - | Ready mixed paint, air-drying red-oxide zinc chrome priming. |
| IS : 2339 | - | Specification for aluminum paints for general purposes in dual container. |
| IS : 2932 | - | Specification for enamel, synthetic exterior type - I. |
| IS : 2933 | - | Specification for enamel, synthetic exterior type - II. |
| BS 4465 | - | Specification for water cooling towers. |
| ACI | - | Reinforced concrete cooling towers shall – practice & Commendatory |

23.03.00 Conformity with designs

The contractor shall design all connections, supply and fabricate all steelwork and furnish all connection materials in accordance with the approved drawings. The method of painting, marking, packing and delivery of all fabricated materials shall be as approved by the Engineer.

23.04.00 Materials to be used

Standard structural steel sections shall be used instead of fabricated steel sections as far as possible.

- a) All steel materials required for the work shall be supplied by the contractor.
All steel materials shall comply with the following IS:-
- i) IS:801 - Cold formed light gauge steel structural member.



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- ii) IS:2062 - Grade – A, Structural Steel for plate thickness upto 20mm
- iii) IS:2062 - Grade – B (Killed), Structural Steel for plate thickness above 20mm
- iv) IS:2062 - Grade – C, for crane gantry girder in turbine hall.
- v) IS:806 - Steel tubes in general building construction.

b) Electrodes

The arc welding electrodes shall conform to the relevant IS; and shall be of heavily coated type having uniform thickness. With each container of electrodes, the manufacturer shall furnish instructions giving recommended voltage and amperage (polarity in case of D.C. supply) for which the electrodes are suitable. All electrodes shall comply with the following IS:

- i) IS: 814 - Covered electrodes for metal arc welding structural steel.
- ii) IS:815 - Classification and coding of covered electrodes for metal arc welding of mild steel and low alloy high tensile steel.
- iii) IS:7280 - Base wire electrode for submerged arc welding.

c) Bolts and nuts

All bolts and nuts shall conform to the requirements of IS:1367 - Technical Supply Conditions for Threaded Fasteners. Materials for bolts and nuts shall comply with the following IS codes. Mild steel for bolts and nuts tested to following IS shall have a tensile strength of not less than 44 Kg/mm²; and minimum elongation of 23 per cent on a gauge length of 5.6 ÖA, where 'A' is the cross sectional area of the test specimen :

- i) IS:1367 - Technical supply conditions for threaded fasteners.
- ii) IS:1608 - Method for tensile testing of steel other than sheet, strip, wire and tube.
- iii) High tensile steel material shall have the mechanical properties as per IS:1367 or as approved by the Engineer.



d Washers

Washers shall be made of steel conforming to the following IS:

- i) IS:1977 - Structural steel (Ordinary Quality) St-39-0
- ii) IS:2062 - Steel for general structural purpose
- iii) IS:6623 - High Strength Structural Nuts
- iv) IS:6649 - Hardened and tempered washers for high strength structural bolts & nuts.

Paints for shop coat of fabricated steel shall be of epoxy based paint to withstand severe corrosive conditions prevailing at site.

23.05.00 Painting

- a) All steel structures shall receive two primer coats and two finish coats of painting. First coat of primer shall be given in shop after fabrication before dispatch to erection site after surface preparation as described below. The second coat of primer shall be applied after erection and final alignment of the erected structures. Two finish coats shall also be applied after erection.
- b) Steel surface which is to painted shall be cleaned of dust and grease and the heavier layers of rust shall be removed by chipping prior to actual surface preparation. The surface shall be abrasive blasted to Sa-2½ finish as per SIS05-5900. Primer paint shall be zinc silicate of approved brand . Dry film thickness of each primer coat shall be 50 microns.
- c) Finish paint shall be 2 coats of High built epoxy finish of approved brand. Dry film thickness of each finish coat shall be 90 microns. The undercoat and finish coat shall be of different tint to distinguish the same from finish paint. The total dry film thickness shall be 300 microns . All paints shall be of approved brand and shade as per the OWNER's requirement.
- d) Joints to be site welded shall have no paint applied within 100 mm of welding zone.

Similarly where Friction grip fasteners are to be used no painting shall be provided. On completion of the joint the surfaces shall receive the paint as specified.
- e) Surfaces inaccessible after assembly shall receive two coats of primer prior to assembly.



Surfaces inaccessible after erection including top surfaces of floor beams supporting gratings or chequered plate shall receive one additional coat of finish paint over and above number of coats specified before erection. Portion of steel member embedded / to be encased in concrete shall not be painted.

23.06.00 Storage of Materials

- a) All materials shall be stored to prevent deterioration ensuring the preservation of their quality and fitness for the work. Any material which has deteriorated or has been damaged shall be removed from the contractor's yard immediately. The contractor shall maintain upto date account in respect of receipt, use and balance of all sizes and sections of steel and other materials. In case the fabrication is carried out in contractor's fabrication shop outside the plant site where other fabrication works are also carried out, all materials shall be stacked separately with easily identifiable marks.
- b) The steel used for fabrication shall be stored in separate stacks off the ground section-wise and lengthwise so that they can be easily inspected, measured and accounted for at any time. If required by the Engineer, the materials should be stored under cover; and suitably painted for protection against weather.
- c) The electrodes for electric arc welding shall be stored in properly designed racks, separating different types of electrodes in distinctly marked compartments. The electrodes shall be kept in a dry and warm condition [if necessary by resorting to heating].
- d) Bolts, nuts, washers and other fastening materials shall be stored on racks off the ground with a coating of suitable protective oil. These shall be stored in separate gunny bags or compartments according to diameter, length and quality.
- e) Paints shall be stored under cover in airtight containers. Paints supplied in sealed containers shall be used up as soon as possible once the container is opened.

23.07.00 Quality Control

23.07.01 The contractor shall establish and maintain quality control procedures for different items of work and materials to ensure that all works are performed as per specification. As far as possible, all inspections by the Engineer shall be made at the contractor's fabrication shop. The contractor shall co-operate with the Engineer in permitting access for inspection to all places where work is being done and in providing free of cost all necessary help in respect of tools & plant, instrument, labour and materials required to carry out the inspection. The inspection shall be so scheduled as to provide the minimum interruption to the work of the contractor.



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Materials or workmanship not in reasonable conformance with the provisions of this specification would be rejected at any time during the progress of the work. The quality control procedure shall cover but not be limited to the following items of work:

- i) Steel : Quality, manufacturer's test certificates, test reports of representative samples of materials from unidentified stocks if permitted to be used.
- ii) Bolts, Nuts : Manufacturer's certificate, dimension & washers checks, material testing.
- iii) Electrodes : Manufacturer's certificate, thickness and quality of flux coating.
- iv) Welders : Qualifying tests
- v) Welding sets : Performance tests
- vi) Welds : Inspection, X-ray, Ultrasonic tests
- vii) Paints : Manufacturer's certificate, physical Inspection Reports.

23.07.02

- a) The dimensions, forms, weights and tolerances of all rolled shapes, bolts, nuts, studs, washers etc. and other members used in the fabrication shall, wherever applicable, conform to the requirements of the latest relevant IS.

- b) **Fabrication Drawing**

The sequence of submission of fabrication drawings for approval shall match with the approved fabrication and erection schedule. It should be ensured that the correctness of general arrangement for centerline dimensions and levels, section sizes, and adequacy of connections including splice joints as to the number of bolts, weld length, size of gusset/end plates are maintained. The approval of the drawing however shall not relieve the contractor of his sole responsibility in carrying out the work correctly and fulfilling the complete requirements of spec.

The fabrication drawings shall include but not be limited to the following:

- i) Assembly drawings giving exact sizes of the sections to be used and identification marks of the various sections.
- ii) Dimensional drawings of base plates, foundation bolt location etc.
- iii) Details of all connections with supporting calculations.



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- iv) Any other drawings or calculations that may be required for the clarification of the works.

The fabrication drawings shall give all the necessary information for the fabrication, erection and painting of the steelwork in accordance with the provisions of this specification. Fabrication drawings shall be made in accordance with the best modern practice and with due regard to sequence, speed and economy in fabrication and erection. Fabrication drawings shall give complete information necessary for fabrication of various components of the steelwork, including the location, type, size and extent of welds. These shall also clearly distinguish between fabrication and field bolts and welds and specify the class of bolts and nuts. The drawings shall be drawn to a scale large enough to convey all the necessary information adequately. Notes on the fabrication drawings shall indicate those joints or groups of joints in which it is particularly important that the welding sequence; and technique of welding shall be carefully controlled to minimize the locked-up stresses and distortion. Welding symbols used shall be in accordance with the requirements of IS:813; and shall be consistent throughout. Weld lengths called for on the drawings shall mean the net effective length.

All steel structural wall beam/columns shall be encased with nominal reinforcement and chicken wire mesh fouling / connecting in brick masonry works. Also, chicken wire mesh shall be provided at the junction of RCC and brick work. The wall beam/tie beam shall be provided at every 2.5 meter height of the brick wall.

All columns shall be encased with RCC upto the height of 500 mm above zero level of STG power house building.

23.08.00 Workmanship

- a) All workmanship shall be equal to the best practice in modern structural shops, and shall conform to the provisions of IS:800 and other relevant Indian standards or equivalent.
- b) Rolled materials before being laid off or worked, must be clean, free from sharp kinks, bends or twists and straight within the tolerances allowed by IS:1852. If straightening is necessary, it shall be done by mechanical means or by the application of a limited amount of localized heat. The temperature of heated areas, as measured by approved methods, shall not exceed 600 Deg. C.
- c) Cutting shall be effected by shearing, cropping or sawing. Use of a mechanically controlled gas cutting torch is permitted for mild steel only. Gas cutting of high tensile steel is permitted provided special care is taken to leave sufficient metal to be removed by machining, so that all metal that has been hardened by flame is removed. Gas cutting without a mechanically



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controlled torch shall be permitted if special care is taken and done under expert hand.

To determine the effective size of members cut by gas, 3 mm shall be deducted from each cut edge. Gas cut edges, subjected to substantial stress or which have weld metal deposited on them, shall be reasonably free from gouges. Occasional notches or gouges not more than 4 mm deep will be permitted. Gouges greater than 4 mm, that remain from cutting, shall be removed by grinding. All re-entrant corners shall be shaped notch-free to a radius of at least 12 mm. Shearing, cropping and gas cutting shall be clean, reasonably square and free from any distortion.

- d) Finishing of sheared or cropped edges of plates or shapes of edges gas-cut with mechanically controlled torch shall not be required, unless specifically required by design and called for on the drawings, included in a stipulation for edge preparation for welding or as may be required after the inspection of the cut surface. Surface cut with hand-flame shall generally be ground, unless specifically instructed.
- e) The erection clearance for cleated ends of members connecting steel to steel shall preferably be not greater than 2 mm at each end. The erection clearance at ends of beams without web cleats shall be not more than 3 mm at each end, but where, for practical reasons, greater clearance is necessary, suitably designed cleatings shall be provided.
- f) **Bolted construction:**
 - i) Holes through more than one thickness of material for members, such as compound stanchions and girder flanges, shall be drilled after the members are assembled and tightly clamped or bolted together. Punching shall be permitted before assembly, if the thickness of the material is not greater than the nominal diameter of bolt plus 3 mm subject to a maximum thickness of 16 mm provided that the holes are punched 3 mm less in diameter than the required size; and reamed after assembly to the full diameter.

Holes for black bolts shall be not more than 1.5 mm or 2 mm (depending on whether the diameter of the bolt is less or more than or equal to 25 mm) larger in diameter than the nominal diameter of the black bolt passing through them.

Holes for turned and fitted bolts shall be drilled to a diameter equal to the nominal diameter of the shank or barrel subject to a tolerance grade of H8 to IS:919. Parts to be connected shall be firmly held together by tacking welds or clamps and the holes drilled through all thicknesses in one operation and subsequently reamed to size. Holes not drilled through all thicknesses in one operation shall be drilled to a smaller size and reamed out after assembly. Holes for bolts shall not be formed by gas cutting process.



- ii) Drifting to enlarge unmatching holes shall not generally be permitted. In case drifting is permitted to a slight extent during assembly, it shall not distort the metal or enlarge the holes. Holes to be enlarged to admit the bolts shall be reamed. Poor matching of holes shall be cause for rejection. The component parts shall be so assembled that they are neither twisted nor otherwise damaged, and shall be so prepared that the specified cambers, if any, are maintained.

Bolted construction shall be permitted only in case of field connections if called for on the drawings and is subjected to the limitation of particular connection as may be specified.

Washers shall be tapered or otherwise suitably shaped, where necessary, to give the heads and nuts of bolts a satisfactory bearing. The threaded portion of each bolt shall project out through the nut at least one thread. In all cases, the bolt shall be provided with a washer of sufficient thickness under the nut to avoid any threaded portion of the bolt being within the thickness of the parts bolted together. In addition to the normal washer, one spring washer or lock-nut shall be provided for each bolt for connections subjected to vibrating forces or otherwise as indicated on the drawings.

g) Welded Construction

- i) Welding shall be in accordance with relevant IS. Welding shall be done by experienced and good welders qualified by tests in accordance with IS:817. Surfaces to be welded shall be free from loose scale, slag, rust, grease, paint and any other foreign material except that mill scale which withstands vigorous wire brushing may remain. Joint surfaces shall be free from fins and tears. Preparation of edges by gas-cutting shall, wherever practicable, be done by a mechanically guided torch.
- ii) Parts to be fillet welded shall be brought in as close contact as practicable and in no event shall be separated by more than 4 mm. If the separation is 1.5 mm or greater, the size of the fillet welds shall be increased by the amount of the separation. The fit of joints at contact surfaces which are not completely sealed by welds, shall be close enough to exclude water after painting. Abutting parts to be butt-welded shall be carefully aligned. Misalignments greater than 3 mm shall be corrected; and in making the correction, the parts shall not be drawn into a sharper slope than two degrees (2 Deg.). The work shall be positioned for flat welding whenever practicable.
- iii) In assembling and joining parts of a structure or of built-up members, the procedure and sequence of welding shall be such as will avoid needless distortion and minimize shrinkage stresses. Where it is impossible to avoid high residual stresses in the closing welds of a rigid assembly, such closing welds shall be made in compression elements.

In the fabrication of cover-plated beams and built-up members, all shop splices in each component part shall be made before such component part is



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welded to other parts of the member. Long girders or girder sections shall be made by shop splicing not more than 3 sub-sections, each made in accordance with this paragraph. Welded assemblies shall be stress relieved by heat treating in accordance with the provisions of the relevant IS.

- iv) All complete penetration groove welds made by manual welding, except when produced with the aid of backing material not more than 8 mm thick with root opening not less than one-half the thickness of the thinner part joined, shall have the root of the initial layer gouged out on the back side before welding is started from that side, and shall be so welded as to secure sound metal and complete fusion throughout the entire cross-section. Groove welds made with the use of the backing of the same material as the base metal shall have the weld metal thoroughly fused with the backing material. Backing strips need not be removed. If required, they may be removed by gouging or gas cutting after welding is completed, provided no injury is done to the base metal and weld metal and the weld metal surface is left flush or slightly convex with full throat thickness.

Groove welds shall be terminated at the ends of joint in a manner ensuring soundness. Where possible, this should be done by use of extension bars or run-off plates which need not be removed upon weld completion. To get the best and consistent quality of welding, automatic submerged arc process shall be preferred. The technique of welding employed, the appearance and quality of welds made, and the methods of correcting defective work shall conform to the welds made, and the methods of correcting defective work shall conform to the relevant IS.

- v) If welding is to be undertaken at low temperature, adequate precautions as recommended in relevant IS shall be taken. When the parent material is more than 40 mm thick, the temperature of the area mentioned above shall be in no case be less than 20°C, all requirements regarding preheating of the parent material shall be in accordance with the relevant IS.
- vi) Where required, intermediate layers of multiple-layer welds shall be peened with light blows from a power hammer, using a round-nose tool. Peening shall be done after the weld is cooled to a temperature warm to the hand. Care shall be exercised to prevent scaling or flaking of weld & base metal from over peening.
- vii) The equipment shall be capable of producing proper current so that the operator may produce satisfactory welds. The welding machine shall be of type and capacity as recommended by the electrode manufacturer.
- viii) Column splices and butt joints of compression members for stress transmission shall be accurately machined and close-butted over the whole section with a clearance not exceeding 0.2 mm locally at any place. In column caps and bases, the ends of shafts together with the attached gussets, angles, channels etc., after welding together, should be accurately machined



so that the parts connected butt over the entire surfaces of contact. Care should be taken that those connecting angles or channels are fixed with such accuracy that they are not reduced in thickness by machining by more than 2 mm.

- ix) Bases and caps fabricated out of steel plates, except when cut from material with true surface, shall be accurately machined over the bearing surface and shall be in effective contact with the end of the stanchion. A bearing face which is to be grouted direct to a foundation need not be machined if such face is true and parallel to the upper face. To facilitate grouting, holes shall be provided, where necessary, in stanchion bases for the escape of air. The ends of lacing bars shall be neat and free from burrs. Rolled section or built-up steel separators or diaphragms shall be required for all double beams except where encased in concrete, in which case, pipe separators shall be used. Provision shall be made for all necessary steel bearing plates to take up reaction of beams & columns and the required stiffeners & gussets whether or not specified. Bearing plates and stiffener connections shall not be permitted to encroach on the designed architectural clearances.
- x) All shop connections shall be welded as specified. Certain shop connections, may be changed to field connections if desired by the Engineer for convenience of erection; and the contractor shall make the desired changes. The steelwork shall be temporarily shop-erected complete so that accuracy of fit may be checked before dispatch. The parts shall be shop-erected with a sufficient number of parallel drifts to bring and keep the parts in place. In case of parts drilled or punched using steel jigs to make all similar parts interchangeable, the steelwork shall be shop erected facilitating the check of interchangeability.

23.09.00 Shop Painting

- a) The steelwork concealed by interior building finish need not be painted; steelwork to be encased in concrete shall not be painted. All other steelwork shall be given one coat of shop paint, applied thoroughly and evenly to dry surfaces which have been cleaned as below, by brush, spray, roller coating, flow coating or dipping. Before leaving the shop, all steelwork to be painted shall be cleaned by hand- wire brushing or by other mechanical cleaning methods to remove loose mill scale, loose rust, weld slag or flux deposit, dirt and other foreign matter. Oil and grease deposits shall be removed by solvent. Steelwork having no shop paint shall, after fabrication, be cleaned of oil or grease by solvent cleaners; and shall be cleaned of dirt and other foreign material by through sweeping with a fiber brush. After completion of the pre- cleaning, the metal surface shall be immediately painted with epoxy based paint.
- b) Inaccessible surfaces after assembly, shall receive two coats of shop paint, positively of different colours to prove application of two coats before assembly. This does not apply to the interior of sealed hollow sections. Contact surfaces shall be cleaned as per para (a) above before assembly.



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Machine finished surfaces shall be protected against corrosion by a rust inhibitive coating that can be easily removed prior to erection or which has characteristics that make removal unnecessary prior to erection. Surfaces within 50 mm of any field weld location shall be free of materials that would prevent proper welding or produce objectionable fumes while welding is being done.

- c) All the grills shall be galvanized

23.10.00 Testing, Acceptance Criteria and Delivery

- a) The contractor shall carry out testing as per IS. The contractor shall get the specimen tested in a laboratory approved by the Engineer and test results shall be submitted to the Engineer in triplicate within 3 days after completion of the test. All electrodes shall be procured with test certificates. The correct grade and size of electrodes not deteriorated in storage shall only be used. The testing of welding shall be performed as under with quantum of minimum non-destructive tests to be conducted during fabrication and after erection as below:
- i) Ultrasonic test should be performed on the columns; girders; Built-up beam fabricated with plates.
 - ii) Fillet welds at junction of flange & web of built-up beams, columns, all shear connections of main beams and all butt welds shall be 100% ultra sonic tested
 - iii) 100% radiographic test shall be performed for butt weld joints of crane girder & its supporting columns, deaerator supporting beams and columns. The minimum percentage of Radiographic test to be carried out at other locations shall be 25 percent.
 - iv) Dypenetration test, Ultrasonic test, Radiographic test shall be carried out at any other location also, if required as per Engineer's approval.

In cases, the test results shows deficiency, the Engineer shall have option to reject or instruct any remedial measures to be carried out by the contractor.

- b) All bolts, nuts and washers shall conform to the relevant IS. If desired by the Engineer, representative samples of these materials should be tested in an approved laboratory and in accordance with the procedures described in relevant IS. All paints and primers shall be of standard quality; and shall conform to the provisions of the relevant IS. The paint shall be epoxy based. The tolerances on the dimensions of individual rolled steel components shall be as per IS:1852. The tolerances on straightness, length etc. of various fabricated components (such as beams and girders, columns, crane gantry girder etc.) of the steel structures subjected to dynamic loading (like wind,



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seismic etc.) and thin walled construction (like box girders) shall be as per IS:7215.

- c) Should any structure or part of a structure be found not to complying to the provisions of the specification, the same shall be liable to rejection. No structure or part of the structure, once rejected, shall be offered again for test, except in cases where the Engineer considers the defects rectifiable. The Engineer may, at his discretion, check the test results obtained at the contractor's works by independent tests at an approved laboratory and should the items, so tested, be found to be unsatisfactory.

When all tests to be performed in the contractor's shop have been successfully carried out, the steelwork will be accepted forthwith; upon receipt of which, the items shall be shop painted, packed and dispatched. No item should be delivered unless an acceptance certificate for the same has been issued. The satisfactory completion of these tests or the issue of the certificates shall not bind the Purchaser to accept the work, should it, on further tests before or after erection, be found not in compliance with spec.

- d) The contractor should deliver the fabricated structural steel materials to site with all necessary field connection materials in a sequence permitting an efficient and economical performance of the erection work. The Purchaser may prescribe or control the sequence of delivery of materials, at his own discretion. Each separate piece of fabricated steelwork shall be distinctly marked on all surfaces before delivery in accordance with the markings shown on approved erection drawings; and shall bear such other marks as will further facilitate identification and erection.

23.11.00 Inspection of Welding

The extent of quality control in respect of welds of structural elements shall be as follows:

a) Visual Examination

All welds shall be 100% visually inspected to check the following:

- i) Presence of undercuts
- ii) Surface cracks in both welds and base metals.
- iii) Unfilled craters
- iv) Improper weld profile and size
- v) Excessive reinforcement in weld



- vi) Surface porosity

Before inspection, the surface of weld metal shall be cleaned of all slag, spatter matter, scales etc. by using wire brush or chisel.

b) Dye penetration Test (DPT)

This test shall be carried out for all fillet welds and groove welds to check the following:

- i) Surface cracks
- ii) Surface porosities

c) Ultrasonic Testing

Ultrasonic test shall be conducted for all groove welds and heat affected zone in dynamically loaded structures and for other important load bearing butt welds in statically loaded structures as desired by Purchaser to detect the following:

- i) Cracks
- ii) Lack of fusion
- iii) Slag inclusion
- iv) Gas porosity

Ultrasonic testing shall be carried out in accordance with American National Standard ANSI/AWS D1.1-92 Chapter 6 Part-C.

Before Ultrasonic test is carried out, any surface irregularity like undercuts, sharp ridges etc. shall be rectified. Material surface to be used for scanning by probes must allow free movement of probes. For this purpose, surface shall be prepared to make it suitable for carrying out ultrasonic examination.

d) Radiographic Testing (X-ray and Gamma-ray Examination)

This test shall be limited to 2% of length of welds for welds made by manual or semi-automatic welding and 1% of length of weld if made by automatic welding machines. The location and extent of weld to be tested by this method shall be decided by Purchaser to detect the following defects:

- i) Gas porosity



- ii) Slag inclusion
- iii) Lack of penetration
- iv) Lack of fusion
- v) Cracks

Radiographic testing shall be conducted in accordance with American National Standard ANSI/AWSD1.1-92.

Any surface irregularity like undercuts, craters, pits, etc. shall be removed before conducting radiographic test. The length of weld to be tested shall not be more than $0.75 \times$ focal distance. The width of the radiographic film shall be equal to width of the welded joint plus 20 mm on either side of the weld.

EPC Contractor shall provide testing equipment for conducting non-destructive tests for confirming the integrity of welding wherever necessary as directed by the Purchaser.

e) Acceptable Limits of Defects of Weld

Limits of acceptability of welding defects shall be as follows:

i) Visual inspection and Dye penetration test

The limits of acceptability of weld defects detected during visual inspection and dye penetration test shall be in accordance with clause 8.15.1 and clauses 9.25.3 of American National Standard ANSI / AWS D1.1-92 respectively, for statically and dynamically loaded structures.

- ii) Ultrasonic testing The limits of acceptability of weld defects detected during ultrasonic testing shall be in accordance with clause 8.15.4 and clause 9.25.3 of American National Standard ANSI/AWS D1.1-92 respectively for statically and dynamically loaded structures.

iii) Radiographic testing:

The limits of acceptability of weld defects detected during Radiographic testing shall be in accordance with clause 8.15.3 and 9.25.2 of American National Standard ANSI/ AWS D1.1-92 respectively for statically and dynamically loaded structures.

f) Rectification of Defects in Welds

In case of detection of defects in welds, the rectification of the same shall be done as follows:



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- i) All craters in the weld and breaks in the weld run shall be thoroughly filled with weld
- ii) Undercuts, beyond acceptable limits, shall be repaired with dressing so as to provide smooth transition of weld to parent metal.

Welds with cracks and also welds with incomplete penetration, porosity, slag inclusion etc., exceeding permissible limits shall be rectified by removing the length of weld at the location of such defects plus 10 mm from both ends of defective weld and shall be re-welded. Defective weld shall be removed by chipping hammer gouging torch wheel. Care shall be taken not to damage the adjacent material.



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24.00.00 ERECTION OF STRUCTURAL STEELWORK

24.01.00 The works related to the erection of structural steelwork including receiving and taking delivery of fabricated structural steel materials arriving at site, installing the same in position, painting and grouting the stanchion bases all complete are detailed below:

- a) Providing all construction & transport equipment, tools, tackles, consumables, materials, labour and supervision as required for the erection of the structural steelwork.
- b) Receiving, unloading, checking and moving to storage yard at site including prompt attendance to all insurance matters as necessary.
- c) Transportation of all fabricated structural steel materials from site storage yard, handling, rigging, assembling, bolting, welding and satisfactory installation in proper location as per approved erection drawings. If necessary suitable temporary approach roads should be built for transportation.
- d) Checking centerlines, levels of all foundation blocks including checking line, level, position and plumb of all bolts and pockets. Any defect observed in the foundation shall be brought to the notice of the Engineer. The contractor shall fully satisfy himself regarding the correctness of the foundations before installing the fabricated steel structures on the foundation blocks.
- e) Aligning, plumbing, leveling, bolting, welding and securely fixing the fabricated steel structures as per drawings.
- f) Painting of the erected steel structures.
- g) Minor modifications of the fabricated steel structures as directed by the Engineer including but not limited to the following:-
 - i) Removal of bends, kinks, twists etc. for parts damaged during transport and handling.
 - ii) Cutting, chipping, filling, grinding etc. if required for preparation and finishing of site connections.
 - iii) Reaming of holes for use of higher size bolt if required.
 - iv) Welding of connections in place of bolting for which holes are either not drilled at all or wrongly drilled during fabrication.
 - v) Refabrication of parts damaged beyond repair during transport and handling or refabrication of parts which are incorrectly fabricated.



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- vi) Fabrication of parts omitted during fabrication by error, or subsequently found necessary.
- vii) Drilling of holes which are either not drilled at all or drilled in incorrect location during fabrication

24.02.00

- a) The work shall conform to the latest revisions of the following IS Codes:

IS-800	:	Code of Practice for general construction in Steel
IS-456	:	Code of Practice for plain or reinforced concrete
IS-7205	:	Safety Code for erection of Structural Steel work
IS-12840	:	Tolerance for erection of Steel Structures

- b) Conformity with designs: The contractor should erect the fabricated steel structures, align all the members, complete all field connections as per approved drawings. All works shall conform to the provisions of the relevant IS. The testing and acceptance of the erected structures shall be in accordance with the provisions of this specification.

24.03.00

- a) The contractor should take delivery of all the materials at site. He shall unload the materials and perform all formalities such as checking of materials and attend to insurance matters as specified above.

Contractor shall make good any such deficiency, if detected later, either by repair or with fresh material as may be directed by the Engineer at the contractor's own cost. All field connection materials such as bolts, nuts, washers and electrodes, other consumables such as oxygen and acetylene gas, paints, fuels, lubricants, oil, grease and any other material as required for the execution of the works shall be supplied by the contractor for erection work.

- b) All materials shall be stored preventing deterioration and ensuring the preservation of their quality and fitness for use in the works. Any material which has been deteriorated or damaged beyond repairs and has become unfit for use shall be removed immediately from the site. The contractor should establish a suitable yard at site for storing the fabricated steel structures and other materials. The yard shall have proper facilities such as drainage, lighting, suitable access for large cranes, trailers and other heavy equipment. The yard shall be fenced all around with security arrangement and shall be of sufficiently large area to permit systematic storage of the fabricated steel structures without overcrowding. All field connection materials, paints, cement etc. shall be stored on well designed racks and platforms off the ground in a properly covered store building.
- c) The contractor shall establish and maintain quality control procedures for different items of work and materials; and shall submit the records of the same to the Engineer. The quality control operation shall include but not be limited to the following:



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- i) Erection : Lines, levels, grades, plumbs, joint characteristics including tightness of bolts.
- ii) Painting : Preparation of surface for painting, quality of primers and paints, thinners, application and uniformity of coats.

24.04.00 Workmanship

- a) The suitability and adequacy of all erection tools and plant and equipment proposed to be used shall be efficient, dependable, in good working condition. The method and sequence of erection shall have the prior approval of the Engineer. The Erection shall arrange in most economical method; and sequence available to him consistent with the drawings.
- b) Unless adequate bracing is included as a part of the permanent framing, the erector during erection shall install, temporary guys and bracings where needed to secure the framing against loads such as wind or seismic forces comparable in intensity to that for which the structure has been designed, acting upon exposed framing as well as loads due to erection equipment and erection operations.

If additional temporary guys are required to resist wind or seismic forces acting upon components of the finished structure during the course of the erection of the steel framing, arrangement for installation by the erector shall be made.

The responsibility of the contractor in respect of temporary bracings and guys shall cease when the structural steel is once located, plumbed, leveled, aligned and grouted within the tolerances permitted under the specification and guyed and braced to the satisfaction of the Engineer. The temporary guys, braces, false work and cribbing shall be removed immediately upon completion of the erection

- c) Positioning and leveling of all steelwork, plumbing of stanchions and placing of every part of the structure with accuracy shall be as per approved drawings. Anchor bolts and other anchor steel shall be embedded. The contractor shall check the positions and levels of the anchor bolts, etc. before concreting and get them properly secured against disturbance during pouring operations. He shall remain responsible for correct positioning. For heavy columns, the contractor shall set proper screed bars to maintain proper level. Each tier of column shall be plumbed and maintained in a true vertical position subject to the limits of tolerance allowable. No permanent field connections by bolting or welding shall be carried out until proper alignment and plumbing has been attained.
- d) All relevant portions in respect of bolted construction for fabrication of



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structural steelwork shall also be applicable for field bolting as below:

Bolts shall be inserted in such a way so that they may remain in position under gravity even before fixing the nut. Bolted parts shall fit solidly together when assembled; and shall not be separated by gaskets or any other interposed compressible materials. When assembled, all joint surfaces, including those adjacent to the washers shall be free of scales except light mill scales. They shall be free of dirt, loose scales, burns, and other defects that would prevent solid seating of the parts. Contact surfaces within friction-type joints shall be free of oil, paint, lacquer, or galvanizing. High tensile bolts shall be tightened to provide the required minimum bolt tension by any of the following methods:-

Turn-of-nut method: When the turn-of-nut method is used to provide the bolt tension, there shall first be enough bolts brought to a "Snug tight" condition to ensure that the parts of the joint are brought into good contact with each other. "Snug tight" is defined as the tightness attained by a few impacts of an impact wrench or the full effort of a man using an ordinary spud wrench. Following this initial operation, bolts shall be placed in any remaining holes in the connection and brought to snug tightness. All bolts in the joint shall then be tightened additionally by the applicable amount of nut rotation as below with tightening progressing systematically from the most rigid part of the joint to its free edges. During this operation, there shall be no rotation of the part not turned by the wrench.

Bolts length not Bolt length exceeding 8 times dia or 200mm	Bolts length not Bolt length exceeding 8 times dia or 200mm	Remarks
½ turn	2/3 turn	Nut rotaion is relative to bolt regardless of the element (nut or bolt) being turned. Tolerance on rotaion – 30 over or under

Bolts shall be installed without hardened washers when tightening is done by the turn-of-nut method. However, normal washers shall be used.

Bolts tightened by the turn-of-nut method may have the outer face of the nut match-marked with the protruding bolt point before final tightening, thus affording the inspector visual means of noting the actual nut rotation. Such marks shall be made by the wrench operator by suitable means after the bolts have been brought up snug tight.

Torque Wrench tightening: When torque wrenches are used to provide the



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bolt tensions, the bolts shall be tightened to the torques as below. Nuts shall be in tightening motion when torque is measured. When using torque wrenches to install several bolts in a single joint, the wrench shall be returned to touch up bolts previously tightened, which may have been loosened by the tightening of subsequent bolts, until all are tightened to the required tension.

The above torque values are approximate for providing tensions of 14.7 MT for 20 mm dia; 18.2 MT for 22 mm dia; and 21.2 MT for 24 mm dia. bolts under moderately lubricated condition. The torque wrench shall be calibrated at least once daily to find out the actual torque required to produce the above required tension in the bolt by placing it in a tension indicating device. These torques shall be applied for tightening the bolts on that day with the particular torque wrench.

In either of the above two methods, if required, for bolt entering and wrench operation clearances, tightening shall be done by turning the bolt while the nut is prevented from rotating.

Impact wrenches if used shall be of adequate capacity and sufficiently supplied with air to perform the required tightening of each bolt in approximately ten seconds.

Holes for turned bolts to be inserted in the field shall be reamed in the field. All drilling and reaming for turned bolts shall be done only after the parts to be connected are assembled. Tolerances applicable in the fit of the bolts shall be as per IS.

- e) Field Welding: All field assembly and welding shall be carried out as specified for fabrication work, excepting such provisions therein which manifestly apply to shop conditions only. Where the fabricated structural steel members have been delivered painted, the paint shall be removed before field welding for a distance of at least 50 mm on either side of the joints.
- f) Holes, cutting and fitting: No cutting of sections, flanges, webs, cleats, bolts, welds etc. shall be done. The erector shall not cut, drill or otherwise alter the work of other trades, or his own work to accommodate other trades, unless such work is clearly specified. Wherever such work is specified the contractor shall obtain complete information as to size, location and number of alterations prior to carrying out any work.

24.05.00

Drifting

Correction of minor misfits and reasonable amount of reaming and cutting of excess stock shall be considered as permissible. For this, light drifting shall be used to draw holes together; and drills shall be used to enlarge holes as necessary to make connections. Reaming, that weakens the member or makes it impossible to fill the holes properly or to adjust accurately after reaming shall not be allowed.



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Any shop work error which prevents the proper assembling and fitting of parts by moderate use of drift pins and reamers shall immediately be called to the attention of the Engineer and approval of the method of correction obtained. The use of gas cutting torches at erection site is prohibited.

24.06.00 Testing and Acceptance Criteria

- a) Loading tests shall be carried out on erected structures to check adequacy of fabrication and/or erection. Any structure or a part thereof found to be unsuitable for acceptance as a result of the test shall be dismantled and replaced with suitable member. On the basis of the tests, the Engineer will decide and his decision will be final. In course of dismantling, if any damage is done to any other parts of the structure or to any fixtures, the same shall be made good.

The structure or structural member under consideration shall be loaded with its actual dead load for as long a time as possible before testing; and the tests shall be conducted as indicated below:-

- i) Stiffness Test: In this test, the structure or member shall be subjected, in addition to its actual dead load, to a test load equal to 1.5 times the specified superimposed load, and this loading shall be maintained for 24 hours. The maximum deflection attained during the test shall be within the permissible limit. If, after removal of the test load, the member or structure does not show a recovery of at least 80 per cent of the maximum strain or deflection shown during 24 hours under load, the test shall be repeated. The structure or member shall be considered to have sufficient stiffness, provided that the recovery after this second test is not less than 90 per cent of the maximum increase in strain or deflection recorded during the second test.
- ii) Strength Test: The structure or structural member under consideration shall be subjected, in addition to its actual dead load, to a test load equal to the sum of the dead load and twice the specified superimposed load, and this load shall be maintained for 24 hours.

In the case of wind load, a load corresponding to twice the specified wind load shall be applied and maintained for 24 hours, either with or without the vertical test load for more severe condition in the member under consideration or the structure as a whole. Complete tests under both conditions may be necessary to verify the strength of the structure. The structure shall be deemed to have adequate strength if, during the test, no part fails and if on removal of the test load, the structure shows a recovery of at least 20 per cent of the maximum deflection or strain recorded during the 24 hours under load.



b) Structure of same design:

Where several identical same design structures exists as a prototype, one structure shall be fully tested, but in addition, during the first application of the test load, particular note shall be taken of the strain or deflection when the test load 1.5 times the specified superimposed load has been maintained for 24 hours.

When a structure of the same type is selected for a check test, it shall be subjected, in addition to its actual dead load, to a superimposed test load, equal to 1.5 time the specified live load, in a manner prescribed by the Engineer. This load shall be maintained for 24 hours, during which time, the maximum deflection shall be recorded. The check test shall be considered satisfactory, provided that the maximum strain or deflection recorded in the check test does not exceed by more than 20% of the maximum strain or deflection recorded at similar load in the test on the prototype.

- c)** Repair for subsequent test and use after strength tests: The structure passed the "Strength Test" as above and is subsequently to be erected for use, shall be considered satisfactory for use after it has been strengthened by replacing any distorted members and has subsequently satisfied the 'Stiffness Test' as specified in above.

24.07.00 Tolerances

Considering expected variation in the finished dimensions of structural steel frames, these shall be within the limits of good practice when they are not in excess of the cumulative effect of detailed erection clearances, fabrication tolerances for the finished parts; and the rolling tolerances for the profile dimensions permitted under the specification for fabrication of structural steelwork shall be as indicated below:

	Component	Description	Variation Allowed
a	For Buildings Containing Cranes		
	i) Main Column	a) Shifting of column axis at foundation level with respect to building line	
		i) In longitudinal direction	(+/-) 3.0mm
		ii) In lateral direction	(+/-) 3.0mm
		b) Deviation of both major column axis from vertical between foundation and	



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Component	Description	Variation Allowed
	other member connection levels :	
	i) For a column upto including 10M	(+/-) 3.5 mm and from true height vertical
	ii) For a column greater than 10M but less than 40M height	(+/-) 3.5 mm from true vertical for length measured between connection levels, but not more than (+/-) 7.0 mm per 30 m length
	c) For adjacent pairs of columns across the width of the building prior to placing of truss.	(+/-) 9 mm on true span.
	d) For any individual column deviation of any bearing or resting level from levels shown on drawings.	(+/-) 3 mm
	e) For adjacent pairs of columns either across the width of building or longitudinally level difference allowed between bearing or seating level supposed to be at the same level.	3 mm
ii) Trusses	a) Deviation at centre of span of upper chord member from vertical plane running through centre of bottom chord	1/1500 of the span or not greater than 10 mm whichever is the least
	b) Lateral displacement of top chord at centre of span from vertical plane running through centre of supports.	1/250 of depth of truss or 20 mm whichever is the least.
iii) Cranes Girders & Tracks	a) Difference in levels of crane rail measured between adjacent columns.	2.0 mm
	b) Deviation to crane rail gauge	(+/-) 3 mm
	c) Relative shifting of ends of adjacent crane rail in plan and elevation after thermit welding.	1.0 mm



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Component	Description	Variation Allowed
	d) Deviation of crane rail axis from centre line of web.	(+/-) 3.5 mm
iv) Setting of expansion gaps	At the time of setting of the expansion gaps, due regard shall be taken of the ambient temperature above or below 30°C. The coefficient of expansion or contraction shall be taken as 0.000012 per Deg.C per unit length.	
b For Buildings without Cranes		
The maximum tolerances for line and level of the steel work shall be ± 3 mm on any part of the structure. The structure shall not be out of plumb more than 3.5 mm on each 10 m section of height and not more than 7 mm per 30 m section. These tolerances shall apply to all parts of the structure unless the drawings issued for erection purposes state otherwise.		



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25.00.00 MASONRY AND ALLIED WORKS

25.01.00 The masonry and allied works associated with the offered power plant cover furnishing, installation, repairing, finishing, curing, protection, maintenance and handing over of all the associated works for use in structures as per the locations shown in approved drawings in accordance with latest IS:2212/IS:2250/ IS:5134.

25.02.00 Brick masonry

i) All masonry work shall be true to lines and levels as shown on drawings. All masonry shall be tightly built against structural members and bonded with dowels, inserts etc. as shown on drawings.

ii) Mortar

Mix for mortar shall be as specified. Cement sand mortar cement and sand in 1:4 proportions shall be mixed dry in a mechanical mixer; and then water added and mixed further to achieve working consistency. Surplus mortar droppings from masonry, if received on surface free from dirt shall be mixed with fresh mortar with additional cement. No mortar which has stood for more than half an hour shall be used.

iii) Brick

Fly ash Bricks shall be soaked by submergence in clean water for at least 2 hours in approved vats before use. Bricks shall be laid in English bond. Broken bricks shall not be used. Cut bricks shall be used if necessary to complete bond or as closers. Bricks shall be laid with frogs upwards over full mortar beds. Bricks shall be pressed into mortar and tapped into final position so as to embed fully in mortar. Inside faces shall be buttered with mortar before the next bricks are placed and pressed against it. Thus all joints between bricks shall be fully filled with mortar.

Mortar joints shall be kept uniformly 10 mm thick. All joints on face shall be raked to minimum 10 mm depth using raking tool with green mortar to provide bond for plaster or pointing. Where plaster or pointing is not provided, the joints shall be struck flush and finished immediately. 2 brick thick or more brickwork shall have both faces in true plane. Brickwork of lesser thickness shall have one selected face in true plane.

iv) Exposed Brickwork

Almost all the brickwork shall be plastered with cement mortar. Brickwork in superstructures uncovered by plaster shall be executed by skilled mason. Courses shall be truly horizontal and vertical joints truly vertical. Wooden straight edges with brick course graduations and position of window sills and lintels shall be used to control uniformity of brick courses. Masons should check workmanship frequently with plumb, spirit level, rule and



string. All brick work shall be cleaned at the end of day's work. In case of face bricks involvement the brickwork shall be in composite shape with face bricks on the exposed face and balance in routine bricks, maintaining the bond fully; using carborandum stone for rubbing down. Where face bricks are not needed, bricks for the exposed face shall be specially selected from routine bricks. All exposed brickwork on completion of work shall be rubbed down, washed clean and pointed as specified.

v) **Reinforced Brickworks**

Reinforcements shall be as specified. The reinforcements shall be thoroughly cleaned and fully embedded in mortar. Where M.S. bars are used as reinforcement, these shall be lapped with dowels if left in R.C. columns or welded to steel stanchions. For partition walls 2 reinforcement bars shall be placed at every fourth layer.

vi) **Encasement of Structural Steel**

The fixtures shall generally be embedded in mortar and masonry units shall be cut as required. Encasement of structural steel shall be done by brick masonry work round flanges, webs etc. and filling the gap between steel and masonry by minimum 12 mm thick mortar. Encased members shall be wrapped with chicken wire mesh with a lap of 50 mm.

vii) All wall beams shall be provided with gunniting of 50 mm thick alround with nominal reinforcement and chicken mesh. Also, chicken wire mesh shall be provided at the junction of RCC & brick work. The wall beam/tie beam shall be provided at every 2.5 meter height of the brick wall. Chicken wire mesh shall be provided at the joints of brick masonry and RCC beams/column.

vii) **Curing**

The brick masonry works shall be cured for a period of fourteen (14) days after laying and the plaster shall be cured for a period of seven (7) days.



26.00.00 ARCHITECTURAL FINISHES**26.01.00 Plastering**

- a) The plastering work including the application of cement punning (Neru) or plaster of Paris treatment on brick or concrete faces with architectural features shall be executed as per in accordance with the following latest IS codes: POP shall be used only on inside surfaces of ceiling and walls for aesthetic and architectural requirement.

IS:1542 : Sand for plastering

IS:1661 : Code of practice for application of cement and cement lime plaster finishes.

IS:2333 : Plaster of Paris

IS:2402 : Code of practice for external rendered finishes

IS:2394 : Code of practice for application of lime plaster finishes

- b) Mortar for plastering shall be mixed in the proportion in a dry state and then wetted and mixed thoroughly to obtain the required consistency. The mortar shall be mixed in an approved manner including machine mixing in batches for its consumption within half an hour of mixing. Any mortar for partially set plaster shall be rejected and removed from site. The mix for plastering shall be as follows:

Brick wall:

- i) Outside plaster (or rough side): 20 mm thick in 2 coats, 1st coat of 12 mm thick in C:M 1:3 & 2nd coat of 8 mm thick in C:M 1:2 sand faced.
- ii) Inside plaster (or plain face) : 12 mm thick (1 cement : 3 sand)
- iii) Concrete ceiling: Plaster : 6 mm thick (1 cement : 3 sand)

- c) Before application of plaster, the surface shall be prepared as per IS:1661. In all plaster work, mortar shall be applied in an uniform layer slightly more than the required thickness and well pressed into the joint and in the surface and rubbed & levelled with a flat wooden rule to give required thickness.



Plaster, when more than 15 mm thick, shall be applied in two coats, base coat followed by the finishing coat. Thickness of base coat shall be just sufficient to fillup all unevenness in the surface; no single coat, however, shall exceed 12 mm in thickness. The under-coat shall be thicker than the upper coat. The overall thickness of the plaster shall not be less than the minimum thickness shown on the drawings. The undercoat shall be allowed to dry and shrink before applying the second coat of plaster. The undercoat shall be scratched or roughened before it is fully hardened to form a mechanical key. The method of application shall be "thrown on" rather than "applied by trowel". The finished surface shall be true to line & plumb; and the contractor shall make up any irregularity in the masonry/concrete work with plaster. The mortar shall adhere to the surface intimately when set; and there should be no hollow sound when struck.

All vertical edges of pillars, door jambs etc. shall be chamfered or rounded off. All corners must be finished to their true angles or rounded. Any plastering damaged shall be repaired and left in good condition at the completion of the job.

- d) All plastered surfaces after laying and sufficiently hardened shall be cured for a minimum period of seven days and shall be protected from excessive heat and sunlight by suitable approved means.
- e) **Plaster of Paris Finish:** The plaster of Paris shall be calcium sulphate hemi-hydrate variety. Its initial setting shall be less than 13 minutes. The material shall be mixed with water to workable consistency. Plaster of Paris shall be applied to the surface in the uniform layer slightly more than 2 mm thick and shall be finished to an even and smooth surface with a steel trowel. Thickness of finish shall not be less than 2 mm. All corners, arises, angle and junctions shall be carefully and neatly finished.

26.02.00 Wall Cladding

26.02.01 Permanent colour coated sandwiched insulated metal cladding system

- a) Permanent colour coated sandwiched (insulated) M.S. / High tensile steel metal cladding of approved colour combination shall be provided for main plant building and any other building with metal cladding where thermal insulation is required as per thermal requirements.
- b) Troughed zinc-aluminum alloy coated (both sides) M.S. sheet having 0.6mm minimum thickness (or high tensile steel sheet of 0.5 mm minimum thickness) shall be used on external face (outer face) of cladding system. Weight of coating shall not be less than 150gms /sq.m. The outer side (exposed face) shall be permanently colour coated with Polyfluro Vinyl Coating (PVF2) of Dry Film Thickness



(DFT) 20 microns (min) over primer. Inner side of external sheet shall be provided with suitable pre-coating of minimum 7 microns.

- c) Galvanised M.S. sheets of minimum 0.6 mm thickness shall be used as inner liner (internal face) of cladding system. The exposed face shall be permanently colour coated with silicon modified polyester paint of DFT microns (min) over primer. Inner face of external sheet shall be provided with suitable pre-coating of minimum 7 microns. The rate of galvanization shall not be less than 180 gm/sq.m.
- d) The permanent colour coated sheet shall meet the general requirements of IS:14246 and shall conform to class 3 for the durability.
- e) Inner sheet shall fixed directly to side runner and Z spacers made of at least 2 mm thick galvanized sheet of grade 375 as per IS:277. Inner sheet shall be fixed at the rate not more than 1.50m centre to centre to hold the insulation and external sheeting.

26.03.0

Permanent colour coated non-insulated metal cladding system

- a) Permanent colour coated (non-insulated) M.S / High tensile steel metal cladding of approved colour combination shall be provided for bunker building and cladding over parapet walls of buildings where metal cladding is specified 0.8mm with galvanization rate of 275 gm/sqm.
- b) Troughed zinc-aluminum alloy coated not less than 150 gm/sqm M.S. sheet having 0.6mm minimum thickness (or high tensile steel sheet of 0.5 mm minimum thickness) shall be used for cladding system. The outer side (exposed face) shall be permanently colour coated with PVF2 paint of minimum DFT 20 microns over primer and the inner side (internal face) shall be coated with same paint of minimum DFT 12 microns over primer. These shall be fixed directly to runners. The sheets shall meet the general requirements of IS: 14246 and shall conform to class 3 for the durability. For roof sheeting the specification remains same as that of side cladding except the thickness and galvanization. The minimum thickness of roof sheeting shall be 0.8mm with galvanization rate of 275 gm/sqm.

26.04.00

Roof Decking And Suspended Ceiling

- a) The roof decking includes furnishing of cold rolled formed troughed profile sheets manufactured from tested quality CR rolls conforming to IS:513 including fixing. The profile sheet section shall be obtained from feeding strips of uniform coil thickness through successive pairs of shaped rollers, each pair of rollers progressively forming the sheet until finished section is obtained.

In general the troughed profile shall have minimum depth of valley 44mm and center to center of valley about 130mm. The thickness of sheet shall be 0.8 mm (minimum). To suit the spacing of purlin, the length of sheet may vary between 1.50 to 2.00 m. The overall minimum width of sheet



shall be 824.20mm and covering width as 780mm i.e. side overlap shall be 22.10 mm.

- b. Pre-Treatment & Phosphating: The pre-treatment process shall conform B to medium class-B conforming to IS:3618. Next operation of rust removal shall be done by dipping in a tank containing highly diluted hydrochloric acid (HCL); and traces of HCL shall be removed by double rinsing in a water tank for about 5 minutes each. Subsequently sufficient conditioning shall be done by dipping in the next tank containing surface conditioner. Hot phosphating shall be accomplished in the next tank followed by water rinsing and finally passivated by heat process in the passivation tank. Thereafter traces of moisture shall be removed by hot compressed air before application of primer and final paint coating as required. In general the pre- treatment stages shall be as under :

Sl.NO	STAGES	CHEMICALS	PROCESSING	PROCESSING
1	Degreasing	Cleaner	55 to 75° C	3 to 10 mins
2	Water Rinsing	-	Room Temp	-
3	Derusting	Rust remover	R. Temp to 60°c	1 to 10 Mts
4	Water Rinsing	-	Room Temp	-
5	Surface conditioning	Surface conditioner	Room Temp	15 to 30 sec
6	Phosphating solution	Phosphating	50 to 55° C	3 to 5 Mins
7	Water Rinsing	-	Room Temp	-
8	Passivation solution	Passivation	Room Temp	15 to 30sec
9	Water Rinsing	-	Room Temp	-
10	Defonised water	-	Room Temp	-
11	Drying	-	-	-

After pre-treatment, the sheet shall be painted with a coat of zinc chromate red oxide primer on both faces.

26.05.00

Flooring

The nominal total thickness of floor finish shall be 50 mm including under bed and topping. The flooring shall be laid on already matured concrete base. The under bed for floors shall consist of cement concrete 1:2:4 with stone chips 12.5 mm down graded as coarse aggregates. The under bed shall be provided with appropriate slope towards catch pit for floor drainage.

26.05.01

False Flooring System

Removable type false flooring system shall be provided in computer rooms and control rooms as required. RCC floor slab shall be sunk to a depth 800 mm which shall be height of the false floor system. The flooring shall consist of fire resistant phenol formaldehyde bonded particle board panels 600x600x35mm size, mounted on steel pedestals of adjustable height and supporting steel grid system to provide under floor space. 2 mm thick flexible anti-static pvc topping on top and



pvc strip edging on sides of each panel shall be provided .

26.05.02 PVC Floor Finish

Two mm thick PVC as per IS:3462 laid as per IS:5318 over concrete under bed of 48 mm .

26.05.03 Carborandum Tiles

Polished heavy duty cement concrete tiles (carborandum) of 300x300x22 mm thick manufactured as per IS:1237 using colouring pigment and hard chips like carborandum, quartz etc shall be laid as per IS:1443 over concrete under bed to result in overall thickness of 50 mm.

26.05.04 Terrazzo Tiles

Tiles shall generally be of size 250 x 250 x 20 mm laid over concrete bedding to result in an overall thickness of 50 mm.

26.05.05 Granolithic flooring

- a) Granolithic flooring (cement concrete flooring in 1:1:2) with non metallic floor hardener topping 12 mm thick with a total thickness of 50 mm shall be provided in maintenance and unloading area of Station building, Mill and bunkerbay, Workshop floors, operating floor of pump house,
- b) Granolithic flooring without floor hardener shall be provided in all MCC and switch gear rooms which are not air-conditioned.
- c) Granolithic flooring shall also be provided in areas which are not provided with any special finish. Areas which are likely to be subjected to oil spillage shall be provided with two coats of oil resistant painting over Granolithic flooring.

26.05.06 Heavy Duty Ceramic Tiles

Heavy duty anti skid ceramic tiles with matt finish shall be used in toilets, pantry, dining hall of canteen, locker rooms etc. The tiles shall be 300 x 300 x 7 mm of approved shade brand and colour. Dado in toilets and pantry ,locker rooms etc shall also be of similar finish.

26.05.07 Acid / Alkali resisting Tiles

Battery rooms, and other areas coming into contact with acid / alkali vapors or fumes shall be given acid / alkali resistant tiles 25 mm thick, jointed with acid / alkali resistant cement slurry. Bedding shall



comprise of potassium silicate mortar conforming to IS:4832 (Part-I) and resin based mortar like epoxy for jointing. Total thickness of flooring shall be 50 mm. Ceramic unglazed vitreous tiles conforming to IS:4457 with minimum thickness of 20 mm may also be used as acid / alkali resistant tile. The above specification do not apply to D.M. Plant.

26.05.08 Integral floor finish

For cable vaults room, floors of wagon tripler shed, reclaim hopper shed, crusher house, junction towers, pent house shall be provided with floor finish integral to the concrete base shall be provided as per IS:2571.

26.05.09 Cast-in-situ Terrazzo

Cast in situ terrazzo flooring shall be laid as per IS:2114, using white cement or cement with colouring pigment. Chequered finish shall be provided for treads. Total thickness of the finish shall be 25 mm.

26.05.10 Acid / Alkali resistant Tiling / Brick lining in D.M Plant

- a) Bitumen primer followed by 12 mm thick bitumastic layer, 6 mm thick potassium silicate mortar bedding and 38 mm thick alkali / acid resistant bricks as per IS:4860 shall be provided for CPU regeneration area, Chemical house floor, effluent drains, floors around equipment & chemical handling vessels, chemical storage area for the floor, curbs and sumps, all as per the acid / alkali proofing specialist Contractor's requirement.
- b) For floor of neutralizing pit the finish shall be as follows.
 Bitumen primer followed by 18 mm thick bitumastic layer, 6 mm thick potassium silicate mortar bedding and 75 mm thick acid / alkali resistant brick as per IS:4860.
- c) For walls of neutralizing pit, the same specification as 1.10.2 shall apply except that thickness of the brickwork shall be 115 mm with suitable pilasters at 2000 mm c/c.
- d) Special instruction to be followed for acid resistant lining in neutralizing pit shall be as follows.
 - i) The structures shall be tested for water tightness.
 - ii) Surface on which lining is to be applied shall be prepared as per IS:2395.
 - iii) Joints between acid resistant bricks / tiles shall be filled with resin type mortar conforming to IS:4832 (Part II). Seal coat of ready made epoxy paint shall



be provided at the joints to cover up any porosity.

- iv) Acid resistant bricks shall be laid with 6 mm wide and 20 mm deep pointing. Pointing shall be with epoxy / furane / CNSL as per the requirement of the agency guaranteeing the performance of lining.
- v) Under side of all precast slabs / steel covers over effluent drains shall be given two coats of epoxy coating, 150 microns thick.
- vi) Acid / alkali resistant treatment shall extend at least 1 meter on all sides from the outermost periphery of pedestals / saddles for indoor installations and 2 meters all round for outdoor installations.

26.05.11 Glazed vitrified tiles

Polished vitrified tiles shall be of 600 mm x 600 mm x 7.5 mm in size and shall be approved shade, brand and colour and shall be laid with CM 1:3. These are proposed in the following areas:

Operating floor, Control room area including control room, computer room, control equipment room, SWAS Room and Conference room, senior executive room – The floor shall be vitrified tiles of size 2 feet X 2 feet

26.05.12 Miscellaneous

- a) PVC nosing / Aluminum angle nosing with minimum size (150 x 25 x 3) shall be provided for edge protection of R.C.C. stair treads.
- b) Angles 50x50x6 mm(min) with lugs shall be provided for edge protection of cutouts / openings in floor slabs, edge of drains covering gratings, edge of R.C.C cable / pipe trenches and at any other place where breakage of edges / corners is expected.
- c) Floors of switchgear rooms shall have embedded steel channel suitable for easy movement of breaker panel.

26.06.00 Skirting / Dado

- a) 150 mm skirting matching with floor finish shall be provided in all areas unless specified otherwise elsewhere.
- b) Toilets & locker rooms shall be provided with dado of 2100 mm high.
- c) For main Control room and control equipment room minimum 5 mm thick decorative coloured ceramic tiles shall be provided upto false ceiling level.



- d) For battery room and other areas coming in contact with acid / alkali spillage/ fume, dado of acid / alkali resistant tiling as per IS:4457 shall be provided to a height of 2100 mm set in potassium silicate mortar and joints pointed with resin bonded mortar.
- e) Staircase wall shall be given dado of cast in situ terrazzo to a height of 2100mm. If Entrance lobby and lift area in Service building and Admin building shall be provided with granite tile dado to a height upto false ceiling level.

26.07.00**Doors, Windows, Ventilators, Louvers Etc.****26.07.01**

Unless specified all doors, windows and ventilators of air conditioned areas, entrance lobby of all buildings and windows/ventilators provided on the outer face of all buildings shall have, electro colour coated (anodized) aluminum framework with glazing. All doors of office areas shall be of factory made pre-laminated particle board (MDF exterior grade). All other doors (unless otherwise specified) shall be of steel.

Main entrance of the control room and control equipment room shall be provided with air-locked lobby with provision of double doors / glazed panels of aluminum frame work with toughened safety double glass glazing conforming to IS 2553 hermitically sealed and separated by 12 mm thick cup for thermal insulation. Doors shall be of double swing type or sliding type.

For common control building, double glazed wall panels with aluminum frame shall be provided between air-conditioned and non air- conditioned areas and on the side of control room and control equipment room facing the operating floor to have a clear view.

Single glazed panels with aluminum frame work shall be provided as partition between two air-conditioned areas wherever clear view is necessary.

Coal conveyor galleries shall have steel windows/ventilators shall be as per IS 1038

All steel doors shall consist of double plate flush door shutters. The door shutter shall be 45 mm thick with two outer sheets of 18 G rigidly connected with continuous vertical 20 G stiffeners at the rate of 150 mm centre to centre. Side, top and bottom edges of shutters shall be reinforced by continuous pressed steel channel with minimum 18 G. The door shall be sound deadened by filling the inside void with mineral wool. Doors shall be complete with all hardware and fixtures like door closer, tower bolts, handles, stoppers, aldrops, etc.



Steel windows and ventilators for coal conveyor gallery shall be as per IS:1361 and for all other areas as per IS:1038. Windows of coal galleries shall be provided with wire mesh.

Wherever functionally required rolling shutters with suitable operating arrangement Manual / Electric shall be provided to facilitate smooth operations. Rolling shutters shall conform to IS:6248.

All windows and ventilators on ground floor of all buildings located in isolated areas shall be provided with suitable anodized aluminum grill.

Fire proof doors with panic devices shall be provided at all fire exit points as per the recommendations of Tariff Advisory Committee (TAC). These doors shall generally be as per IS:3614 (Part I and Part II). Fire rating of the doors shall be as per TAC requirements. However minimum rating shall be 2 hours. These doors shall be double cover plated type with mineral wool insulation.

Hollow extruded section of minimum 3mm wall thickness as manufactured by INDAL or equivalent shall be used for all aluminum doors, windows and ventilators.

All door sizes shall be decided carefully, keeping in view the size of the machinery/panels to be erected inside the building. In any case opening height shall not be less than maximum height of machinery/panel installed inside the building.

IS Codes

Following are some of the important IS codes (latest edition) applicable to this section:

IS:204 (Part-II)	: Tower bolts non ferrous metals
IS:208	: Door handles
IS:2002 (Part-I)	: Plywood face panels
IS:1341	: Steel butt hinges
IS:1868	: Anodic coatings on aluminum and its alloys
IS:3564	: Door closers (hydraulically regulated)
IS:5187	: Flush bolts
IS:6315	: Floor springs (hydraulically regulated) for heavy doors



IS:7195	: Hold fast
IS:7452	: Hot rolled steel sections for doors, windows and ventilators
IS:10019	: Mild steel stays and fasteners
IS:1038	: Steel doors, windows and ventilators
IS:1361	: Steel windows for industrial ventilators
IS:1948	: Aluminum doors, windows and ventilators
IS:1949	: Aluminum windows for industrial buildings
IS:4351	: Steel door frames
IS:3614 (Part-I)	: Fire check doors
IS:10451	: Steel sliding shutters (top hung type)
IS:4021	: Timber door, window and ventilator frames
IS:1003 (Part-I)	: Timber paneled and glazed shutters– door shutters.

26.07.02**Wall paneling**

All the exposed structural steel i.e. wall, columns, bracings in STG control room and air-conditioning areas shall be provided with wooden wall paneling. The wall panel shall consist of teak wood batten grid of size 600x600 mm or as per site requirement. The size of batten shall be 2 inch x 1 inch. BWP grade plywood conforming to IS:710 shall be fixed on this grid. The Vanner of approved quality and shade shall be fixed on the plywood as exterior finish with suitable adhesive. The entire work shall be carried out to the satisfaction of engineer.

26.08.00**Glass and Glazing**

All ventilators and windows on external face of turbine building, conveyor gallery, pump house, compressor house, DG set building, transfer points, workshop building, fire escape staircase and those buildings located in fire prone areas shall be provided with wired glass of minimum 6 mm thickness conforming IS:5437.

Glazing in control room between AC and Non AC areas shall be double glazing consisting of two 6 mm thick clear toughened safety glass conforming to IS:2553, hermetically sealed and separated by 12 mm thick gap for thermal insulation.

For single glazed aluminum partitions and doors, Float glass or flat transparent sheet glass of minimum 6 mm thickness shall be used.



Ground glass / frosted glass of minimum 4 mm thickness shall be used for all windows / ventilators in toilets.

Unless specified otherwise in this specification minimum thickness of plain sheet glass used for windows/ventilators shall be 4 mm.

Float glass or flat transparent sheet glass shall conform to IS:2835. All

glazing work shall conform to IS:1083 and IS:3548

26.09.00 False Ceiling and Under Deck Insulation

All air conditioned areas shall be provided with the suspended false ceiling system. Under deck insulation system shall be provided on the under side of the roof / floor slab of the air conditioned areas based on the functional requirement. Ceiling of air washer room shall also be provided with under deck insulation.

Aluminum false ceiling system shall comprise of 84 mm wide 12.5 mm 6 deep closed type plain panels of approved colour, roll formed out of 0.5 mm thick corrosion resistant aluminum alloy AA 5050 fixed on roll formed carriers. Additional hangers and height adjustment clips shall be provided for return air grills, supply air diffusers, light fixtures, AC ducts etc.

Gypsum board false ceiling system shall consist of 600x600x12 mm board with one coat of primer and two or more coats of acrylic emulsion paint. The suspension system shall consist of 6 mm dia galvanized steel rods suspended from ceiling supporting aluminum grid of 38x25x1.5 mm and cross tie of 25x25x1.5 mm and aluminum angle of 25x25x1.5 mm.

Suitable M.S channel (minimum ISMC100) grid shall be provided above false ceiling for movement of personnel to facilitate maintenance of lighting fixtures, AC ducts etc.

CONTRACTOR shall prepare a layout of the false ceiling system incorporating light fixtures, supply air diffuser, return air grills, fire protection sprinklers etc. such that the ceiling looks aesthetically pleasing. Work shall commence only after the OWNER approves the layout.

Under deck insulation shall comprise of 50 thick PUF material of density 36 kg/cubic meter . This mat shall be backed with 0.05 mm thick aluminum foil and 24 Gx25 mm wire mesh netting. They shall be fixed to ceiling or wall as the case may be with 100x50x6 slotted mild steel plate welded to M.S. plate inserts embedded at the soffit of the slab at 600 mm c/c and 14 G steel wire drawn through slots and fixed to wire netting.



26.10.00 Painting

Details furnished here in below are the minimum acceptable standard for painting. Superior finish if any required by CONTRACTOR to Enhance overall appearance will be permitted if such finish meet with the technical requirements.

Water proof cement based paint as per IS:5410 shall be provided on external faces of walls, sunshades etc.

Inside surfaces shall be provided with Acrylic washable distemper for all areas as per IS:428 except for Control room, Control equipment rooms, all air conditioned areas for which Acrylic emulsion paint shall be provided as per IS:5411.

Inside surfaces shall be provided with Acrylic distemper as per IS:428 for plant buildings like Workshop, Permanent stores, D.G. house, Compressor house, pump houses, Ash handling pump house etc.

Walls in D.M.Plant shall be provided with chlorinated rubber based paint as per IS : 9862 over walls. Walls above Dado in battery rooms shall also be provided with similar painting.

All plastered ceilings shall be provided with acrylic washable white distemper as per IS:42.

Oil resistant paint as per IS:161 shall be provided for oil canal and oil equipment room.

All wood work shall be provided with fire resistant transparent paint as per IS : 162 over french polish as per IS:348 or flat oil paint as per IS : 137.

Painting for structural steel have been specified else where in this document.

Following general instruction for painting shall be followed.

- i) For painting on concrete, masonry and plastered surfaces IS:2395 parts I and II shall be followed.
- ii) For painting on wood work IS:2338 part I & II shall be followed.
- iii) All paints shall be of brand and make to the approval of OWNER.
- iv) A minimum of two finishing coats of paint over a primer shall be provided to give a smooth uniform finish for the painted surface.
- v) All painting on masonry or concrete surfaces shall preferably be applied by rollers.



- vi) Thinner shall not be used with textured paint (sandtex matt etc) finish.
- vii) All fire exits shall be painted in Post office red colour shade which shall not be used anywhere except to indicate emergency or safety measure.

26.11.00**Sanitary Facilities**

Toilet and shower facilities shall be provided for personnel in finished areas of all major buildings and in other areas as determined by project requirements.

Janitor closets and cleaning material storage areas shall be provided in finished areas of all major buildings.

Minimum one toilet block with all the facilities shall be provided on each floor of main plant building and service building. Separate toilets for ladies shall be provided wherever required. Attached toilets shall be provided for all senior executive rooms and conference rooms. One toilet shall be provided in the boiler area. All other buildings where toilet is specified shall have one toilet block each. The facilities provided in the toilet block shall depend on the number of users. However, minimum facilities to be provided shall be as stipulated below. IS: 1172 shall be followed for working out the basic requirements for Water supply, drainage and sanitation. In addition, IS: 2064 and IS: 2065 shall also be followed.

Each toilet block shall have the following minimum facilities. Unless specified all the fittings shall be of chromium plated brass (fancy type).

- i) WC (Indian type) Orissa pattern (580x440 mm) as per IS: 2556 (Part-3) and 1 no. bibcock with all fittings including flushing arrangement of appropriate capacity and type-1 no.
- ii) WC (western type) 390 mm high as per IS: 2556 (part 2) with toilet paper roll holder and bibcock and all fittings including, flushing system of appropriate capacity and type-1 no.
- iii) Urinal with all fittings with photo voltaic control flushing system as per IS: 2556 (Part-6, Sec. 1) – 2 nos. (Minimum).
- iv) Wash basin (oval shape) with all fittings as per IS: 2556 to be fixed on concrete platform finished with 12 mm thick polished granite stone – 2 nos.
- v) 1200x900x5.5 size Bathroom float glass mirror with beveled edges including all fittings.
- vi) Stainless steel towel rail (600x20 mm) – 2 nos.
- vii) Stainless steel liquid soap holder cum dispenser – 2 nos.
- viii) Provision for installation of water cooler.
- ix) Provision of ventilation shaft.
- x) Attached toilets provided for senior executive rooms and conference rooms shall have 1 WC, 1 Urinal, 1 washbasin, 1 mirror, 1 no. towel rail, 1 liquid soap holder cum dispenser.



WC shall be of western type 390 mm high as per IS: 2556 (Part-2) with toilet paper roll holder and all fittings including flushing valve of appropriate capacity and type. All toilets shall have at least 1 no. of exhaust fan. Final no. of exhaust fans shall be decided as per ventilation requirements to maintain hygienic conditions in the toilet.

- xi) Unless specified all fittings and fixtures in the toilets shall follow technical specification.
- xii) In main Turbine generator building, Service building and Administrative building at least one toilet block shall be provided for ladies.
- xiii) Other buildings which are not specifically mentioned if found necessary shall be provided with adequate toilet facilities in accordance with NBC (National Building Code (latest)). Final requirement shall be as indicated by the owner.

All sanitary fittings and plumbing fixtures shall be of the best quality and make, and shall be subject to Owner's approval. All water supply fittings in toilets, kitchen etc viz., Bib cocks, pillar cocks, P-traps, towel rail, shower rose, gratings etc., shall be of best quality chromium plated brass of approved make.

Supreme B class stoneware pipes or equivalent shall be used for sanitary works above ground level.

26.12.00

Chemical Resistant Treatment Battery Room

- a) Battery room floors and other surfaces as required shall be provided with acid/alkali resistant tiles and treated suitably as detailed below by supplying & furnishing of all labour, materials & equipment in accordance with the following latest codes:
 - 1. IS:3384 : Specification for bitumen primer for use in water proofing & damp proofing
 - 2. IS:4443 : Code of practice for use of resin type chemical resistant mortars
 - 3. IS:4457 : Specification for ceramic unglazed vitreous acid resistant tiles.
 - 4. IS:4832 : Specification for chemical resistant mortars (Part I to III).
 - 5. IS:4971 : Recommendation for selection of Industrial floor finishes.
- b) The surfaces to be treated against acid shall be levelled smooth, dry & clean. The entire surface shall be tapped by wooden hammer to determine hollows if any. In case hollows are found, they shall be suitably sealed with concrete or other approved materials.
- c) The unsatisfactory surfaces shall be replaced by new concrete or other suitable material after ensuring proper bond between the new and old surface. Surface to be prepared shall be wetted for 24 hours; use of epoxy resins for bonding fresh concrete during repairs shall be permitted with Engineer's subject to its execution as per Manufacturer's instructions. All patched areas shall be properly cured by sprinkling water for a period of not less than 10 days.



26.13.00**Acid/Alkali Resistant Tiles on Floors & Walls**

- a.) The surface after preparation shall be applied with a coat of bitumen primer conforming to IS:3384 (latest). The primed surface shall be subsequently applied with a uniform coat of bitumen conforming to IS:1580.
- b.) For bedding of tiles, potassium silicate based cement mortar [15 mm thick] as per IS:4832 shall be used spreading on the back & two adjacent sides of the tile. The tile shall be pressed on the floor or wall & push against floor or wall until the joint in each case of 6 mm thick & 6 mm width is maintained with spacers. Before the bedding mortar sets completely, the jointing material shall be removed to a depth of 20 mm. After the bedding mortar is properly set, the joints shall be cured with resin type mortar filling up the entire length of the joint. The excess mortar shall be trimmed off to make the joints smooth & place. Seal coat shall then be applied over the epoxy mortar to cover up porosity that may be left in mortar. Acid resistant tiles shall be provided for a height of at least upto lintel level on all the walls of battery room. The rest of the battery room walls & ceiling shall be provided with 2 coats chlorinated rubber paint. The tiles used for flooring & walls shall be 20 mm thick conforming to IS 4457. Tiles of approved quality and shade shall be used.

Acid curing shall be carried out as per manufacturer's directions using the safety precautions normally used when handling acids. The mortar joints shall be cured with 20 to 25 percent hydrochloric acid or with 30 to 40 percent sulphuric acid no sooner than 2 days and not later than 6 days after the masonry units are bonded with the mortar. The curing time shall be at least 60 minutes.

Performance guarantee: The contractor shall be responsible for the safety, suitability & efficient functioning of all the acid resistant treatment. He should guarantee for efficient performance of the treatment for a period of five years time from the date of the completion. During the guarantee period, if at any stage, it is found that any of such treatments has given way or has not been functioning satisfactorily, the same shall be made good by the contractor at his own cost.



27.00.00 WATER SUPPLY, DRAINAGE AND SANITATION

27.01.00 The technical details of permanent water supply, storm water drainage, sewerage disposal system and sanitary appliances required for proper supply drainage, disposal and completion of buildings are detailed below:

27.02.00 Sewerage Pipes**a) Glazed Stoneware Pipes**

Glazed stoneware pipes of diameter not exceeding 150 mm can be used in localized areas not subjected to any traffic loads. The glazed stoneware pipes with spigot and socket ends shall conform to IS: 651 Grade “A” or “AA” as specified. These shall be sound, free from visible defects such as fine cracks or hair cracks. The glaze of the pipe shall be free from erasing. The pipes shall give a sharp clear note when struck with a light hammer. There shall be no broken blisters. The pipes shall be handled with sufficient care to avoid damage to them.

b) HDPE soil, waste, vent pipes and fittings

- i) HDPE soil, waste, vent pipes and fittings shall be of “Approved make” conforming to I.S. 4984-1995. The outer and inner surfaces of the pipes and fittings shall be smooth and clean and shall in all respects be free from cracks, pin holes, laps or other imperfections, which may impair the strength and durability of the pipes and fittings. The ends of the pipes shall be reasonably square to their axes.
- ii) The clamps for fixing the pipes to the external wall shall be of approved design and shall fit closely round the pipe or accessory directly beneath the socket.
- iii) The HDPE. soil, waste and vent pipes shall normally be fixed externally to the wall or in suitably designed shafts as per alignment shown on the drawings or as directed by the Engineer. The pipes shall be fixed and kept in position by means of brackets or holder clamps which shall be securely embedded in the wall. For vertical runs, each pipe shall hang freely on its own brackets fixed just below the socket. For horizontal runs, an additional clamp shall be provided at the centre. The brackets or clamps should be of such a design so as to maintain a minimum distance of 50 mm between the surface of the pipe and the wall.



- iv) The soil, waste or vent pipes shall be carried up, above the roof by a least 1.5 m and shall have a cowl of suitable design at the upper end.

c) RCC Pipes

- i) RCC pipes shall be used below ground level for sewage disposal. Pipes connecting toilet facilities to manholes shall be minimum 100 mm diameter NB. Pipes connecting various manholes shall be minimum 150 mm diameter NB. RCC Hume pipes shall be supplied and laid as required as per latest editions of the following IS codes:

IS : 458 : Concrete pipes (with and without reinforcement)

IS : 783 : Code or practice for laying of concrete pipes

IS : 4111(Part-1) : Code of practice for ancillary structure in sewerage system

- ii) The contractor shall supply the RCC hume pipes [to class NP3/NP2 of IS:458 as required] of various diameters along with fittings required for carrying out the work. The contractor should furnish manufacturer's test certificates from recognized authorities. PCC encasement shall also be provided whenever required as per design requirement.

iii) Alignment, Levels and Grade

The work shall be carried out in conformance to the alignments, levels and grades specified. The layout and levels shall be made by the contractor from one reference grid and bench mark given by the Engineer with the assistance of instruments, materials and men for checking the detailed layout and levels as and when required. Making of reference layout and level pillars along the pipeline route and maintaining them upto completion of the work shall be the Contractor's responsibility.

iv) Laying of Pipes

The laying of RCC pipes shall conform to IS:783. To suit the site conditions, the pipes should be laid as per drawings and instructions of the Engineer. Under the culvert condition, the pipe shall be laid under embankment and may project wholly or partly above the original ground surface.

Under trench condition, the pipe shall be laid in the excavated trench which shall be refilled with thoroughly tamped earth after laying and jointing of pipes in an approved manner. Adequate sand fill



cushioning shall be provided below pipes. Minimum depth of cushion shall be 600 mm.

Under open condition, the pipe shall be laid such that it projects wholly or partly above original ground surface, there being no super imposed over burden on the pipe.

v) Bedding and Supports

Under culvert condition, the pipes shall be laid generally on “First Class bedding” as per IS:783. Under trench condition, the pipes shall be laid generally on “First Class Bedding” as per IS:783. If shown on the drawing or so instructed by the Engineer, the pipes shall be laid on concrete cradles, conforming to IS:783. Minimum crushing strength shall be 140 Kg/sqm. Under open condition, the pipes shall be supported over rigid C.C. pedestals constructed at intervals not greater than the length of one individual piece of pipe, as per drawings and instructions of the Engineer. In no case shall the joint between two pieces of pipe shall lie at center of the span between the supports. The pedestal shall be of plain/reinforced concrete with a properly shaped out top to receive the pipe. Manholes shall be provided as per latest IS:4111 (Part-I) for pipe drains at the change of diameter of pipe, alignment and direction at a max. spacing of 30 m c/c upto 900 mm diameter and 45 m c/c for higher diameters of the pipe. Each manhole shall be provided with a heavy duty C.I. cover as per IS:1726.

d) Jointing

i) Glazed Stoneware Pipes

The cement mortar for jointing shall be 1:3 (1 cement: 3 fine sand) with spun yarn in neat cement. Tarred gasket or hemp/ spun yarn soaked in thick cement slurry shall first be placed round the spigot of each pipe and the spigot shall then be placed well into the socket of the pipe previously laid. Then pipe shall be adjusted and fixed in the correct position and gasket/ spun yarn caulked tightly so as to fill not more than $\frac{1}{4}$ th of the total depth of the socket.

The remainder of the socket shall be filled with stiff mixture of cement mortar in the proportion of 1:1 (1 cement: 1 fine sand). When the socket is filled, a fillet shall be formed round the joint with a trowel forming an angle of 45 Degree with the barrel of the pipe. After a day's work any extraneous material shall be removed from the inside of the pipe. The newly made joints shall be cured.



ii) Jointing of RCC pipes

IS : 783 provisions shall be followed for jointing of pipes; and every possible care shall be taken to ensure that the joints made are leak proof. Curing of joints shall be done for a period of 10 days.

e) Testing of Joints**i) Stone Ware Pipes**

Stone ware pipes used for sewers shall be subjected to test pressure of 1.5M head of water at the highest point of the section under test. The test shall be carried out by suitably plugging the low end of the drain and the ends of the connection if any and filling the system with water. A knuckle bend shall be temporarily jointed in at the top end and a sufficient length of vertical pipe jointed to it so as to provide the required test head. Or this may be plugged with a connection to a hose ending in a funnel which could be raised or lowered till the required head is obtained and fixed suitably for observation.

ii) RCC pipes

All RCC pipe joints shall be checked for water tightness as per relevant IS code.

27.03.00**Internal Work**

- a)** All water supply pipes for internal plumbing of buildings shall be of GI pipe of medium class conforming to IS : 1239 of approved make. Galvanizing of pipes shall conform to IS : 4736.

- b)** The pipes and sockets shall be cleanly finished, well galvanized in and out and free from cracks, surface flaws, laminations and other defects; all screw threads shall be clean and well cut. The ends shall be cut cleanly, and square with the axis of the tube. The thickness and weight of pipes and sockets shall be in accordance with IS : 1239.

All screwed tubes and sockets shall have pipe threads conforming to latest IS:554. Screwed tubes shall have taper threads while the sockets shall have parallel threads.

- c)** The pipe fittings shall be of malleable cast iron or mild steel tubes complying with all the appropriate requirements of clause No.3 or as specified. The fittings shall be designated by the respective nominal bores of the pipes for which they are intended. The fittings shall have screw threads at the ends conforming to IS: 554 (latest edition).



Female threads on fittings shall be parallel and male threads (except on running sipples and collars of unions) shall be taper.

d) Cutting, Laying and Jointing

Where the pipes have to be cut or rethreaded, the ends shall be carefully filled out so that no obstruction to bore is offered. The ends of the pipes shall then be threaded conforming to IS: 554 (latest edition) with pipe dies and tapes carefully in such a manner as well not result in slackness of joints when the two pieces are screwed together. The tapes and dies shall be used only for straightening screw threads which have become bent or damaged and shall not be used for turning of the threads so as to make them slack, as the later procedure may not result in a water tight joints. The screw threads of pipes and fittings shall be protected from the damage until they are fitted.

The pipes shall be cleaned and cleared of all foreign matter before being laid. In joining the pipes, the inside of the sockets and the screwed end of the pipes shall be oiled and rubbed over with white lead; and a few turns of spun yarn wrapped round the screwed end of the pipe. The end shall then be screwed in the sockets, tee etc. with the pipe wrench. Care should be taken that all the pipes and fittings are properly jointed so as to make the joints completely water-tight; and pipes are kept at all times free from dust and dirt during fixing. Bore from the joint shall be removed after screwing. After laying, the open ends of the pipes shall be temporarily plugged to prevent access of water, soil or any other foreign matter.

Any threads exposed after joining shall be painted or in case of underground piping thickly coated with approved anticorrosive paint to prevent corrosion.

e) Fitting

- (i) For internal work, the galvanized pipes fittings shall be done by means of standard pattern holder bat clamps, keeping the pipes about 1.5 cm. clear of the wall. When it is found necessary to conceal the pipes chasing may be adopted or pipes fixed in the ducts or recesses etc. provided there is sufficient space to work on the pipes with the usual tools. The pipes shall not be buried in walls or solid floors, where unavoidable pipes may be buried for short distances provided adequate protection is given against damage and where so required joints are not buried. Where directed by the Engineer, a steel tube sleeve shall be fixed at a place a pipe is passing through a wall or floor for reception of the pipe and to allow freedom for expansion and contraction and other movements. In case the pipe is embedded in wall or floors, it



should be painted with anticorrosive bitumastic paint of approved quality. The pipe should not come in contact with lime mortar or lime concrete as the pipe is affected by lime. Under the floors the pipes shall be laid in layer of sand filling done under concrete floors.

- ii) All pipes and fittings shall be fixed truly vertical and horizontal unless unavoidable. The pipes shall be fixed to walls with standard pattern holder bat clamps of required shape and size so as to fit tightly on the pipes when tightened with screw bolts. The clamps shall be embedded in brick work in cement mortar 1:3 (1 cement : 3 coarse sand) and shall be spaced at regular intervals in straight lengths as indicated below :-

Size of pipe (mm)	LENGTHS	
	Horizontal runs (m)	Vertical runs (m)
15	2	2.5
20	2.5	3
25	2.5	3
32	2.5	3
40	3	3.5
50	3.5	3.5
65	3.5	5
80	3.5	5

The clamps shall be fixed at shorter lengths near the fittings as directed by the Engineer.

- iii) For G.I. Pipes 15mm to 25 mm dia, the holes in the walls and floors shall be made by drilling with chisel or jumper and not by dismantling the brick work or concrete. However, for bigger dimension pipes, the holes shall be carefully made of the smallest size as directed by the Engineer. After fixing the pipes, the holes shall be made good with cement mortar 1:3 (1 cement and 3 coarse sand) and properly finished to match the adjacent surface.

27.04.00

Trenches and Other Excavations

2 x 660 MW Ennore SEZ Coal Based Supercritical Thermal Power
Project at Ash Dyke of NCTPS
Spec. No. CE/C/ P & E/ EE/ E/OT. No.03 /2013-14
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Width of the trench at the bottom shall be such as to provide 200 mm clearance on either side of the pipe for facility of laying and jointing.

Excavated material shall be stacked sufficiently away from the edge of the trench and the side of the spoil bank shall not be allowed to endanger the stability of the excavation. Spoil may be carted away and used for filling the trench behind the work.

Turf, top soil or other surface material shall be set aside, turf being carefully rolled and stacked for use in reinstatement.

All excavation shall be properly timbered, where necessary.

Efficient arrangements for dewatering during excavation and keeping it dry till backfilling shall be made to the satisfaction of the Engineer. Sumps for dewatering shall be located away from the pipe layout.

Where the excavation proceeds through roads necessary permissions shall be secured by the Contractors from the appropriate authorities.

Special care shall be taken not to damage underground services, cables etc. These when exposed shall be kept adequately supported till the trench is backfilled.

The backfilling shall be done only after the pipeline has been tested and approved by the Engineer. Special care shall be taken under and sides of the pipe during handpacking with selected material. At least 300 mm over the pipe shall also be filled with soft earth or sand. Consolidation shall be done in 150 mm layers. The surface water shall be prevented from getting into the filled up trench. Traffic shall not be inconvenienced by heaping up unduly the backfilling material to compensate future settlement. All future settlements shall be made good regularly to minimise inconvenience of traffic where applicable.

27.05.00

Sanitary Appliances

a) Water Closet

- (i) **Water closet Pan** : The water closet shall be of Orissa pan/European type of approved make, Vitreous China conforming to IS: 2556 Part- III. The closet shall be of size as specified. The flushing rim shall be integral with the pan and shall be Self draining type. The closet shall be provided with a self cleaning trap of the same material as the pan. The trap shall be either a “P” or “S” outlet and with a vent hole. The effective depth of the water Seal shall not be less than 50 mm.



- (ii) The pan shall be fixed in position as shown on the drawing. It shall be embedded on a suitably prepared surface rendered smooth with neat cement or cement plaster so that uniform support is provided to the pan. The pan shall normally be at the floor level unless otherwise specified. The WC pan shall be fixed to the trap by means of a cement mortar joint as in the case of SW pipes; and the entire job carried out so that the effective seal is always kept maintained the vent of the trap being connected to Anti siphonage pipe.
- (iii) While laying flooring, a section about 1 x 1.5 m or less of the part of the floor where WC is to be fixed shall be left out. This shall be finished only after the pan has been fixed and laid in position. The pan shall be kept suitably protected against all damages. It shall include both pan and trap, as well as fixing materials as needed.

b) Flushing Cisterns for Water Closets

The flushing cistern for water closet shall be PVC low level type of 10 Ltr. capacity (IS 2556 mark) with PVC bend of approved make. The cistern shall be secured on wall or supported on steel brackets which shall be well embedded in the wall. The height of the bottom of the cistern from the top of the pan shall be 30 cm for low level cistern. The cistern shall be provided with brass balcock with ball to IS:1703. It shall include brackets, screw, flush pipes, brass ball cock with ball as needed.

- c) The Urinal shall be of first quality, WVC flat back (Large) or half shall type IS 2556 of size as specified of approved make. The urinal shall be fixed in position by means of four plugs embedded in wall and screws, the top of urinal lip being 65 cm from the floor level. Urinal bring a soil fitment should be connected to soil pipe. Each urinal shall be provided with 25 mm dia B Class GI Waste pipe connected to the pan by suitable joint and shall discharge in to a drain connected to a soil pipe through a trap. The flushing of urinal shall be with all fittings including voltaic control flushing system as per IS:2556 (Part-6 Section-1).
- d) The wash basin (oval shape) with all fittings as per IS:2556 made of vitreous China clay to be fixed on a concrete plat form finished with 12 mm thick polished granite stone. The basin shall be provided with single tap connection. The washbasin shall have an integral soap holder recess or recesses which shall drain into the bowl.

Each Wash Basin shall be provided with :



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- i) 32 mm \varnothing CP brass waste fitting with CP brass chain and 32 mm dia plug.
- ii) 25 mm \varnothing galvanized waste pipe “B” class discharging into a drain suitably connected to waste fitting by means of brass check nuts.
- iii) 15 mm \varnothing CP pillar taps of superior quality to IS-8994 of approved make.

The basin shall be connected to the waste pipe discharge in to drain, by means of the waste fittings and check nuts, the join being completely leak proof. The tap shall be connected to the water supply system by means of a lead connection. A stop cock shall be provided to regulate supply to the wash basin. It shall include fixing material complete with brackets, waste coupling, waste pipe, NP pillar tap, CC chamber and grating.

- e) Wall to wall bath room mirror (5.5MM thick) float glass with beveled edges including all fittings shall be provided.
- f) All toilets shall have at least one No. of exhaust fan. The No. of exhaust fan shall be decided as per ventilation requirements to maintain the high hygienic conditions in the toilet.



28.00.00**SWITCHYARD CIVIL WORKS****28.01.00**

The structural and civil works associated with erection and commissioning of station switchyard shall be executed as detailed below. The scope covers design, engineering, fabrication, galvanizing, supply and erection of:

- a) Lighting mast, towers, beams, equipment lattice support, equipment pipe support inclusive of all fixtures such as nuts, bolts, hangers, shackles, clamps, anti-climbing devices, bird guards, step bolts/ladders, number plates, danger and phase plates, inserts in concrete, foundation bolts, base plates, stiffeners, dampers, fixtures for supporting and operating mechanism boxes, control cubicles etc.
- b) Framed RCC switch yard building with brick cladding shall be provided to accommodate various equipments including GIS as per electrical drawing. Roof shall be of RCC. Size of building shall meet electrical requirements.
- c) Cable tray supports, cable trays, embedments in cable trenches, bolts and nuts and any other accessories etc.
- d) R.C.C. cable trench and pipe trenches with necessary precast R.C.C. covers with lifting facilities, sump pit, cable trays, supports etc. [wherever trenches cross road, the section below such crossings shall be designed as per Indian Road Congress Drainage of these trenches shall be suitable designed]. All precast slabs for trenches shall be provided with MS edge protection angle on all sides (min. size of angle 50x50x6 mm). The RCC trench edges shall be provided with MS angle protection (min. size of angle 50x50x6 mm)
- e) Drainage and lighting of the area [cable trench should not be used for drainage purpose]. An approved system for draining the transformer and oil collection, and disposal system shall be provided.
- f) For prevention of vegetation, the graded ground shall be covered with fly ash brick pavement over 300 mm ash filling is to be provided in the switchyard area. Each fly ash layer shall be compacted/ consolidated by using ½ ton roller with 4 to 5 passes and suitable water sprinkling. The method of application of chemicals used for soil sterilization/anti-weed treatment shall be as per manufacturer's recommendations. Also, the RCC pathway shall be provided in switchyard as per the requirement.



- g) Transformer foundations; RCC roads within switchyard, fire protection walls and chain link fencing for switchyard. RCC paving shall be provided in Transformer yard.

28.02.00**General Requirements****28.02.01**

Steel shall conform to IS:226/IS2062. Where plate cut pieces are used to form a part of either flange plates or web plates of fabricated columns with butt joints, length of such pieces shall not be less than 3 (three) meters. When cut pieces out of rolled sections are to be joined by butt welding, the length of such pieces shall not be less than 3 (three) meters and the total number of pieces of any member shall not exceed three. The steel structures for the switchyard shall be double dip galvanized with zinc coating not less than 0.91 kg/m².

All foundations for the switchyard structures (except transformer) shall be designed as per relevant IS:4091 “Code of practice for design and construction of foundations for transmission line towers and poles” and IS-456 (latest) “Code of practice for plain and reinforced concrete”. All the foundations shall be protected by as per clause 6.11 of this specification.

28.02.02**Transformer/shunt reactor foundations**

The transformer/shunt reactor foundations shall be designed as per relevant IS Code. This shall include collection of oil, drainage of oil along with rain water collected in such foundation; as well as necessary cable connection works.

28.02.03

- a) The steel girders shall be designed for static tension dead load of conductors, insulators, and accessories, erection load, short circuit forces, wind/seismic loads and secondary stresses. The girders shall be connected with lattice columns by bolted joints.
- b) The fabrication and erection of the works shall be carried out generally as per IS:802 Part-II, IS:800. All materials shall be completely shop fabricated. Towers, beams etc. shall be trial assembled at shop keeping in view the actual site condition for convenient pre-assembly during erection. Necessary match marks shall be made on these components before dismantling in the shop.

28.02.04**Bolting**

Every bolt shall be provided with a steel washer under the nut so that no part of the threaded portion of the bolt is within the thickness of the parts bolted together. Notwithstanding anything to the contrary contained in IS:1363, IS:1364 and IS:1367 the unthreaded length of



the bolt shank shall be equal to total thickness of metal being bolted together plus 2 mm. The threaded length shall be equal to at least the diameter of bolt plus 6 mm.

- a) Cable and pipe trenches shall be in R.C.C. with removable R.C.C. covers. (The top of trench shall be kept 150 mm above the grade level). The bed of trenches shall be slope 1 in 200 to drain water into a collection sump suitably. All trenches shall be given a slope of 1 in 500 along run of the trench. Cable trenches crossing the road shall be designed for class "AA" loading as per IRC standard. However, the design shall also be checked for class "A" loading as per IRC standard. All precast slabs for trenches shall be provided with MS edge protection angle on all sides (min. size of angle 50x50x6 mm). The RCC trench edges shall be provided with MS angle protection (min. size of angle 50x50x6 mm) Surcharge load of 2T /sqm. Shall be considered in design of trenches in switchyard area.
- b) The design of foundation and all trenches should take care of sub-soil water pressure as per relevant IS Codes. If the ground water table is above the base level of the trench, PVC water stops (minimum 230 mm wide and 5 mm thick) should be provided at all construction and expansion joints to the approval of the Engineer.

28.02.05

All anchors, anchor bolts, insert pipes, conduits, sleeves, bolts, etc. required to be embedded in concrete shall be placed in position before concreting. These inserts shall be welded to the nearest reinforcement to keep them in position.

28.02.06

- a) The fence around switchyard area shall comprise of 12 gauge G.I. chain link fencing of mesh size 75 mm to a height of 2.4 m above the toe wall with a 600 mm high galvanized concertina at the top such that total fence height of 3.0m above toe wall level is achieved. Toe wall shall be minimum 200 mm above the formation level.
- b) The chainlink fencing shall comprise of size 2400x2000 mm size panel made from 75x75mm & 8 gauge GI chainlinks (including PVC coatings) and 50x50x6 mm angle including fixing the chainlink in angle frame by means of 40x3mm. MS flat welded to angle frame drilling to holes in angle frame & flat and fixing the same with angle post in the level and plumb by means of 10 mm dia bolts frames and flat & nuts, 3 coats of epoxy base paint to achieve required 250 DT to angle iron etc. completed.

This chainlink fencing panel shall be fixed with the fence posts of 65x65x6 MS angles spaced as per requirement.



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- c) Above the chain link fencing panel 3 horizontal rows of barbed wire of gauge 12/14 with 3 diagonal rows shall be fixed as per drawing.
- d) All fence posts shall be 65 x 65 x 6 MS angles spaced at 2.5 m c/c distance. All straining posts i.e. end posts shall be 65 x 65 x 6 angles. Suitable concrete foundations for the angle iron posts and stays shall be provided based on the prevailing soil conditions. Toe walls either of bricks of minimum 50kg/cm² compressive shall be provided between the fence posts all along the run of the fence with suitable foundation. Toe wall shall be minimum 200 mm above the switchyard formation level with PCC coping (M-20) and minimum 300 mm below the switchyard formation level. All gates shall be of hot dipped galvanized structural steel as specified.
- e) All structural steel work for the fencing shall be painted with chlorinated rubber paint over a suitable primer or epoxy based paint (of total DFT 250 microns) to withstand severe marine/corrosive exposure condition for protection.



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29.00.00

GALVANIZING

- a) Structural steel works and pipe supports wherever required other than that of fencing shall be hot double dip galvanized after fabrication as per relevant IS codes. Galvanising of the towers shall be as per IS:4759 and 2633 and as given in the following paras. Zinc required for galvanizing shall be arranged by the contractor. Purity of zinc to used for galvanizing shall be 99.5% as per IS:209. All burrs and irregular edges shall be ground smooth before galvanizing.
- b) After completion of all shop work, structural material shall be punched with the erection marks and be hot dip galvanized including the portion to be embedded in concrete. Each grillage member including stubs shall also be fully galvanized. Before galvanizing, the steel section shall be thoroughly cleaned of any paint, grease, rust, scale, acid, alkali or such other foreign matters likely to interfere with the galvanizing process or with the quality and durability of the zinc coating. Pickling shall be carefully done and shall be proper.
- c) The weight of the zinc coating shall be at least 0.9 kg/ sq.m including stub members. The galvanized surfaces shall consist of a continuous and uniformly thick coating of zinc, firmly adhering to the surface of steel. The finished surface shall be clean and smooth and shall be free from defects like discoloured patches, bare spots, unevenness of coating, spelter which is loosely attached to the steel, globules, spiky deposits, blistered surface, flaking or peeling off etc. The presence of any of these defects noticed on visual or microscopic inspection shall render the material liable to rejection.
- d) There shall be no flaking or loosening when struck squarely with a chisel faced hammer. The galvanized steel member shall withstand in copper sulphate solution as per IS:2633. When the steel section is removed from the galvanizing kettle, excess spelter shall be removed by 'bumpng'. The process known as 'wiping' or 'scrapping' shall not be used for this purpose.
- All bolts, nuts, washers etc. shall be hot dip galvanized. Excess spelter from bolts, nuts etc. shall be removed by centrifugal spinning of bolts and nuts. Threading after galvanizing, shall not be permitted. Nuts, however, may be tapped, but not to cause appreciable racking of the nuts on the bolts. Spring washers shall be electro-galvanised.
- e) Defects in certain members indicating presence of impurities in the galvanizing bath in quantities larger than that permitted by the specification or lack of quality control in any manner in the galvanizing plant shall render the entire production in the relevant shift liable to



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rejection. All the towers, pipe supports and accessories shall be treated with sodium dichromate or an approved equivalent solution after galvanizing, so as to prevent white storage stains.

- f) Similar If the galvanizing of any member is damaged, the galvanizing shall be redone in the manner as stated above to Engineer's approval. The contractor shall also furnish sufficient quantity of appropriate paint, for repairing galvanized surfaces, damaged in transit.



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30.00.00**PILING****30.01.00**

The detailed design, preparation of construction drawings, installation and testing of piles forming foundations to buildings and structures are detailed below conforming to the latest IS:2911 – Code of Practice for Design and Construction of Pile Foundations : Part I Concrete Piles and IS:2911 Part : IV Load test on piles and to this Specification. The contractor shall be responsible for all aspects of the pile performance installed including demonstration of the adequacy of his design by testing. During detailed design the contractor shall submit the proposed firm to undertake piling work with details piling system, method of installation, summary of design basis, number, type and size of construction plant items to be employed for the work including crane, piling equipment, concreting equipment and proposal for installation & testing of trial piles and tests on work piles.

30.02.00

Prior to installation of trial piles, the contractor shall submit final details of the trial piling installation including specifications, detailed design calculations and construction drawings for the trial piling together with associated equipment to be provided for construction and testing of the same. Design report discussing the overall results of the tests on trial piles and concluding with and justifying the proposed definition of reference piles; specification, with fully detailed construction drawings, supporting calculations for the final design of the working piles, and the field control procedures the contractor intends to use to verify that pile installation satisfies the requirements of his design should also be submitted.

30.03.00**Design Criteria**

The following shall be the design criteria for the working piles :

- a) Piles shall be designed in line with approved soil investigation report as single vertical piles acting under a design load as deemed fit.
- b) Driving formula (where relevant) may be adopted inter alia as a criteria for acceptance. But the minimum founding requirements shall be based on recognized methods of soil mechanics design, appropriate to the type of pile.
- c) The piles shall be designed for a minimum factor of safety of 2.5 against failure under the design load specified.
- d) Piles shall be designed for a limiting settlement of a single pile under vertical design load of 8 mm at the head.



- e) Piles shall be designed to resist a maximum force due to critical load combination in any direction applied to the head of the pile at cut-off level. For this design condition it shall be assumed that the pile head is effectively as a fixed-end by the pile-cap.

30.04.00**Tolerance**

Setting out shall be carried out from the main grid lines of the proposed structure. Immediately before installation of the pile, the pile position shall be marked with suitable identifiable pins or markers. For a pile head cut-off, at or above ground level, the maximum permitted deviation of the pile head cut off center from the center point, shown on the setting out drawing shall be 75 mm in any direction. For a pile head cut-off below ground level, an additional tolerance will be permitted as below on the assumption that the pile head would have been within tolerance if the cut-off had been at ground level. The maximum permitted deviation of the finished pile from the vertical is 1 in 75. Forceful corrections to concrete piles shall not be made.

30.05.00

The contractor shall carry out the work to minimize noise and disturbance.

If during the execution of the work, damage is likely to be, caused to mains, services or adjacent structures, the contractor shall submit proposal for repair or avoidance of such damage. The contractor shall ensure that damage does not occur to completed piles.

30.06.00**Records**

The contractor shall keep records as below of the installation of each pile and shall submit record to the Engineer.

Table 1.0

a)	Piles reference number (location)	*
b)	Piles cut off level	*
c)	Pile type	*
d)	Nominal cross-sectional dimensions or diameter	*
e)	Pile design capacity	*
f)	Standing groundwater level	*
g)	Date and time of driving, red riving or boring	*
h)	Date of concreting	*
i)	Ground level at commencement of installation of piles	*
j)	Working level	*
k)	Depth from working level to pile toe	*
l)	Toe level	*
m)	Depth from working level to pile head level	*
n)	Length of temporary / permanent casing	*
o)	Type weight, drop and mechanical condition of hammer	*



p)	Number of type of packings used and type and condition of	*
q)	Record of blows per 300 mm over complete drive	*
r)	Set of pile or pile tube in mm per 10 blows or number of	*
s)	Temporary compression of ground and pile from time of	*
t)	Length and details of reinforcement	*
u)	Grade and slump of Concrete mix and control test	*
v)	Volume of concrete installed in pile	*
w)	Method of placing concrete	*
x)	Number of blows to form bulb where appropriate	*
y)	Level of water or drilling fluid at commencement of	*
z)	All information regarding obstructions delays and other	*

30.07.00**Driven Cast-in-situ Concrete Piles**

- a) Installation of piles shall be carried out as per layout construction drawings in accordance with latest IS codes as below:
- i) IS:456-2000 : Code of practice for plain and Reinforced Concrete
 - ii) IS:2911-Part I : Code of Practice for Design and Section-I Construction of pipe foundation
 - iii) IS:2911-Part IV : Test load on piles
- b) The sequence of installation of piles shall be to the approval of the Engineer.
- c) The equipment and accessories required for the pile driving shall be standard type. The capacity of the pile frame shall be selected after considering the size and weight of the casing to be handled, the location of the work and the hardness of driving. The steel casing used for installing the piles shall be straight and shall have sufficient wall thickness and strength to prevent damage, distortion etc. Detachable shoes used with the temporary steel casing shall be fabricated steel or cast iron, double rimmed and of requisite strength, and of the standard sizes used with the respective steel casing. The shoe shall be co-axial with the steel casing. Casings distorted from true uniform shape whether caused by driving or earth and water pressure resulting in a reduction of the cross sectional area of the pile in excess of 10 percent shall not be used in the works. The nominal casing size shall be 500 mm diameter.



- d) The pile shall be driven to a set proposed by the contractor. The final set values shall be repeated thrice before terminating the pile at that level. All relevant data with back up, computations pertaining to “set” shall be submitted before commencing piling work for Purchasers approval. Any sudden change in rate of penetration shall be brought to the notice of the Engineer.

e) **Control of Pile Driving**

Protection of pile heads and selection of pile hammer shall be as per IS:2911 Part I. The pile shall be driven as accurately as possible to the vertical. Piles shall be installed with due consideration for the safety of adjacent structures by a method leaving their strength unimpaired and which develops and retains the required load bearing capacity. In the case of piles close to existing plant and structures, the Engineer may specify suitable precautions which may call for additional working hours for the rig. The quipment and methods for installing piles shall be such that the loss of energy is limited to a minimum. Should the deviation of the installed pile be greater than the specified tolerances, the Engineer may at his discretion order the installation of additional piles after checking the design load on the pile group of which the particular pile forms part. Care shall be taken not to damage the piles by over driving. Any sudden change in the rate of penetration shall be noted and informed to the Engineer. Jetting of Pile by means of water shall not be permitted.

- f) After complete installation of the casing before concreting, the contractor shall inform the Engineer to inspect the casing for proper plumb location and other conditions. Only after approval concreting should proceed. Level readings shall be taken on each pile after driving and again after all the piles in a group are driven. Piles which are found to have resin shall be driven to the original depth and set.

- g) The main longitudinal reinforcing bars in piles shall be in one continuous length. In long piles, joints shall be permitted in main longitudinal bars but these shall be kept to a minimum. Joints in adjacent bars shall be staggered at least 1 meter part along the length of the pile. Joints in reinforcement shall be such that the full strength of the bar is effective across the joint.

Longitudinal reinforcement shall be provided for the full length of the pile extending above pile cut off level to provide adequate bond length into the pile cap. The minimum area of longitudinal reinforcement within the pile shaft shall be 0.4 percent of cross-sectional area of the pile on the basis of the nominal casing diameter. Longitudinal reinforcement shall be formed into a rigid cage to resist deformation during handling and installation by the use of links or helical reinforcement. The minimum diameter of links



and helical reinforcement shall be 6 mm spaced at 150 mm and not more than 300 mm. The longitudinal reinforcement shall project 52 times its diameter; above the cut off level of the pile to ensure adequate bond length inside the pipe cap. Minimum 6 Nos. of longitudinal reinforcement shall be provided. Reinforcement provided in pile shall be symmetrical about any axis on the pile cross section. The diameter of longitudinal reinforcement shall not be less than 12 mm. Lap or splice joints shall be provided with sufficient link bars to resist eccentric forces. Minimum cover to main reinforcement in the pile shall be 75mm.

- h)** The method of placing and the workability of the concrete shall be such that continuous monolithic concrete shaft of the full cross-section is formed. Mechanical vibrators shall not be used to compact concrete. Concrete in piles shall be of design- mix with minimum grade M25 with minimum cement of 400 kg/cum water cement ratio within 0.45 to 0.55 using super plasticized Sikament 170 or equivalent. Generally the slump of the concrete shall be within the range 100 to 150 mm. The contractor shall ensure the minimum specified strength in accordance with IS:456, IS:10262 & SP. 23 shall be followed as guidance for mix design. Compaction of test cubes shall be done under same condition as followed for concreting of piles.

- i)** In soft clays or loose sands ,or where there is likelihood of danger to green concrete due to driving of adjacent piles, the designed reinforcement should be provided up to the full pile depth, regardless of whether or not it is required from uplift and lateral load considerations. Refer clause 5.11.2 of IS : 2911 (Part I / Sec-1)

j) Cleaning out of casing

Before placement of concrete ,it shall be ensured that the inside of the casing is free from sludge or any foreign matter. The contractor shall ensure that water does not enter the casing tube/pile shoe at the end of driving & during the concreting operations. The existence of water inside casing tube after driving and before executing the concrete operation, if any shall be checked by the contractor by suitable probe & concreting of pile work shall be allowed to proceed if water level is less than 15 cm measured above the inner surface at the top of the shoe. If the water level is more than 15 cm, contractor shall take remedial measures viz. bailing or pumping to reduce the depth of water before concreting & if it is not possible to do so then the contractor shall re-drive the casing at no extra cost to the Purchaser.

k) Concrete Level – finishing of pile head

The cut-off level shall be as shown on the construction drawings. Concrete shall be cast to minimum of 500 mm above the cut-off level, or local ground water level, whichever is higher. Withdrawal of temporary casing



shall be carried out in stages during the concrete placing operation with utmost care maintaining an adequate head of concrete above the bottom of casing at each stage of withdrawal so as to prevent the inflow of soil and water into the bore. Section of empty hole remaining above the head of the pile shall be temporarily backfilled with sand or other approved material prior to excavation for pile cap construction.

I) Trimming of pile heads

Following excavation subsequently for pile cap construction, the contractor shall carefully remove excess concrete above specified cut-off level, without damaging the remainder of the pile including the projection reinforcement. Any cracked or defective concrete occurring below specified cut-off level shall be cut away and made good with new concrete properly bonded to the old. Piles that are defective shall be pulled out or left in place as judged by Engineer without affecting performance of adjacent piles. Also the contractor shall be bound to install additional piles to substitute the defective ones as per the directions of the Engineer.

- m)** The dynamic cone penetration tests shall be carried out for a depth of at least 2 meters from the termination depth. The number of such tests shall be as follows
- a. One number dynamic cone penetration test for each pile group consisting of more than four piles.
 - b. For TG and Chimney pile groups the number of dynamic cone penetration tests shall be one per cent of the total number of piles.

30.08.00

Load Tests on Piles

- a)** Load test shall be in accordance with IS:2911 Part IV (latest revision) Initial load tests on "Trial Piles" shall be executed in advance of works pile construction and shall comprise vertical load tests at locations as directed. Load test on trial piles shall also include lateral load test and uplift load tests as directed by the Engineer. 6 number of trial piles shall be tested from each capacity of the pile. Out of the six (6) nos. of piles to be tested for two (2) nos. each capacity shall be tested for vertical load, two (2) nos. for lateral load and two (2) nos. for uplift load.

Routine load tests on "Works Piles" shall be vertical load test and lateral load tests on piles as directed by the Engineer. Number of routine tests to be conducted on work piles shall be equal to one and half (1.5) percent of the total number of piles used in the work for vertical load tests as well as lateral load tests.



- b) Load tests shall not normally be undertaken by the contractor within 28 days after the installation of the piles in position. However, the contractor may be allowed by the Engineer to use rapid hardening cement or add necessary admixtures to enable him to conduct the tests within a lesser period. Before any load test is done, the proposed arrangement for carrying out the load tests including the preparation of the structure to receive the loads and the type of loading to be adopted shall be furnished by the contractor.

c) **Vertical Load Test on Trial Piles**

Load tests to verify the contractor's design shall be carried out on piles of different capacities proposed to be used. The contractor shall commence testing as early as possible after installing the piles. The test shall be carried out by applying series of loads on RC cap over single pile. The load shall be applied by means of standard hydraulic jack reacting against a loaded platform which shall be preloaded platform which shall be preloaded to three times the estimated safe load carrying capacity of the pile. Reading of settlement and rebound shall be recorded with four dial gauges of 0.01 mm sensitively and resting on diametrically opposite ends of the pile cap. The dial gauge shall be fixed to a datum bar whose ends rest-upon non-movable supports. The supports for datum bar with reference to which the settlement of the pile is measured shall be at least 5 times diameter of pile away from the pile.

The test shall be conducted by the cyclic loading method. The test load shall be applied in equal increments of about one-fifth of the estimated safe load. Each stage of loading or unloading shall be maintained till the rate of movement of the pile cap is not more than 0.02 mm per hour. However, load shall be maintained for longer periods at 1 to 1.5 times the assumed safe capacity, and at final loads as directed by Engineer. The safe load on pile shall be the least of the following :

- i Two third of the final load at which the total settlement attains the value of 12 mm.
- ii Two third of the final load at which the net settlement attains a value of 6 mm.
- iii 50 per cent of the final load at which the total settlement equals one-tenth of the pile diameter.

d) **Lateral Load Test on Trial Piles**

The test pile shall be decided by the Engineer. The test pile shall be cut off at the proper level and provided with a cap with vertical plane sides having



an adequate area for proper seating of the jack and dial gauge. The lateral load shall be applied on the pile at or approximately at cut off level and the deflection shall be measured at a point diametrically opposite to the point of load application. The loading shall be applied by the hydraulic jack of adequate capacity, abutting the pile horizontally and reacting against a suitable system. The reaction may be provided by the well of the excavated pit when the test is being conducted below ground level or by a neighbouring pile in which case thrust pieces shall be inserted on either end of the jack to make up the gap. Lateral load applied on the pile shall be measured by a calibrated pressure gauge mounted on the jack, having a least count of 500 kg. Deflection of the pile head shall be measured by four dial gauges, fixed to datum bars & having a least count of 0.01 mm. The datum bars shall be provided with rigid supports. Loading shall be applied in increments of 500 kg. Each stage shall be maintained for a period till the rate of movement of the pile head is not more than 0.02 mm/hour or 1 hour whichever is greater .

Loading shall be continued till one of the following occurs :

- i. Deflection of pile head exceeds 12 mm.
- ii. The applied load on the pile is three times the assumed lateral load capacity of the pile.

The safe load shall be smaller of the following :

- i. Half the final load for which the total deflection is 12 mm.
- ii. Load at which the total displacement corresponds to 5 mm.

e) Pull-out Test

The test piles shall be decided by the Engineer. The test pile shall be built upto the proper length and the head provided with suitable arrangements for anchoring the load applying system. Loads shall be applied using a approved reaction system, uplift forces on the pile shall be applied directly to the test pile or through a lever system. The reaction shall be provided by neighbouring piles or blocks may be constructed for the purpose. Hydraulic jacks shall be used for load application. Load applied by the hydraulic jack shall be measured by a calibrated gauge with a least count of 100 Kg. Movement of the pile shall be measured by dial gauges, fixed to datum bar and having a least count of 0.01 mm. Four dial gauges placed diametrically opposite shall be used. Datum bars shall be provided with rigid supports. The load shall be applied along the longitudinal axis of the pile using method approved by the Engineer. Loading shall be applied to the pile top in increment of one-fifth of the expected safe capacity. Each stage shall be



maintained for a period till the rate of movement of the pile head is not more than 0.02 mm per hour or four hours whichever is greater. Loading shall be continued till one of the following occurs :

- i. Yield of soil pile system occurs causing progressive movement of pile exceeding 12 mm.
- ii. The loading on the pile top equals three times the estimated safe load or as specified in the case of separate test pile.

The safe capacity of the pile should be least of the following :

- i. Two third of the load at which the total displacement is 12 mm or the load corresponding to a specified permissible uplift, and
- ii Half of the load at which the load displacement curve shows a clear break (downward trend)

f) Routine Test on Working Piles

Three tests shall be carried out on working piles upto one and half times the design load and the maximum settlement of test loading in position being not exceeding 12 mm as per IS:2911 (Part IV). In case a pile fails under or during the load test, the Engineer shall select two additional piles in lieu of each of such failed pile and the contractor shall carryout load tests on such additional piles.

30.09.00

Cast-In-Situ Piling Bored

The following specification deals with the requirements of materials, workmanship and installation of Bore Cast-in-situ reinforced concrete piles.

The materials and workmanship shall conform with the provision of the latest IS:456, IS:2911 (Part I, Section 2) & IS:2911 (Part 4).

Care shall be taken while boring piles so that no existing foundations / sub-structures are disturbed by providing suitable arrangements like shoring etc.

a) Sequence of Piling

The sequence of piles installed shall be such that the adjacent piles already installed are not disturbed nor their carrying capacity reduced by subsequent boring operation. In a group the installation of the piles shall normally proceed from the center of the group towards the periphery.

b) Control of Alignment



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Piles shall be installed as accurately as possible as per the designs and drawings either vertically or to the specified batter. Great care should be exercised in respect of installation of single pile or piles in two pile group. For vertical piles a deviation of 1.5 percent should not normally be exceeded. Piles should not deviate more than 75 mm or D/10 whichever is more in case of piles having diameter more than 600 mm from their designed positions at the working level of the piling rig. In the case of a single pile in a column, positional tolerance should not be more than 50 mm (100 mm in case of piles having diameter more than 600 mm).

For piles carried to substantial depth, the design should provide for the worst combination of the above tolerance in position and inclination. In case of piles deviating beyond these limits and to such an extent that the resulting eccentricity cannot be taken care of by a redesign of the pile cap and ties, the piles shall be replaced or supplemented by one or more additional piles, as directed by the Engineer, at no extra cost.

Any deviation from the designed location, alignment or load capacity of any piles shall be noted and brought to the attention of the Engineer and rectification and supplemental works carried out to the satisfaction of the Engineer at no extra cost.

A minimum length of one meter of temporary casing shall be inserted in each bored pile unless otherwise specifically desired. Additional length of temporary casing may be used depending on the condition of the strata etc.

Drilling mud of suitable consistency may also be used instead of temporary casings for stabilizing sides of the holes.

In case, a bored pile is stabilized by drilling mud the bottom of the hole shall be cleaned very carefully before concreting work is taken upon. The cleaning of the hole shall be ensured by careful operation of boring tool and / or flushing of the drilling mud through the bottom of the hole. Flushing of bore holes before concreting with fresh drilling fluid/ mud is preferred.

In case a holes is bored by use of drilling mud, the specific gravity of the mud suspension near about the bottom of the hole shall, wherever practicable, be determined by suitable interval of piles and recorded. Consistency of the drilling mus suspension shall be controlled throughout the boring as well as concreting operations in order to keep the hole stabilized as well as to avoid concrete getting mixed up with the thicker suspension of the mud.

The concreting operatings should not be taken up when the specific gravity of bottom slurry is more than 1.2. Concreting shall be done by tremie method in all such cases.



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The top of concrete in a pile shall be brought above the cut-off level as per cl. 7.8 of IS: 2911 (Part 1/sec.2).

In case, defective piles are formed, they shall be removed or left in place, if so directed by the Engineer, if these do not affect the performance of the adjacent piles or the cap as a whole. Additional piles shall be provided to replace them as directed by the Engineer.

Any deviation from the designed location alignment or load capacity of any pile shall be noted and adequate measures taken well before the concreting, of the pile cap and plinth beam if the deviations are beyond the permissible limit.

During chipping of the pile top manual chipping may be permitted after three days of pile casting, pneumatic tools for chipping shall not be used before seven days after pile casting.

After concreting the actual quantity of concrete shall be compared with the average obtained from observations actually made in the case of a few piles initially cast. If the actual quantity is found to be considerably less, special investigations shall be conducted and appropriate measures taken.

The bentonite suspension used for piling work shall satisfy the following requirements:

- i. The liquid limit of bentonite when tested in accordance with IS:2720 (Part V) shall be more than 300 percent and less than 450 percent.
- ii. The sand content of the bentonite powder shall not be greater than 7 percent.
- iii. Bentonite solution should be made by mixing it with fresh water using pump for circulation. The density of the freshly prepared Bentonite solution should be between 1.034 and 1.10 gm/ml depending upon pile dimensions and type of soil in which the pile is to be installed. However, the density of bentonite solution after mixing with deleterious materials in the pile bore may be up to 1.25 gm/ml.
- iv. The marsh velocity when tested by a Marsh cone should be about 37 seconds.
- v. The differential free swell shall be more than 540 percent.
- vi. The PH value of the bentonite suspension shall be between 9 & 11.5.



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30.10.00**Codes and Standards**

The following Indian Codes and Standards (Latest) shall be used for design of the structural elements:

IS:456 : Code of Practice for plain and Reinforced concrete

IS:875 : Code of Practice for Design loads (other than earthquake) for Buildings and structures.

IS:2911: Code of Practice for Design and Construction of Pile Foundations – Bored Cast-in-situ Piles (Part 1/sec. 2)

IS:2911 : Code of Practice for Design and Construction of Pile Foundation – Load Test on Piles (Part 4)

IS:1786 : Code of Practice for HYSD bars

IS:2751 : Code of Practice for welding of mild steel bars used for reinforced concrete construction

IS:2062 : Steel for general structural purposes

SP:16 : Design Aids for Reinforced Concrete to IS:456.



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